



NORTH KENT 1

Surface and Subsurface Vibration Monitoring Report, Test Piles T5 and T42

Submitted to: Mr. Jody Law c/o North Kent Wind 1 LP 355 Adelaide Street West, Suite 100 Toronto, Ontario M5V 1S2



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REVISED REPORT

Executive Summary

This report summarizes the results of a test pile vibration monitoring program for the North Kent 1 project (NK1) completed as part of Section H1 of the Renewable Energy Approval (REA) document issued by the Ontario Ministry of the Environment and Climate Change (MOECC). The work was carried out in accordance with a vibration monitoring program prepared by Golder Associates Ltd. (Golder) dated March 3, 2017, subsequently approved by MOECC on March 6, 2017 for Pattern Development (Pattern) and Samsung Renewable Energy Inc. (Samsung). The intent of the work was to evaluate the surface and subsurface magnitudes, propagation and attenuation characteristics of ground vibrations associated with driving of test piles. This report focuses on:

- maximum vibration amplitudes near the pile driving source;
- patterns of vibration attenuation in soil and rock as compared to published methods of vibration attenuation prediction or analysis;
- magnitudes of vibrations at distant domestic water wells; and
- comparison of the site-specific data to published and regulatory vibration monitoring thresholds.

Vibrations of various magnitudes were detected by monitoring instruments during many different activities on the test pile T5 and T42 sites and during various activities at nearby domestic water wells. The resulting data has permitted an evaluation of distance-vibration attenuation behaviour for this site. The data further indicate that the character of vibrations at monitored water wells, as measured by the sensitive instrumentation, were not of concern as compared to those induced by common day-to-day activities. As compared to typical vibration magnitudes associated with other causes or published acceptable vibration thresholds for different activities and conditions, and as compared to background quiet¹ periods and deliberate actions to simulate day-to-day activities near the well location, vibrations at the residential well locations, were significantly below regulatory values for human perception and other published thresholds related to residential uses. It is our opinion that pile driving will not expose distant groundwater wells (e.g., more than 550 m) to vibrations in excess of those that the wells commonly experience otherwise.

¹ For the purposes of this report "quite" periods are defined as absent of construction activities, deliberate or clearly defined activities but inclusive of background vibrations, nearby road traffic and environmental influences (e.g., wind/air pressures).



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1.0 INTRODUCTION

This report summarizes the results of a test pile vibration monitoring program for the North Kent Wind 1 project (NK1) as part of Section H1 of the Renewable Energy Approval (REA) document issued by the Ontario Ministry of the Environment and Climate Change (MOECC). The work was carried out in accordance with a vibration monitoring program prepared by Golder Associates Ltd. (Golder) dated March 3, 2017, subsequently approved by MOECC on March 6, 2017 for Pattern Development (Pattern) and Samsung Renewable Energy Inc. (Samsung). The intent of the work was to evaluate the surface and subsurface propagation and attenuation characteristics of ground vibrations associated with driving of two test piles. Two test pile sites, T5 and T42, and the related vibration monitoring, are discussed in this report. Test pile driving and vibration monitoring, was completed at site T42 first followed by site T5. This chronological rather than numerical order is used throughout this report.

This report on vibration monitoring associated with the Turbine T42 and T5 site test pile locations focuses on:

- measured vibration amplitudes adjacent to the test pile driving source;
- vibrations amplitudes at the ground surface and in the subsurface soil and rock at known distances from the test pile driving source;
- magnitudes of vibrations at distant domestic water wells; and
- patterns of vibration attenuation in soil and rock as compared to published methods of vibration attenuation prediction or analysis; and
- a comparison of these vibrations to regulatory and published thresholds.

This report summarizes:

- a description of the source of vibrations (test pile driving);
- the subsurface conditions at the NK1 and T42 and T5 test pile sites;
- test pile monitoring work plan along with rationale for borehole depths, monitoring locations, vibration sensor selection and data processing equipment;
- field activities during test pile driving program;
- a summary of the monitoring data gathered during test pile driving and at other key times; and
- interpretations of the data.

Where applicable, this report also references "Information to be Submitted for Approval of Stationary Sources of Sound (NPC-233)" and the two relevant references cited in NPC-233 "Procedures (NPC-103)" and "Impulse Vibration in Residential Buildings, (NPC-207)" as published by MOECC. The MOECC NPC-233, NPC-103 and NPC-207 documents have been referenced for guidance related to background information and data provided in vibration monitoring reports and the thresholds for human perception of vibrations as a means for comparison purposes.





2.0 PROJECT DESCRIPTION

The NK1 project is planned to include 34 to 36 wind turbines producing 100 MW of electrical power constructed over a 12-month period within the Municipality of Chatham-Kent. The turbines are planned within an area generally between Wallaceburg and Chatham, Ontario, bound by Bear Line on the west, Centre Side Road on the east, Oldfield Line on the north and Darrell Line and Pine Line on the south. In the project region, the site topography is relatively flat and the primary land use is agricultural.

Two of the turbine sites, numbers T42 and T5, were selected for installation of a single foundation test pile at each site to refine the preliminary design of turbine foundations for the NK1 project. Work at the T42 test pile site was completed first and included a surface, subsurface and domestic water well vibration monitoring. The general location, site layout and subsurface instrumentation for the Turbine T42 area are illustrated on Figures 1 and 2. Work at the T5 site included surface and domestic water well vibration monitoring at the locations illustrated on Figure 3. In general, the topography surrounding the T42 and T5 sites is relatively flat, with a topographic relief generally less than about 2 m. Buildings in the vicinity, considered potential receptors under NPC-233, consist of rural residences and farm buildings, typical of southwestern Ontario and the nearest residences are more than 550 m from the T42 and T5 test pile sites.

Each of the turbines is planned to be supported by a circular mass concrete foundation, measuring about 18.5 m across that will be supported by 20 to 26 pipe piles driven into the ground. The final number of driven piles had yet to be determined at the time of this report pending the outcome of field testing. At the time this report was prepared, the preliminary foundation design was based on the centre of the future piles being a radial distance of 8.675 m from the centre of the foundation and typically ranging in length from about 20 to 30 m depending on location.

Test pile driving was undertaken between March 29 and 31, 2017 at the T42 site and on May 3 and 4, 2017 at the T5 site. The test pile program consisted of driving one pile at each of sites T42 and T5 where the pile was driven to the depth of pile refusal to penetration to measure resistance to downward loads. The test piles will not form part of the foundation in the future and will be abandoned within the footprint of the future foundation.

The test piles, selected to be consistent with the future foundation piles, were about 410 millimetre (mm) outside diameter, driven with a closed end and filled with structural concrete after driving. The closed end was formed with an approximately 430 mm diameter, 50 to 60 mm thick plate welded to the tip of the pile.

With respect to the information required by NPC-233, the test pile driving that forms the source of the vibrations was completed using a Berminghammer B-32 diesel hammer with a rated energy of 110 kJ with an operational hammer rate of 35 to 56 blows per minute. Specifications related to this pile driving hammer are provided in Appendix A. The pile driving hammer and pile were suspended on a mobile crane mounted on a tracked/crawler carrier. Pile driving occurred during daytime and normal construction operational hours.

Ground surface and subsurface vibration monitoring was undertaken at the T42 site and ground surface monitoring was undertaken at the T5 site. Details regarding the test pile vibration monitoring are presented in subsequent sections of this report. As part of the monitoring plan, three pairs of boreholes were drilled at the T42 site to allow installation of subsurface instruments and these borehole locations are illustrated on Figure 1 along with the location of the T42 site borehole completed by AMEC (2016a) and locations of the monitored domestic water wells. A profile of the subsurface conditions and subsurface instruments is shown on Figure 2. Figure 3 illustrates the location of the surface and domestic water well monitoring locations for the T5 test pile site.





3.0 GROUND VIBRATIONS – BACKGROUND

Ground vibrations are caused by many every-day and construction activities. Every-day sources include normal road traffic and wind forces on structures and trees. Seismic activity (earthquakes) are another natural source of large ground motion. Sources related to human activities include stationary and mobile machinery of all different sizes where the machinery is in contact with the ground and construction operations such as soil compaction and pile driving among others. Much like ripples caused by dropping a stone in a pond, ground vibrations propagate radially in all directions from the source. As the ripples propagate away from the source, the amplitude (height of the wave) and frequency (e.g., time between wave peaks or troughs)² decrease and the rate at which the amplitude and frequency decrease is related to the physical character of the material (e.g., density, etc.) through which the wave propagates. Vibrations are usually described in terms of time and displacement, such as velocity or acceleration. For example, the distance a particle of ground (or water) moves up and down (displacement) as the wave propagates through the medium will be measured in units of length (e.g., metres (m)). This same particle will travel that distance in a given time and the distance divided by time is the average particle velocity. This same particle will also, however, accelerate from rest, to a maximum (peak) velocity before it then decelerates and reverses direction in the pattern of wave oscillation. Wave velocity can also be used to express the speed at which a wave crest (or trough or any other part of the wave) travels through the medium (ground, water, steel, etc.) away from the source. Velocity of a particle moving up and down (or side to side) from trough to crest and back again in an oscillating wave, however, should not be confused with the possible or characteristic velocity that a moving vibration wave crest can pass through the medium.

For simple, harmonic motion (e.g., uniform waves), the mathematics relating acceleration, displacement, velocity and time can be summarized as³:

$$V = \frac{d}{dt}D = \int Adt$$
$$A = \frac{d}{dt}V = \frac{d^2}{dt^2}D$$

and, for a simple sine wave function, at peak values when the trigonometric function is equal to 1, these equations reduce to:

$$D_o = \frac{V_o}{2\pi f} = \frac{A_o}{(2\pi f)^2}$$
$$V_o = 2\pi f D_o = \frac{A_o}{2\pi f}$$
$$A_o = (2\pi f)^2 D_o = 2\pi f V_o$$

A



 $^{^{\}rm 2}$ measured in cycles per second or Hertz, Hz

³ See, for example, Fundamentals of Physics, 3rd Edition, by D. Halliday and R. Resnick, 1988 or USBM RI 8507 (see references at conclusion of text).

where:

A and A_o	=	acceleration and peak acceleration, respectively;
D and D_o	=	displacement and peak displacement, respectively;
V and V_o	=	velocity and peak velocity, respectively;
f	=	frequency;

Complex and transient vibrations, such as those created by construction, cannot be approximated by the basic equations noted above in isolation and usually computer-aided waveform⁴ analysis programs and reasonable approximations or simplifications are made. When characterizing ground vibrations as related to human perception and damage criteria, the most common parameters are frequency along with fractions of gravitational acceleration (*g*, with a value 1 *g* = 9.81 m/second²), peak particle velocity (ppv)⁵ or RMS velocity⁵. Within this report, acceleration and peak particle velocity are used along with frequency to characterize the measured ground vibrations. Methods of evaluating the complex vibrations are also described in subsequent sections of this report.

4.0 SUBSURFACE CONDITIONS AT NK1 T42 AND T5 TEST SITES

Available information relating to the subsurface conditions of the project site is summarized as background and context for the vibration monitoring and for consistency with the intent of NPC-233. Details of the subsurface conditions are described in AMEC (2016a, 2016b) and Golder (2016). A copy of the records of boreholes T42 and T5 completed by AMEC (2016a) are included in Appendix B of this report for reference. Additional data gathered by Golder at the T42 site are included in Appendices C and D.

In summary, the NK1 project site lies in the St. Clair Clay Plains Physiographic region of Southwestern Ontario (Chapman and Putnam 1984). The T42 and T5 sites are typical of the NK1 stratigraphy overall, being characterised by relatively shallow surficial deposits of layered sand, silt and, in some areas, silty clay underlain by a regionally-extensive and thick silty clay deposit. Beneath the silty clay, a basal glacial till unit is commonly found. This glacial till unit is of variable grain size distribution. Bedrock of the Kettle Point Formation is found beneath the glacial till, where present, at depths ranging from about 12 to 28 m below ground surface. The major geologic units, and the conditions found at the specific T42 and T5 sites, are described from the ground surface down as follows:

■ Topsoil is commonly encountered near the surface and, in many areas, represents tilled and worked farmlands. About 0.4 m of topsoil was encountered by AMEC (2016a) at both the T42 and T5 sites.

⁵ Peak particle velocity is the maximum oscillation speed of a particular particle (of ground in this case) as it driven by a passing displacement wave. Typically, peak particle velocity is measured in three mutually perpendicular directions and maximum vector resultant is used to describe the vibration intensity. Peak particle velocity is the most common parameter used for defining threshold vibration amplitudes associated with construction and blasting. Other systems measure, report and limit vibrations based on the root mean square (RMS) velocity. The RMS velocity is the square root of the average of individual velocity measurements squared, typically calculated over a time interval of one second. The RMS amplitude is always less than the peak particle velocity and the two can be related through a "crest factor" that is defined as the peak particle velocity divided by the RMS velocity. The US Federal Transit Administration guidance on noise and ground-borne vibration notes that the crest factor is "....always greater than 1.71, although a crest factor of 8 or more is not unusual for impulsive signals." Sometimes, vibrations are also reported in terms of decibels (VdB) calculated as 20 times the logarithm (base 10) of RMS velocity divided by a reference velocity. Accepted reference vibration velocities are 1x10⁶ inches/second in the U.S. and 1x10³ m/s or 5x10³ m/s elsewhere (FTA 2006).



⁴ velocity or acceleration wave patterns as related to time



- In some areas, below the topsoil, deposits of sand and silt exist ranging in total thickness between nil and 8.2 m with an average of 1.7 m, based on the boreholes completed for this project (AMEC 2016a). At the T42 test pile site, a 1.0 m thick layer of silty clay was found beneath the topsoil, and a 0.7 m thick layer of fine sand existed below the topsoil and upper layer of silty clay. At the T5 site, these same layers were about 1.0 m and 0.7 m thick, respectively.
- Below the sand and silt, where present, the majority of the soils consist of a regionally extensive deposit of very soft to firm silty clay, ranging in thickness at the planned turbine sites from about 10 to 20 m with an average of 13.2 m (AMEC 2016a). At the T42 and T5 sites, the clay extended to depths of approximately 18 m and 14.6 m below ground surface, respectively.
- At various turbine, sites sand and gravel soils with varying proportions of silt and clay, either representing ice-contact outwash or basal glacial till soils, were commonly found between the overlying thick silty clay deposits and the underlying bedrock. These soils represent the local aquifer and are as much as 10.4 m thick with an average of about 2.2 m (AMEC 2016a). Glacial till, consisting of a broadly-graded heterogeneous mix of granular and fine grained particles was encountered at the T42 and T5 sites. The AMEC borehole samples were described as "Sand and Gravel Till, trace silt, trace clay, grey, compact, rock fragments, saturated, very dense". Glacial till was encountered by AMEC at the T42 site between depths of about 17.7 m and 19.9 m below the ground surface. The boreholes drilled by Golder encountered the glacial till at depths of about 17.7 to 17.8 m below ground surface. Grain size distribution testing completed by Golder for samples of the glacial till indicated that, at the Golder borehole locations, the glacial till was more fine-grained than at the AMEC T42 borehole location, likely indicative of the natural variability in till composition (AMEC 2016b). At the T5 site, the glacial till was encountered at a depth of 14.6 m below the ground surface and was 2.2 m thick.
- Fine-grained shale bedrock of the Kettle Point Formation was encountered beneath the glacial till at the T42 site at depths of between about 18.9 and 19.9 m below ground surface based on the AMEC (2016a) and Golder data. Bedrock was encountered at a depth of 16.8 m below ground surface at the T5 site (AMEC 2016a).

4.1.1 Silty Clay

The near-surface soils in the area are mapped largely as being of glaciolactustrine or glaciomarine origin (Chapman and Putnam 1984, Barnett et al. 1991). These soils possess a distinctively till-like structure with a small fraction of sand and gravel sized particles distributed randomly throughout. For the purposes of this report, only those dense or hard soils that exist between the bedrock and overlying softer or looser sediments are referred to as "glacial till" or "till". The silty clay soils in the geographic region are generally composed of "rock flour" of silt-and clay-size particles with activity⁶ typically less than about 0.6 and Atterberg limits plasticity indices ranging from 11 to 24 per cent based on 5 tests AMEC (2016a). The near-surface silty clay soils are typified by a stiff, brown and fissured "crust" where these soils are not overlain by saturated silt and sand. Below the "crust", if and where



⁶ Plasticity index (PI) divided by percent <0.002 mm clay.



present, the silty clay is saturated, grey and of relatively low strength. The undrained shear strength of this soil (S_u) typically ranges from about 15 to 35 kilopascals (kPa) with an average of about 21 kPa.

4.1.2 Granular and Basal Till Deposits

Granular soils (silt, sand and gravel) and basal till soils that overlie the bedrock in the area are the result of glacial action along the bedrock surface during advances and retreats of continental ice sheets. Compositionally, the till is formed of a wide variety of mineral types including fragments of hard igneous and metamorphic rocks as well as sedimentary rocks including fragments of the regional Kettle Point shale bedrock. Particle size distribution and permeability (hydraulic conductivity) of these soils varies significantly, depending on the amount of silt and clay-size particles within the soil mass. Grain size distribution tests completed on samples obtained by Golder from the T42 site (see Appendix D) indicated that this glacial till consists of about 60 to 72 per cent silt and clay-size particles at the location of the Golder boreholes. Standard Penetration Test (SPT) N values⁷ ranged from about 10 to 103 blows per 0.3 m of sampler penetration (10th to 90th percentile values, respectively) with an average value of about 50 blows per 0.3 m of sampler penetration (thus categorised as compact to very dense or hard to very hard). Based on the AMEC (2016a) data, the glacial till soils are not as dense at the T5 site as at the T42 site.

4.1.3 Bedrock

The Upper Devonian Kettle Point Formation is the upper-most bedrock formation in the NK1 project area and Chatham-Kent Region. The Kettle Point Shale Formation is a black, siliclastic, organic-rich shale and siltstone with minor green-grey, organic-poor shale and siltstone interbeds. In general, the Kettle Point Formation is of relatively high strength compared to other shale formations in southern Ontario. During subsurface exploration at the NK1 turbine sites, standard penetration testing could not drive the samplers into the rock and augers could not penetrate the rock. Laboratory testing for the NK1 project included four unconfined compressive strength (UCS) tests on core samples and the results ranged from about 49 megapascals (MPa) to 72 MPa. Rock quality designation (RQD) data reported by AMEC (2016a) ranged between a minimum of 20 per cent to 99 per cent, with an average of about 76 per cent. This same RQD data indicated that there was only a 10 per cent chance that the RQD value would be below 50 per cent for any given core run (i.e., 10th percentile value).

4.1.4 Regional Hydrogeology

About 90 per cent of the water supply wells in nearby Lambton County, also situated over the Kettle Point Formation, obtain water from the sand and gravel near the top of the bedrock" (Kent et al. 1986), commonly referred to as the "interface" or "contact" aquifer, while others in the Chatham-Kent area penetrate into the bedrock (Singer et al. 2003) depending on the local hydraulic conductivity of the soils immediately overlying the bedrock and the underlying upper few metres of bedrock.



⁷ American Society for Testing and Materials standard ASTM D1586.



5.0 MONITORING WORK PLAN

The test pile vibration monitoring work plan summarized below, supplemented by monitoring at the T5 site, was designed to address vibration monitoring during initial construction of a test pile at the T42 turbine site. The need for, scope, frequency and duration of vibration monitoring during the later construction and long-term operational phases of the facility will be determined subsequent to this report.

The test pile vibration monitoring program at the T42 site, as implemented, generally consisted of the following major elements:

- 1) subsurface vibration sensors installed within three pairs of boreholes at depths
 - a. within the bedrock near the bedrock and overlying glacial till interface; and
 - b. at the approximate mid-depth of the soft to firm clay soils;

where the borehole pairs were located at distances of 10, 30 and 50 m from the test pile.

- 2) ground surface vibration monitoring within the construction site
 - a. very close to the test pile (about 3 m distant); and
 - b. close to each of the borehole pairs; and
- 3) vibration sensors installed on two domestic water well casings at neighbouring properties.

For the T5 site, the vibration monitoring program consisted of the following elements:

- 1) Ground surface vibration monitoring within the construction site
 - a. very close to the test pile (about 3 m distant); and
 - b. at distances of 10, 30 and 50 m from the test pile; and
- 2) vibration sensors installed on two domestic water well casings at neighbouring properties.

These elements are discussed in greater detail below.

5.1 Subsurface Vibration Monitoring, T42 Site

As part of the monitoring plan, three pairs of boreholes were drilled to allow installation of subsurface instruments as illustrated on Figures 1 and 2. The borehole completed by AMEC is also illustrated on Figures 1 and 2. The intent of the borehole layout was to obtain surface and subsurface vibration measurements at different distances from the turbine location. Distances to borehole monitoring sensors were chosen to be 10, 30 and 50 m from the driven test pile. These distances were chosen to capture three points within the region of most significant changes in vibration magnitudes expected at this site based on typical vibration attenuation characteristics. Using well-known distance and vibration amplitude attenuation relationships as a guide (e.g., CALTRANS, 2004) the anticipated magnitudes of vibrations at different distances from piling are summarized in Table 1, expressed as a percentage of the magnitudes at the source (pile). Shaded cells in Table 1 illustrate the selected off-set distances from the test pile to surface and subsurface monitoring locations. Instrument installation depths are illustrated on





Figure 2 and identified on the borehole records included in Appendix C. Additional details regarding equipment and installation depths are provided in Section 5.4 below.

Distance from Source/Pile (m)	Approximate Percentage of Peak Magnitude at Source (%)		
	In Soil	In Rock	
0	100.0	100.0	
3	22.0	34.0	
10	4.0	10.0	
30	1.0	4.0	
50	0.5	2.0	
70	0.3	1.5	
80	0.2	1.3	
100	0.2	1.0	
150	0.1	0.7	
300	<0.5	<0.5	
500	<0.1	<0.3	

 Table 1: Estimated vibration magnitudes and distances from pile driving

Between March 14 and 18, 2017, boreholes were drilled in pairs, consisting of one deep borehole cored into bedrock for one set of sensors (boreholes BH-101, BH-102 and BH-103) and an adjacent borehole for installing sensors at the approximate mid-depth of the soft to firm clay soils (boreholes BH-101A, BH-102A and BH-103A). Borehole pairs were selected to facilitate installation for the following reasons:

- Vibration monitoring sensors needed to be firmly coupled to the ground (rock or soil) using cement grout fully around and above the sensors.
- Filling the entire volume of a single borehole with cement grout and two sets of sensors at the different depths would have created a hard inclusion and coupled the sensors together through the hardened column of grout. If the sensors installed in rock and those installed at the mid-depth of the soil profile were coupled together the resulting data would be compromised given the very different vibration attenuation characteristics of soil and rock.
- The boreholes had to be a minimum of 125 mm inside diameter to permit appropriate installation of the sensors.

Drilling of the boreholes was completed using conventional geotechnical exploration equipment mounted on a Terramac rubber track crawler system (see Photographs in Appendix E) operated by a specialist drilling and licensed water-well contractor under the direction of a Golder technician. Drilling was completed using a combination of hollow stem augers for the top 5 to 7 m, with the remaining depth drilled using mud-rotary drilling tools and soil and rock coring using PQ-size wireline drilling equipment. For each borehole, a protective steel





casing was set and grouted in place over the top 4 to 5 m with a portion of this pipe above ground to house cables and provide a visible identification of the sensor locations.

An automatic Standard Penetration Test (ASTM D1586) hammer system and thin-walled tube methods were used for occasional soil sampling at selected depths and locations within the boreholes. The PQ soil coring system was used through the glacial till and into the rock. Conventional field vane shear tests were carried out in one borehole in general accordance with ASTM D2573. All of the samples obtained during the investigation were transported to our laboratory for further examination and testing. As part of the subsurface explorations and testing, one sample of the glacial till soils obtained with the PQ coring system was subjected to laboratory mechanical and hydrometer grain size distribution testing. The soil stratigraphy encountered in the boreholes and the results of the field and laboratory testing are shown on the Record of Borehole and laboratory test sheets included in Appendices C and D, respectively, and Figure 2.

To address competing schedule constraints related to accelerometer availability, driller availability, the test pile driving and grout curing times, all boreholes were drilled prior to installing the subsurface instruments between March 14 and 18, 2017. After completing coring in the deep holes and drilling to the planned depths in the shallow boreholes, the holes were flushed of cuttings and drilling mud was left to fill the hole. Prior to installing the vibration sensors, the borehole was checked again for depth continuity. One borehole, BH-101A, was found to have experienced caving of the near surface granular soils and was abandoned by filling with bentonite. A second hole was drilled, off-set approximately 2 m from the first hole (see Figure 1). In all boreholes, approximately 20 to 30 litres of cement grout was injected into the bottom of the boreholes using a tremie pipe. Drilling mud displaced from the borehole was collected in the drilling circulation tank at the surface. The vibration sensors were then lowered into place, sinking these into the grout for full encapsulation and bonding of the sensor systems to the surrounding ground. Subsequently, the boreholes were backfilled above the cement grout with bentonite chips that, after hydrating and swelling, would exhibit a strength and consistency similar to that of the surrounding soft natural clay soils. Sensor installation depths are indicated on the Record of Borehole sheets included in Appendix C. Installation of the sensors and grouting were completed during March 20 through 22, 2017.

Groundwater levels were observed in the boreholes during drilling as feasible given the different drilling systems used for this work, some of which required use of potable water for drilling and thus masked observations of local groundwater levels. Encountered groundwater levels are shown on the Record of Borehole sheets included in Appendix C. Upon completion of drilling, sampling, instrument installation, grouting and backfilling with bentonite the completed boreholes complied with Ontario Regulation 903 (as amended) with respect to plugging of drilled holes and wells.

5.2 Surface Monitoring, T42 and T5 Sites

Surface vibration monitoring points at the T42 and T5 sites are illustrated on Figures 1 and 3. The monitoring positions at the T5 site were selected to be consistent with the surface subsurface monitoring points at the T42 site with an additional location near (within 3 m) of the pile driving. The point close to the pile was selected to better capture relatively large amplitude vibrations near the source for better definition of vibration attenuation behaviour. At the borehole pairs at the T42 site, the surface points were located between the borehole casings to provide surface data at a position consistent with the subsurface monitoring systems.



5.3 Domestic Water Wells

The monitoring work plan called for monitoring at or near a domestic water well or at a location close to the property line nearest a well. Water well locations for which permission was granted for monitoring, are illustrated on Figures 1 and 3 and are summarized in Table 2, below. In accordance with the approved work plan, vibration measurements were not made at the bottom of the well or with the sensors submersed under water since such work would require removing the pump and/or piping and disrupting the water supply or otherwise influencing the internal character of the well and water. Vibration monitoring was also to be conducted without altering or damaging the well casing. Photographs of the well casings are included in Appendix E.

Municipal Address	Available Well Details	Distance from Test Pile (m)
25209 Baldoon Road (Well #1, T42)	Well close to south side of house, near junction between older home and addition. Well depth extended to 37.5 m and serviced 25 years ago.	1,102
25345 Baldoon Road (Well #2, T42)	Well in farm field. Total depth about 21.3 m with bedrock encountered at 18.9 m and a stabilized water level at about 3.87 m below ground surface. Water is removed from well using a vacuum system with a pipe installed about 7 m down into the well. Six other well attempts were made on this property in 1974 before encountering water.	1,283
8522 Bush Line (Well #3, T5)	Well located in driveway area south of house. Well depth reported to be 18.7 m below ground surface. Well is reported to be equipped with piston pump but the location of the pump was not confirmed.	911
26347 St. Clair Road (Well #4, T5)	Well enclosed within small well house, located south of house. Pump within well house was not connected and not powered at the time of monitoring.	1,033

Table 2: Summary of Domestic Water Wells

5.4 Vibration Sensor and Data Logging Systems

Vibration sensors were selected based on the need to collect vibration data over a wide range of amplitudes and relatively low frequencies and to provide a suitable degree of instrumentation redundancy in the event of field problems. Based on the CALTRANS (2004) methodology, driving of piles at a hammer rate of 35 to 56 blows per minute was expected to generate vibration amplitudes in both the soil and rock in the range of 1 to 150 mm/s (ppv) or 0.01 to 1 times the gravitational acceleration constant (*g*) with dominant pile and ground response vibration frequencies in the range of 5 to 70 Hertz (Hz) in the near vicinity of the driven pile (i.e., within 3 to 30 m). However, it was also recognized that at longer distances from the test pile, vibration amplitudes could be one to two orders of magnitude smaller than those associated with pile driving near the test pile location.

As well as planning for appropriate sensitivity and frequency response characteristics, the sensors selected for subsurface use also had to be of physical dimensions that were compatible with installation in conventional borehole diameters in soil and rock (ranging from about 100 to 125 mm). Based on these considerations, the following sensors were selected for use in the approved work plan:



- 1) The PCB356B18 triaxial accelerometers were selected for measuring underground vibrations in three orthogonal directions. These sensors are rated as having a sensitivity of 1000 millivolts (mV) per 1 g. The manufacturer states that these sensors are capable of a broad band resolution to 0.25 milli-g (2.5x10⁻⁴g) in the range of 0.5 to 100 Hz. Triaxial sensors were specifically selected to allow computation of peak particle velocity in three orthogonal directions and vector sums as a basis for comparing measured vibration amplitudes to published research related to construction-induced damage or disturbance criteria.
- 2) Meggit 731A uniaxial accelerometers were selected for measuring very small underground vibration amplitudes at longer distances from the turbine. These sensors are rated as having a sensitivity of 10,000 mV/g with a broad band resolution (signal detection above noise level) of about 1 μg (1x10⁻⁶ g). These sensors only measure vibrations in one direction (in this case, the vertical direction as relative to the sensor base and mounting orientation).
- 3) For measuring vibrations at the ground surface and at the domestic water wells with portable systems, uniaxial PCB393A03 accelerometers were selected for measuring small vibration magnitudes. These sensors are rated as having sensitivity of 1,000 mV/g with a broad band resolution of about 1x10⁻⁵ g between frequencies of 1 and 10,000 Hz.
- 4) Ground surface vibration amplitudes near the pile driving were anticipated to be above the rated limit of the PCB393A03 sensors. Therefore, conventional triaxial geophone (velocity measurement system) construction vibration monitoring systems were used for this purpose. For monitoring at the T42 test pile site, the Instantel Minimate system was selected for the 3 and 10 m distant positions which has a rated resolution up to 254 mm/s peak particle velocity with an accuracy of 0.5 mm/s over a frequency range of 2 to 250 Hz. These systems are commonly used for monitoring pile driving and other construction-related vibrations, are robust for difficult field environments and could measure the high-amplitude vibrations very close to the pile driving. These systems rely on triggering impulses and provide output in the common peak particle velocity parameter in three orthogonal directions. For ground surface monitoring at the T5 site, two additional Instantel Minimates were used with the one located at the farthest distance from the test pile (50 m) equipped with a low-level geophone with a maximum amplitude rating of 25.4 mm/s.
- 5) A pre-programmed Crystal Instruments Spider20 data logger connected to a PCB393A03 accelerometer was obtained for this purpose. The data logger was provided and programmed by the supplier according to requirements defined by Golder specifically to meet the needs of this project and collect continuous vibration data when operating.

Copies of manufacturer specifications and manufacturer calibrations for the instruments used for this phase of vibration monitoring are included in Appendix F. Prior to field deployment, these sensors were checked in Golder's offices once upon arrival and a second time after the sensors were connected with all cables, power sources and data collection systems. Checking of vibration responses was accomplished using a Modal Shop trusted sources TMS9110D and TMS9100D. Accelerometer mounting systems used for adapting to field conditions at the well sites are addressed in Appendix G.

Subsurface field deployment of the sensors in boreholes filled with water or drilling fluids required protection of the systems from the down-hole environment and to provide a mechanism for controlling the depth and orientation of the sensors within the boreholes. Therefore, a housing was designed and constructed as follows:



- 285 to 310 mm long, 89 mm diameter, Schedule 40 ABS pipe housing with the PCB 356B18 triaxial accelerometers mounted by bolting to the inside of the pipe where, because of the physical construction of the accelerometer, the:
 - positive y axis was oriented horizontally toward the test pile "longitudinal" direction;
 - positive x axis was oriented vertically downward; and
 - z axis was horizontal and perpendicular to the y axis "transverse" direction;
- 300 mm long, 76 mm diameter, round steel stock for adding base weight for sinking through cement grout and as base for mounting Meggit 731A uniaxial accelerometers (mounted with positive vertical axis upward);
- the pipe was bolted to the steel stock such that the two accelerometers were physically and rigidly coupled to the housing within 100 mm vertical distance separating the individual accelerometers or about 134 mm to 156 mm separating the mounting positions within the housing;
- sand to fill interior around mounted sensors;
- protective end cap with hole for passing sensor wires through; and
- twin steel cables for lowering and orienting the sensor and housing package, with the cables mounted at 180 degree positions at the housing top to facilitate orientation of the triaxial accelerometer directions.

While the connections and instrument housings were water-tight and sealed, prior to installing the accelerometers in the housing, each sensor and cable connection was coated with an epoxy sealant and spray-on plastic to assist with protecting the electrical connections. Photographs of the sensors and housings at various stages of the assembly are illustrated in Appendix E. Prior to subsurface deployment, output from the combined sensors and housings was compared with output from an independent and field-checked accelerometer where all sensors and housing were clamped to the same source of vibrations (drilling rig) as another confirmation of input-output reliability (see photographs in Appendix E).

Surface deployment of the Instantel Minimates was carried out in accordance with the manufacturer's instructions where the geophone was coupled to the ground using the provided pins and sand bags were placed over top of the sensors to minimize influences of air compression waves and provide better ground coupling. The data recording trigger thresholds for the Minimate systems at the T42 test pile site, located at 3 and 10 m distant from the test pile, were initially set at a value of 4 mm/s (ppv) during the first day of monitoring and was later set during this same day to 1 mm/s (ppv) to capture more vibration waveform data prior to and as the pile encountered hard driving conditions. While longer periods of recording can be achieved, internal data memory is limited and continuous recording for long periods of time with low trigger thresholds can be problematic. At the T5 test pile site, the low-level geophone system located at 50 m from the test pile was set to trigger recording at 0.25 mm/s particle velocity.

Surface deployment of the PCB393A03 accelerometers was carried out using steel pins driven into the ground with a threaded sensor coupling at the pin top. The small excavations for these sensors were covered with a small, lightweight, plywood square without contacting the sensor or its wires to minimize external influences (weather), to minimize the potential for materials falling into the small pit or on the sensors, and for site safety (see photograph of small excavation in Appendix E).





The field configuration of the sensors and data logging systems are summarized in the table below. Because of the remote location, all power for the subsurface monitoring systems was provided by a small gasoline-powered alternating current generator.

Table 3. Summary	y of Sensor Identification	Denth and Sid	nal Paths T42 Site
Table 5. Summar	y of Sensor Identification	, Depin anu Sig	jiiai railis, 142 Sile

Sensor Identification	Signal Conditioner	Data Logger	Data Storage
Pile Driving Sensor (3 m)	-	-	-
SS3 (Minimate)	Self-	Contained	Unit
Borehole Pair 101 (10 m from pile)			
101SS (Minimate)	Self-	Contained	Unit
Subsurface Shallow			
101SU (7.144 m)			
101STx (7.004 m)			
101STy (7.004 m)			dny
101STz (7.004 m)			Dell Notebook Computer and External Memory Backup
Subsurface Deep			
101DU (20.693 m)		000	
101DTx (20.550 m)	PCB 481A02 and 482A22		
101DTy (20.550 m)	482,		
101DTz (20.550 m)	pu ⁷	CR5	Ext
Borehole Pair 102 (30 m from pile))2 a	ell C	and
101SS (PCB393A03)	31A(Campbell CR5000	nputer
Subsurface Shallow	3 48		
102SU (7.591 m)	PCI		Cor
102STx (7.455 m)			yoo
102STy (7.455 m)			oteb
102STz (7.455 m)			I NC
Subsurface Deep			Del
102DU (20.723 m)			
102DTx (20.589 m)			
102DTy (20.589 m)	7		
102DTz (20.589 m)			

Sensor Identification	Signal Conditioner	Data Logger	Data Storage
Borehole Pair 103 (50 m from pile)			g
103SS (393A)		Ś	r an <up< td=""></up<>
Subsurface Shallow		ient:	puter al Backup
103SU (7.692 m)		National Instruments CDAQ 9188	Ē
Subsurface Deep		Inst VQ 9	ook Com Memory
103DU (20.843 m)		cDA	
103DTx (20.687 m)		latic (ell Noteb External
103DTy (20.687 m)		2	ell N Ext
103DTz (20.687 m)			Δ
Well #1 25209 Baldoon Road (1,102 m)	Rion DA-21, Battery Powered		vered
Well #2 25345 Baldoon Road (1,283 m)	Crystal Instrur Battery Power		er20,

Note: 1) depth shown in parentheses measured from ground surface to mounting bolt on accelerometer

Table 4: Summary of Sensor Identification and Signal Paths, T5 Site

Sensor Identification	Signal Conditioner	Data Logger	Data Storage
Pile Driving Sensor (3 m from pile)			
SS3 (Minimate)	Self-C	ontained U	nit
Ground Surface Sensor (10 m from p	ile)		
SS10 (Minimate)	Self-Contained Unit		
Ground Surface Sensor (30 m from pile)			
SS30 (Minimate)	Self-Contained Unit		
Ground Surface Sensor (50 m from pile)			
SS50 (Minimate with low-level Geophone)	Self-Contained Unit		
Well #3 8522 Bush Line (911 m)	Rion DA-21, Battery Powered		
Well #4 26347 St. Clair Road (1,033 m)	National Instruments CDAQ 9188, Dell Notebook Computer and External Memory Backup		



6.0 FIELD VIBRATION MONITORING ACTIVITIES

6.1 Background Vibration Measurements

6.1.1 T42 Test Pile Site and Associated Domestic Water Wells

Once the vibration sensors were installed in the boreholes, data was gathered from the subsurface sensors on March 20 and 23, 2017. Site activities during these two days are summarized in Table 5, below. On March 20, 2017 the monitoring activities were limited to system checks to confirm that the first sensors installed were responding to voltage signals once in the borehole. On March 23rd, 2017 data was collected as one measurement of background conditions and to further test the entire system. This background data was obtained within 24 hours after installing the instruments and, since the grout may not have sufficiently cured at this time, additional background data was gathered on other dates when no construction vehicles were operating and test pile driving was not taking place. Activities on subsequent dates are described under Section 6.2.

Time	Activity		
March 20	•		
16:00	Drilling rig idling at BH103, tracks on ground, no sampling		
18:21	Drilling rig idling at BH102, tracks on timber mats, no sampling, hammer tap on casing BH102		
18:24	Drilling rig idling at BH102, tracks on timber mats, no sampling		
March 23			
11:52	Excavator moving on timber mats, passing sensor locations		
12:08 to 13:00	Vac truck operating on timber mats to remove spoils from bin		
12:43	Kicked casing at 102A with sensor clamped to casing		
13:20 to 14:18	Quiet ⁸ site		
13:53	Car on road		
14:21	Vacuum truck backing along timber mats		
14:22	Vacuum truck operating on timber mats to remove spoils from bin		
15:20	Vacuum truck left site		
15:20 to 15:30	Quiet site		

Table 5: Summary of Site Activities during Initial Vibration Monitoring System Tests and Measurements, T42 Site

Following permission to enter private properties, background vibration readings were obtained at each of the domestic water supply well locations associated with the T42 test pile site (Wells #1 and #2) on March 28, 2017. Background readings were obtained with a PCB393A03 sensor since it was the more sensitive of the devices available and the self-contained Rion data logger was most suited to this application.

⁸ For the purposes of this report "quite" periods are defined as absent of construction activities, deliberate or clearly defined activities but inclusive of background vibrations, nearby road traffic and environmental influences (e.g., wind/air pressures).



Table 6: Summary of Site Activities during Background Monitoring at Wells #1 and #2

Time	Activity				
March 28, 2017	·				
8:50	On site at Well #1 and attached accelerometer to well casing				
10:20	Monitoring system active				
10:20 – 10:25	Quiet period, no activities				
10:26	Golder staff member jumped up and down 3 times at 0.9 m from well				
10:27	Golder staff member jumped up and down 3 times at 1.8 m from well				
10:28	Golder staff member jumped up and down 3 times at 2.7 m from well				
10:29	Golder staff member walked toward well				
10:29 – 10:30	Transport truck drove past on Baldoon Road				
10:30	5 light finger taps on well casing lid				
10:31:45 – 10:32	Cell phone on well lid ringing in vibrating mode				
10:31	Walking away from well				
10:34:30	Climbed steps, knocked on door and descended steps				
10:35	Walked toward well and retrieved cell phone				
10:37	Started van in driveway at Baldoon Road				
10:37 – 10:38	Drove van in driveway to end of drive, drove back and parked 3.3 m from well, shut off engine				
10:41	Opened and slammed van door				
10:42	Opened and slammed van door again				
10:43	Started van				
10:41	Cars passing on Baldoon Road				
10:45 – 10:50	Quiet site except for individuals speaking at about 3 m from well				
10:50	Stopped monitoring and removed system				
10:50 – 11:00	Relocated to Well #2 and mounted accelerometer				
11:07	Monitoring system active				
11:21	Golder staff member jumped up and down 3 times at 1 m from well				
11:22	Golder staff member jumped up and down 3 times at 2 m from well				
11:23	Golder staff member jumped up and down 3 times at 3 m from well				
11:24	5 light finger taps on well casing lid				
11:22 – 11:50	Walking toward and away from well				
11:25 – 11:30	Quiet site				
11:29	Backhoe on road passing site (approximately 50 m away)				
11:31	Cell phone on well lid ringing in vibrating mode				
11:32	Removed cell phone, accelerometer and left site				



6.1.2 T5 Test Pile Site and Associated Domestic Water Wells

Following permission to enter private properties, background vibration readings were obtained at domestic water supply well locations associated with the T5 test pile site on May 3, 2017 prior to test pile driving. Additional background readings were obtained prior to pile driving on May 4, 2017. All background readings were obtained using the same accelerometer and data logger systems as for monitoring during test pile driving. Monitoring and other activities at the Well #3 and Well #4 locations during times when pile driving was and was not occurring are all summarized in Table 9 and Table 10, below.

6.2 Pile Driving Vibration Monitoring

6.2.1 T42 Test Pile Site and Associated Domestic Water Wells

Vibration monitoring was carried out prior to, during and after pile driving on March 29 and 31, 2017. Heavy rains and wind precluded pile driving and monitoring on March 30, 2017. Pile driving activities are summarized in Table 7, below.

Time	Activity				
March 29, 2017	(See Table 8, below for activities at domestic water wells during pile driving)				
9:35 to 10:16	Minimate set up and unit clocks set. Recheck 102SS and 103SS (surface accelerometers) with controlled source.				
10:47	Surface monitoring systems active				
11:20	Subsurface monitoring systems active at 103/103A				
11:24	Subsurface monitoring systems active at 101/101A, 102/102A				
12:47	Contractor starts lifting pile				
12:55	Pile hammered twice to set				
12:56	Pile driving starts				
13:00	Pile tip at 4.0 m				
13:02 to 13:22	Channel 14 of data logging system temporarily lost connection (accelerometer 102STX)				
13:03	Pile tip at 10.75 m				
13:05	Pile driving stops and lifting bracket cut from pile				
13:14	Pile driving starts				
13:15	Pile driving stops for welding of next pile section				
13:17	Crane backs along timber mats toward BH102				
13:23	Fuel truck travels along timber mats past BH103 towards BH102				
13:24	Welding generator started				
13:27	Grinding of pile starts				
13:33	Grinding of pile stops				
13:33	Worker hammering on pile weld				

Table 7: Summary of Site Activities during Vibration Monitoring at Test Pile Site T42



Time	Activity
13:34	Welding stopped
13:38	Fuel truck leaves
13:53	Heavy hammering on pile with sledge
13:55	Lifting second pile section
13:59	Heavy hammering on pile with sledge
14:43	Welding completed
14:49	Pile driving starts
14:50	Pile driving slower progress (inferred harder material)
14:51	Pile driving stops
14:56	Pile hammered 10 times followed by pause
15:01	Pile hammered 10 times followed by pause
15:06	Pile hammered 10 times
16:08	Work concluded for day, monitoring systems shut down
17:21 to 17:28	Surface monitoring accelerometers rechecked using controlled source
March 31, 2017	(See Table 8, below, for activities at domestic water wells during pile driving)
9:48	Rechecked surface monitoring accelerometers with controlled source
12:20	All monitoring systems active
12:20 – 13:24	Welding, occasional hammering on steel platform
12:53	Pile hammered 17 times
12:55	Pile driving stops
13:05	Pile hammered 3 times
13:08	3.6 kg sledge hammer hit pile 10 times
13:13	3.6 kg sledge hammer hit pile 10 times
13:15	Crane shut down (quiet)
13:24	Monitoring system shut down

At the T42 site, during the top few metres of pile driving, the number of hammer blows required to drive the pile approximately 0.25 m ranged from 1 to 17, with an average of about 7 blows. Between the tip depths of 2 and 3 m, the number of hammer blows per 0.25 m of penetration dropped to a typical value of about 3. Below 4 m, only 2 to 3 hammer blows were required to drive the pile a full metre into the ground. Driving resistance then increased again near a depth of about 17 m to about 2 to 4 blows per 0.25 m of penetration, increasing to about 10 until the tip depth of 19 m was achieved. After 20 blows was required for 0.25 m of penetration, the pile driving was halted on March 29, 2017 at a tip depth of 18.92 m. On March 31, 2017 pile driving was restarted to complete pile driving energy measurements. Monitoring of the last increments of pile driving on March 31, 2017 was completed by EXP using the dynamic Pile Driving Analyzer (PDA). The final pile depth was recorded as only progressing an additional 2 mm during the last phase of driving (19 blows of the hammer). Based on the EXP data, the pile hammer delivered approximately 66 kJ to the pile top at a hammering rate of about 35 blows per minute. The pile penetrated to a





depth consistent with the top of bedrock in Golder borehole BH101 and within the glacial till, about 1 m above bedrock as identified in the AMEC (2016) borehole T42. Given the separation distances between these two boreholes, the pile could have reached refusal conditions on the top of bedrock or within the bottom 1 m of the glacial till unit immediately overlying the bedrock.

Time	Activity				
March 29, 2017	·				
12:05	Monitoring system active at Well #1, water pump running, water sampling in progress (by others)				
12:10	Departed Well #1 (25209 Baldoon Road)				
12:24	Monitoring system active at Well #2 (25345 Baldoon Road)				
12:27, 12:27:30	Car, car				
12:29	2 Golder vehicles leave Well #2 (Fry)				
12:32	Golder vehicle parked on shoulder near Well #1				
12:32:30	Water pump off, vehicle turned around in driveway at Well #1				
12:33, 12:33, 12:34	Light truck, light truck, light truck passed				
12:52	Check on Well #1 (approached on foot)				
12:59	Heard pile driving sounds				
13:14	SUV				
13:22, 13:23	Approached Well #2 by car, then by foot				
13:29	Light truck left Well #2 (owner)				
13:26	Golder vehicle left Well #2				
13:29	Golder vehicle entered Well #1 driveway and turned around				
13:38, 13:43, 13:45	Car, SUV, light truck				
13:59	Heard pile driving sounds				
14:34, 14:39, 14:40	Light truck, light truck, SUV				
14:41	School bus				
14:44	Approached Well #1 on foot to check status				
14:48, 14:50	SUV, car				
14:50	Heard pile driving sounds				
14:51, 14:57, 15:10 15:12, 15:14, 15:20	SUV, car, light truck, car, light truck, 2 cars passed in opposite directions				
15:21	Vehicle arrives at Well #1				
15:22	Golder vehicle arrives at Well #1				
15:23	Start of water sampling at Well #1 (by others)				
15:30	Car				

Table 8: Summary of Activities at Domestic Wells Vibration Monitoring, Wells #1 and #2



Time	Activity				
15:33	Completed water sampling at Well #1 (by others)				
15:35	Approach Well #1 on foot				
15:38	Stop recording at Well #1				
15:40 to 15:41	Recheck accelerometer with controlled source				
14:48	Approached Well #2 by car				
15:50	Approached Well #2 by foot				
15:51	Stop recording at Well #2				
15:52 to 15:53	Recheck accelerometer with controlled source				
March 31, 2017					
11:38	Arrived at Well #1, completed accelerometer recheck with controlled source				
11:44	Start recording at Well #1				
11:46	Plastic wind and water shield over Well #1 accelerometer to protect from heavy rain				
11:47	3 vehicles leave Well #1				
11:50	Arrived at Well #2, completed accelerometer recheck with controlled source				
11:57	Start recording at Well #2				
11:58	Plastic bag used as wind and water shield over Well #2 accelerometer to protect from heavy rain				
12:03	3 vehicles leave Well #2				
12:03	2 light trucks				
12:04	Golder vehicle turns in Well #1 driveway to reposition				
12:07	Approached Well #1 on foot to check status				
12:33	Pickup passed				
12:44, 12:51, 13:00 13:32	SUV, SUV, SUV, car				
13:32	3 vehicles arrive at Well #1				
13:34	Approached Well #1 on foot to check status				
13:34 to 13:53	Start of water sampling Well #1 ⁹ (by othes), outside garden tap and kitchen tap being sampled, water remained flowing until end of time period (13:53)				
13:53	Transport truck				
13:55	Stop recording Well #1				
13:58 to 13:59	Recheck of Well #1 accelerometer with controlled source				
14:04	3 vehicles approach Well #2				
14:05	Car				
14:04 to 14:16	Vehicle at Well #2				
14:16	Approached Well #2 on foot, removed plastic bag shield, removed accelerometer				

⁹ Field times among AECOM and Golder notes differ due to personal watches not being synchronized





Time	Activity
14:17	Stop recording Well #2
14:22 to 14:24	Recheck Well #2 accelerometer with controlled source

6.2.2 T5 Test Pile Site and Associated Domestic Water Wells

Vibration monitoring was carried out prior to, during and after pile driving on May 3 and 4, 2017. Activities on these dates are summarized in Table 9, below.

Table 9: Summary of Site Activities during Vibration Monitoring at Test Pile, Site T5

Time	Activity				
May 3, 2017 (Se	e Table 10 for activities at domestic water wells prior to and during pile driving)				
8:22	Arrived to set up Minimate systems, set unit clocks				
9:30	Surface monitoring systems ready for activating				
9:30	Pile crane preparation				
17:08	Minimate systems active				
17:49	Crane crawls toward test pile				
18:04	Hoisting pile				
18:14	Pile driving starts				
18:39	Pile reached refusal				
18:44	10 pile hammer blows				
18:46	10 hammer blows				
18:51	10 hammer blows				
18:52	Pile driving stops				
18:53	Crane off pile				
19:02 – 19:08	Minimate systems shut off and removed from site				
May 4, 2017 (Se	e Table 10 for activities at domestic water wells prior to and during pile driving)				
14:33 – 14:36	All monitoring systems active at T5 site				
14:40	Pile Dynamic Analyzer (PDA) being connected				
14:54	Crane moving to pile				
15:10	Hammer set on pile				
15:14	Pile driving, 16 blows for PDA test				
15:19	Hammer off pile				
15:29	5 hits with sledge hammer on pile				
15:31 – 15:34	Monitoring systems at T5 shut off				
16:08	All monitoring equipment removed from T5 site				

At the T5 site, during the top few metres of pile driving, the number of hammer blows required to drive the pile approximately 0.25 m ranged from 0 to 5. Between the tip depths of 3 and 4 m, the number of hammer blows per 0.25 m of penetration dropped to a typical value of about 1. Below 4 m, only 2 to 4 hammer blows were required to drive the pile a full metre into the ground. Driving resistance then increased again near a depth of about 15 m to about 2 blows per 0.25 m of penetration until the tip depth of 16.5 m was achieved. After 89 blows with little additional penetration, pile driving was halted on May 3, 2017 at a tip depth of 16.5 m. On May 4, 2017 pile driving was restarted to complete pile driving energy measurements. Monitoring of the last increments of pile driving on May 4, 2017 was completed by EXP using the dynamic Pile Driving Analyzer (PDA). The final pile depth was recorded as only progressing an additional 2 mm during the last phase of driving. The pile penetrated to within 0.3 m of the top of bedrock as identified in the AMEC (2016a) borehole T5.

lime	Activity				
May 3, 2017 (Se	ee Table , above				
11:03 – 11:38	On site at Well #3, checked accelerometers with controlled source and attached accelerometers to well casing				
11:38	Monitoring system active, placed protective bag over system				
11:39 – 11:45	Air compressor running in garage, running water to flush system for sampling (by others)				
11:45 – 11:49	Water samples being taken				
11:51	Removed protective bag from system, personnel walking near well and accelerometers				
11:56 – 12:00	6 vehicles left Well #3 site				
12:26	One vehicle returned to Well #3 to cover with plastic garbage can for protection against predicted heavy rain				
12:29	Departed Well #3				
12:33 – 12:59	Relocated to Well #4, checked accelerometers with controlled source and attached accelerometers to well casing				
13:35	Monitoring system active at Well #4				
13:45	Closed and locked well house door				
13:52	2 vehicles left Well #4				
17:30	Returned to Well #4, monitoring system still running, grass had been cut around well house, vehicle left at well site				
17:35	Returned to Well #3, Rion batteries had stopped functioning, replaced batteries, system active, heavy farm vehicles/equipment had been moved and parked near well				
17:35 onward	Relatively constant flow of car and truck traffic along Highway 40 near Well #4				
17:37, 17:48, 17:50	SUV, truck and car passing by Well #3 on Bush Line				
17:50	Grass being cut on Well #3 property				
17:51	Car turned into Well #3 driveway				
18:18	Heard pile driving sounds				

Table 10: Summary of Site Activities at Domestic Wells during Vibration Monitoring, Wells #3 and #4

Activity

Time





Time	Activity				
18:25, 18:26, 18:28, 18:29, 18:36, 18:41, 18:41, 19:02	2 cars, car, SUV, car, car, truck, car and car pass by Well #3 site on Bush Line at respective times noted.				
19:05 – 19:07	Returned to Well #4, monitoring systems stopped, data logger removed, accelerometers left in place on well casing, well house locked				
19:34	Returned to Well #3				
19:35	Golder staff member jumped up and down 3 times at 3 m from well				
19:37	Golder staff member jumped up and down 3 times at 2 m from well				
19:39	Golder staff member jumped up and down 3 times at 1 m from well				
19:40	Monitoring systems stopped, accelerometers removed and checked with controlled source				
May 4, 2017					
12:17 – 12:35	On site at Well #3, checked accelerometers with controlled source and attached accelerometers to well casing				
12:39 – 13:21	Relocated to Well #4 and reconnected data logger to accelerometers left in place on well casing, system active				
13:10	Vehicle arrived at Well #4				
14:38	Checked Well #4				
14:50	Observed relatively constant car and truck traffic on Highway 40				
14:50	Relocated to Well #3, monitoring system active				
14:50, 14:51, 14:52, 14:54, 14:56, 15:01, 15:14, 15:21, 15:25	SUV, large transport truck (Highway 40), light truck, school bus, light truck, car, heavy trucks (Highway 40), SUV, SUV passing Well #3 site on Bush Line at respective times noted.				
15:06	Light truck left Well #3 site				
15:21	Heard pile driving sounds				
15:29	Sledge hammering on pile				
15:32	3 vehicles arrive at Well #3				
15:48	Remove accelerometers at Well #3 and check with controlled source				
16:12 – 17:13	Return to Well #4, remove accelerometers and check with controlled source				



7.0 DATA SUMMARY

Data summarized in this report reflects examination of data files for background and peak vibration measurement parameters (e.g., particle velocity) at specific time intervals associated with the activities identified above. Individual data reports from Instantel Minimate systems for the time periods after which recording was triggered are included in Appendix H. The large volume of data collected by the accelerometers, however, is not conducive to such reporting and is therefore summarized in this report using a format commonly employed in vibration monitoring reports (e.g., Appendix H) along with modifications as described below to address the lower range of frequencies and amplitudes measured by the sensitive instruments used for this project.

General notes regarding data collection are provided below as context for the overall data summary:

- Data was not captured appropriately by the Spider20 data logging equipment at Well #2 during the T42 test pile monitoring program and, therefore, no data was available for analysis. However, this was the more distant of the two wells associated with the T42 site. Data was captured for the closer Well #1 associated with the T42 site and both Wells #3 and #4 associated with the T5 test pile site.
- During test pile driving at the T42 site, the closest uniaxial accelerometer mounted in the bedrock (101DU) was overloaded (internal amplifier voltage saturation) on multiple pile strikes to the point where the data from these events could not be used. The uniaxial accelerometer was not overloaded to the point of damage based on all other vibration data, the manufacturer's specifications and technical notes and since this accelerometer performed successfully after the overload events. These events occurred during the last 30 pile strikes on March 31, 2017 and several strikes during the PDA testing on March 31, 2017.
- The triaxial accelerometer installed within the bedrock at borehole BH-103 (103DTX, 103DTY, 103DTZ) experienced electrical signal interference on March 29, 2017, expected to have been the result of supply voltage and connections that was only observable after data processing. Based on examination of data from March 23, 29, and 31, 2017 and comparisons to data from other accelerometers, data from this triaxial accelerometer collected on March 29, 2017 was not used while data from March 31, 2017 was considered suitable.
- Based on examination of all data collection events, the low-level geophone used for the Instantel Minimate located 50 m from the T5 test pile did not capture data appropriately on May 4, 2017, though data from the previous day, during times when pile driving was not occurring, when pile driving through soil and when pile driving with the tip on hard glacial till or bedrock was of high quality and clearly identified the effects of pile driving.

7.1 Data Evaluation Methods

Data gathered by the Instantel Minimates was processed using the Blastmate software. While the geophones measure velocity, the determination of the frequency of a complex vibration waveform requires mathematical approximations or simplifications. The Instantel system uses the "zero crossing" approach to calculate vibration frequencies that correspond approximately to the peak particle velocities of a given waveform. In this approach, the time for one half of a complete cycle is measured as the time between two successive points at which the vibration wave crosses the zero amplitude time axis. This zero crossing frequency calculation is somewhat limited because it assumes a single predominant frequency at the peak particle velocity as might be expected with a





simple sine wave pattern. With complex vibrations, the peak can be the result of multiple superposed frequency components. This limitation, however, is recognised in monitoring of construction-related vibrations as an acceptable means for practically addressing the complex waveform analysis and reporting. Geophone data is provided in Appendix H.

Data generated by the accelerometers required processing for the following reasons:

- high-precision, low-noise accelerometers were chosen for much of the monitoring since, for the physical size, frequency and amplitude ranges necessary for this work, available accelerometers were more sensitive than most geophone systems;
- background vibrations and other vibration noise (e.g., generated by nearby road traffic, other equipment operating at the ground surface, environmental influences, etc.) and system electronic noise (e.g., voltage variability of power generators, signals outside of rated frequency ranges, etc.) must be considered apart from the primary frequency source (pile driving) to arrive at appropriate measurements of related particle velocities;
- pile driving using an impact hammer produces discrete, transient, or discontinuous, complex waveforms (i.e., not continuous, repeating sinusoidal waves); and
- acceleration data must be mathematically integrated to obtain corresponding velocity data.

Data reduction and analysis was carried out using the MATLAB¹⁰ Version 2017A software along with the Signal Processing Toolbox. Acceleration time histories for each accelerometer were used to relate specific activities to instrument responses.

Figure 4 illustrates acceleration time history data from various accelerometers during different phases of pile driving. An approximately 2 minute period of data is shown for accelerometer 101DU (10 m from pile, in bedrock) during the approximately 2 minute period of pile driving on March 29, 2017 between 14:49 and 14:51, prior to the last three groups of 10 hammer blows. This time history clearly illustrates accelerometer responses to pile driving actions during this period including reactions to initial upstrokes (diesel firing), ram strikes and hammer rebounds. Figure 4 also shows a detailed view of the acceleration time history during the second to last full hammer strike at accelerometer locations 101DU, 102DU, 103 DU (at 10, 30 and 50 m within the bedrock) and at Well #1 on March 31, 2017. On this figure, the additional detail shown for accelerometer 103DU is associated with the increased sampling rate. Figure 4 also clearly illustrates diminishing acceleration responses with increasing distance from the test pile. Further, for the more detailed time interval shown for the multiple accelerometers, the data show a very small phase or time lag as the distance between the test pile and accelerometers increases, as the vibration wave front travels through the bedrock.

A Fast-Fourier Transform (FFT) algorithm was used to convert data from the time domain into the frequency domain with the FFT data subsequently integrated to velocity. Based on examination of background measurements obtained at the T42 test pile site on March 23, 2017 and at the Well #1 site on March 28, 2017, when no other activities were occurring, the data clearly indicated background noise and the influence of transforming the time domain acceleration data into the frequency domain at and below about 5 to 10 Hz. Figure 5 illustrates unfiltered data obtained from one accelerometer (103DTX) for 75 one-second periods at the site during



¹⁰ MATLAB 2017a (2017). MATLAB, 1 Apple Hill Drive, Natick, MA 01760-2098; https://www.mathworks.com/products/matlab.html

which pile driving was not occurring (March 23, 2017) after FFT processing and integration to velocity. These onesecond durations were individually selected to coincide with the directly observable start of individual pile driving impulses within the acceleration time histories. A known characteristic of the FFT process is that for transient or non-continuous signals sampled in finite time intervals (seconds, minutes, hours, etc.), the underlying mathematics for defining time-continuous (infinite) periodic cycles result in a phenomenon termed "leakage" where signal energy spreads to other frequencies within the signal spectra. In this case, the leakage and FFT process influences the character of low frequency signals as presented in FFT graphs. Therefore, subsequent to FFT and velocity integration for each pile striking event (7 strikes when driving through the top few metres of soil, and 94 strikes from the time at which difficult pile driving began), the values of each peak within the velocity spectrum for each pile strike event were chosen using the "findpeaks" function in MATLAB. This function identifies the local peaks of input signal vectors where a local peak is defined as a data point that is either larger than its two neighboring points or is equal to infinity and signal endpoints are excluded. This approach limited the potential influence of various filtering algorithms and signal power suppression that might otherwise be used to address FFT spectra noise and leakage while at the same time addressing the FFT results that appear in the low frequency range below 5 Hz. Verification of Parseval's theorem was also completed to ensure that the recorded energy in the time domain was the same as the energy in the frequency domain. Data obtained from the well locations was also processed similarly where specific events and time intervals when test pile driving was on-going and during other deliberate or day-to-day activities were occurring. For data processing at the well sites, in some cases, longer periods of the acceleration time histories were aggregated for subsequent FFT and velocity evaluation because of the long periods during which pile driving was not occurring yet the wells were subject to other background conditions (i.e., property-specific traffic, road traffic and deliberate site-specific comparison actions).

The data processing and reduction process described above is illustrated by the examples shown in Figure 5. The data shown in Figure 5 were collected at the location most subject to influence from background noise because of the small amplitude accelerations measured 50 m from test pile driving. Similar FFT results were observed with all accelerometers for times when pile driving was not in progress. Clearly identifiable pile strike events and ground responses were examined individually (see examples shown on Figure 4). Figure 5 also illustrates integrated FFT velocity data from 18 pile strike events (when driving the pile on till/rock) readily discernable from the acceleration time history. Subsequent to FFT and velocity integration for each pile striking event (7 strikes when driving through soil, and 94 strikes from the time at which difficult pile driving began through to refusal and PDA testing), the values of each peak within the velocity spectrum for each pile strike event were identified. Then, the maximum of each of these peaks was selected from all accelerometer FFT data for individual pile strike events, resulting in more than 2,000 maximum particle velocity and corresponding frequency data pairs to characterise ground vibrations when driving the test pile through soil and rock. Additional examples of data analyses are included in Appendix I illustrating acceleration time histories, FFT and resulting velocity spectra.

7.2 Data Summary

Subsurface measurements obtained at the T42 test pile site on March 23, 2017 prior to any test pile driving indicated background vibrations of about 0.003 mm/s when no construction vehicles were operating on site, weather conditions were optimal and in the absence of nearby traffic. At the well sites, measurements obtained when the test piles were not being driven indicated a range of background vibrations from other causes including





traffic, environmental conditions and other activities on the well sites, exclusive of activities specifically undertaken to deliberately induce vibrations, typically ranged from about 0.010 to 0.035 mm/s.

Figures 6A, 6B, 7A and 7B illustrate peak particle velocities measured by the Instantel Minimate systems within the T42 site during the periods of pile driving on March 29 and 31, 2017. These figures compare all data collected to typical reporting scales and thresholds (e.g., BlastMate software as used by Instantel as in Appendix H). Figures 6A through 7B also expand the measurement scales by two orders of magnitude lower in amplitude to address the range of measurements from vibration data summarized in subsequent graphs.

All data collected during all pile driving events from the close-proximity Instantel Minimate device at 3 m from the T42 test pile is illustrated on Figure 6A. Figure 6B shows only the data collected when the pile was nearing and at its termination depth in the glacial till or on bedrock (from March 29 and 31, 2017). Similarly, for the Minimate geophone system located 10 m from the test pile, data collected from this instrument during all pile driving events is illustrated on Figure 7A. Figure 7B shows only the data collected when the pile was near or at its termination depth on till or bedrock (from March 29 and 31, 2017).

Particle velocities and frequencies measured by the surface and subsurface accelerometers at the borehole pairs at the T42 test pile site are illustrated in Figures 8 to 10 as follows:

- Borehole pair BH101/101A, located 10 m from test pile T42:
 - Figure 8A surface and subsurface vibrations in soil (mid-depth accelerometers) during pile driving through the top 2 m of soil;
 - Figure 8B subsurface vibrations in rock during pile driving through the top 2 m of soil;
 - Figure 8C surface and subsurface vibrations in soil (mid-depth accelerometers) during hard pile driving at the till/rock pile termination depth;
 - Figure 8D subsurface vibrations in rock during hard pile driving at the till/rock pile termination depth.
- Borehole pair BH102/102A, located 30 m from test pile T42:
 - Figure 9A surface and subsurface vibrations in soil (mid-depth accelerometers) during pile driving through the top 2 m of soil;
 - Figure 9B subsurface vibrations in rock during pile driving through the top 2 m of soil;
 - Figure 9C surface and subsurface vibrations in soil (mid-depth accelerometers) during hard pile driving at the till/rock pile termination depth;
 - Figure 9D subsurface vibrations in rock during hard pile driving at the till/rock pile termination depth.
- Borehole pair BH103/103A, located 50 m from test pile T42:
 - Figure 10A surface and subsurface vibrations in soil (mid-depth accelerometers) during pile driving through the top 2 m of soil;
 - Figure 10B subsurface vibrations in rock during pile driving through the top 2 m of soil;





- Figure 10C surface and subsurface vibrations in soil (mid-depth accelerometers) during hard pile driving at the till/rock pile termination depth;
- Figure 10D subsurface vibrations in rock during hard pile driving at the till/rock pile termination depth.

Figures 10B and 10D include an inset graph to illustrate data magnitudes below the lower bound of the velocity axis as illustrated on Figures 6A through 10A and 10C.

Figures 11A through 14B summarize data from the Instantel Minimates obtained during pile driving at the T5 site. As above, Figures 11A, 12A, 13A and 14A illustrate all measurements at each of the monitoring locations at 3, 10, 30 and 50 m from the test pile whereas Figures 11B, 12B, 13B and 14B illustrate measurements only when the pile was being driven on the till/rock. Data illustrated for the low-level geophone, located 50 m from the test pile at site T5, is illustrated using histograms since, for the long-duration monitoring event, the low-level geophone software as implemented generates output only showing the triaxial peak particle velocity measurements without corresponding zero-cross frequencies for the recording period.

	Distance from	Particle Velocity (mm/s)				
Location	Test Pile (m)	Vertical	Transverse	Longitudinal 31.7		
Close Proximity	3	38.1	7.2			
BH-101/101A						
Surface	40(40)11	7.2	3.3	10.4		
Soil Mid-Depth		0.36	0.51	0.21		
Rock		0.13	0.04	0.04		
BH-102/102A						
Surface		0.21	N/A	N/A		
Soil Mid-Depth	- 30	0.27	0.15	0.20		
Rock		0.03	0.04	0.02		
BH-103/103A				1		
Surface		0.18	N/A ¹²	N/A		
Soil Mid-Depth	- 50	0.15	N/A	N/A		
Rock		0.04	0.00	0.06		

Table 11: Summar	of Particla	Valacities	durina Dila	Driving H	brough Top	2 m at T/2 Sita
Table 11: Summary		velocities	uuring File	Driving u	mougn rop	2 III al 142 Sile

¹² N/A indicates "Not Applicable" since vibrations in the noted directions were not instrumented or measured. See Section 5 for additional details.



¹¹ Sensors 101SU, 101STX, 101STY and 101STZ were installed in the shallow borehole off-set 2 m further distant from the test pile as described in the report text.

	Distance from	Particle Velocity (mm/s)				
Location	Test Pile (m)	Vertical	Transverse	Longitudinal		
Close Proximity	3	11.0	7.2			
BH-101/101A						
Surface	10/12)13	2.9	2.2	3.2		
Soil Mid-Depth		6.38	13.28	12.90		
Rock		13.24	8.76	7.33		
BH-102/102A						
Surface	20	0.85	N/A	N/A		
Soil Mid-Depth	- 30 -	8.65	4.98	7.84		
Rock		5.12	0.45	2.54		
BH-103/103A						
Surface	50	0.12	N/A	N/A		
Soil Mid-Depth	- 50 -	0.10	N/A	N/A		
Rock		0.01	0.00	0.01		

Table 12: Summary of Particle Velocities during Pile Driving at Till/Rock at T42 Site

Table 13: Summary of Particle Velocities during Pile Driving through Soil at T5 Site

Location	Distance from Test Pile (m)	Particle Velocity (mm/s)		
		Vertical	Transverse	Longitudinal
Surface	3	23.8	12.3	20.1
Surface	10	2.5	5.7	10.0
Surface	30	0.8	0.6	0.9
Surface	50	1.4	0.5	1.6

Table 14: Summary of Particle Velocities at Ground Surface during Pile Driving at Till/Rock at T5 Site

Location	Distance from Test Pile (m)	Particle Velocity (mm/s)			
		Vertical	Transverse	Longitudinal	
Surface	3	11.3	12.3	11.1	
Surface	10	2.5	2.9	5.1	
Surface	30	0.8	0.6	0.9	
Surface	50	0.7	0.4	0.7	

Data collected at Well #1 is summarized in Table 15, below, for time periods on March 28, 29 and 31, 2017 when pile driving was not occurring ("quiet") and when pile driving was occurring on March 29 and 31, 2017. During the

¹³ Sensors 101SU, 101STX, 101STY and 101STZ were installed in the shallow borehole off-set 2 m further distant from the test pile as described in the report text.



"guite" periods, the only other activities occurring within the near vicinity of the accelerometer at Well #1 consisted of traffic on Baldoon Road, people walking carefully near the accelerometer data logger to check on its status and environmental influences (e.g., wind/air pressures). On March 28, as indicated in Table 15, deliberate activities were also undertaken to examine their effect on the readings and to provide a basis for data comparison. These activities included jumping near the well, finger tapping on the well lid, driving a vehicle in the driveway, opening and closing vehicle doors and placing a vibrating cell phone on the well lid among others. These data collected on March 28, 29 and 31, 2017 during guiet periods is virtually identical to data during pile driving. Data collected at Wells #3 and #4 is also summarized in Table 15, below, for time periods on May 3 and 4, 2017 when pile driving was not occurring ("quiet") and when pile driving was occurring on these same dates. During the times when pile driving was not occurring, other activities occurred on the Well #3 and #4 properties including movement of farm equipment, lawn mowing with ride-on equipment, operating well pumps, traffic on St. Clair Road and Bush Line, typical light vehicles entering and exiting the sites and foot traffic in the vicinity. Well #3 was the monitored well closest to a test pile site. The accelerometers at this well captured pile driving induced accelerations during part of the pile driving period when the data was not otherwise obscured by the influence of other activities on the well site, background vibration noise and signal noise. Appendix I includes graphical summaries of data evaluation for this relevant time period along with examples of other conditions when test pile T5 was not being driven. These data illustrate the relative and inconsequential magnitudes of pile driving-induced vibrations as compared to the influences of common conditions otherwise.

Particle Velocity ¹⁴ (mm/s)	Relevant Activity	
March 28, 2017, Well #1		
0.01	Quiet period, no activities	
0.018 (0.0182)	Golder employee jumped 3 times at 0.9 m from well (see Appendix I)	
0.018 (0.0178)	Golder employee jumped 3 times at 1.8 m from well (see Appendix I)	
0.018 (0.0178)	Golder employee jumped 3 times at 2.7 m from well (see Appendix I)	
0.012	Golder employee walked toward well (a composite set of footfalls)	
0.002	Golder employee walked toward well (see Appendix I)	
0.083	Cell phone on well lid ringing in vibrating mode (see Appendix I)	
0.016	Driving vehicle up and down driveway (see Appendix I)	
0.018	Climbed steps, knocked on door and descended steps (see Appendix I)	
March 29, 2017, Well #1		
0.037	No pile driving	
0.031	Pile driving time period	
March 31, 2017, Well #1		
0.025	No pile driving	
0.017	Pile driving time period	

¹⁴ Values indicated in table are maximum measured values for vertical, longitudinal and transverse directions. Values have been rounded to 3 decimal places except where shown in parentheses to provide additional detail.





Particle Velocity ¹⁴ (mm/s)	Relevant Activity	
May 3, 2017, Well #3		
0.003 to 0.005	No pile driving	
0.009 to 0.014	No pile driving	
0.003	Pile driving time period (see Appendix I)	
0.013	Golder staff member jumped up and down 3 times at 1 m from well (see Appendix I)	
May 4, 2017, Well #3		
0.003 to 0.074	No pile driving, other activities appear to have occurred on site based on acceleration time history data	
0.028 to 0.031	Pile driving time period	
May 4, 2017, Well #4		
0.008 to 0.021	No pile driving	
0.006 to 0.014	Pile driving time period	





8.0 DATA INTERPRETATION

Ground surface and subsurface vibrations were measured at the T42 site. Ground surface vibrations were monitored at the T5 site. Vibrations of domestic water wells were successfully measured at three well locations with one of these near the T42 site and two near the T5 site. These domestic water wells were located at distances ranging from about 911 to about 1.1 km from the test pile sites.

Ground surface vibrations measured at the T42 and T5 sites were similar when driving the closed-end pipe piles through the first few metres of soil and when driving the piles within the glacial till or on the bedrock surface. When driving through the first few metres of soil, vibrations at the T42 site were greater than those at the T5 site. For example, the maximum vertical vibration amplitude measured at 3 m distant from the pile at the T42 and T5 sites were about 38 mm/s (Table 11) and about 24 mm/s (Table 13), respectively. The difference in ground surface vibration amplitudes can be attributed to the softer or looser soil conditions within the top few metres of soil at the T5 site as compared to the T42 site. The soil conditions at the T5 site required less energy for pipe pile penetration and, therefore, the ground surface vibration magnitudes were smaller than at the T42 site when driving through the top few metres of soil. Based on Golder's review of the borehole records as completed by AMEC, the ground surface vibration amplitudes summarized within Tables 11 and 13 for pile driving through the top few metres of soil are expected to be typical throughout most of the NK1 project site. In some NK1 project areas, the nearsurface soils are expected to be stiffer or denser than at the T42 and T5 sites. At such locations, ground surface vibrations when driving through the top few metres of soil might be larger than those experienced at the T42 and T5 sites. While differences in ground surface vibration measurements are expected when driving through the first few metres of soil, these differences in vibration magnitude have little effect on the bedrock at the turbine site as illustrated by a comparison of Tables 11 and 12 for the vibration magnitudes in bedrock when the pile was being driven in the soil.

Data from the T42 site demonstrate that driving of the closed-end pipe piles on or near the bedrock surface represented the worst conditions with respect to vibrations of any substantive magnitudes measured within the bedrock. Maximum ground surface vibrations, near the test pile, when driving the closed-end pipe piles near or on the bedrock surface were not substantively different at the two test sites with measurements of about 11 to 15 mm/s at the T42 site and about 11 to 12 mm/s at the T5 site. Differences between ground surface vibration magnitudes measured at the ground surface when pile driving at or near the bedrock surface at the T42 and T5 sites were not substantive and are expected to be representative of conditions throughout the NK1 project area. The similarity in ground surface vibration data from these two sites illustrate the effects of the overlying soil mass on ground surface vibrations when the piles experienced refusal to further penetration and were subject to the largest pile driving energies during the test pile installation. For the remaining pile driving, ground surface vibrations are expected to be similar to those measured at both the T5 and T42 sites when driving the piles on or near the bedrock surface.

The site-specific data is also compared to earlier estimates (Golder 2016) based on published research and engineering practices (e.g., CALTRANS 2004). Maximum velocities, regardless of direction, measured at the ground surface by the Minimates and accelerometers during all pile driving activities at test pile sites T42 and T5 sites are illustrated on Figure 15. Maximum values for velocities, regardless of direction, measured by the accelerometers installed in the rock during intervals of pile driving in the glacial till/on rock are also illustrated on Figure 15. The site-specific data clearly illustrates the attenuation of particle velocity with increasing distance from the test pile.



As noted above, all data from the monitoring of Wells #1, #3 and #4 indicate that the peak particle velocity was less than 0.035 mm/s during pile driving. At times when pile driving was not occurring, the maximum particle velocity was nearly double this value as related to other activities on and near the monitored wells. As compared to typical vibration magnitudes associated with other causes or vibration thresholds for different conditions, as illustrated in Table 16 and Table 17, below, the vibrations generated by pile driving are expected to be significantly below the threshold of human perception for transient vibrations at domestic water supply wells located several hundred metres or more from pile driving. The data further indicate that the character of vibrations at Wells #1, #3 and #4 as measured by the sensitive instrumentation were similar to or less than those induced by common day-to-day activities (e.g., traffic, walking nearby, farming equipment and light vehicles) and by walking or jumping near the well casing.

Table 16: Examples of the effects, thresholds or conditions associated with small magnitude ground vibrations¹⁵

PPV (mm/s)	Effect or Condition				
2 - 30	Pile driving in soft ground at 1 to 3 m from hammer using vibratory and impact hammers				
23	Close-proximity nail driving in residential structure				
2.5 – 12	Equates to normal daily family activity within residential structure				
3 – 9	Vibration limits for pumps ranging from 10 hp to 3000 hp				
7.6	Equivalent to jumping on floor of residential structure				
6	Transient vibrations distinctly perceptible				
5	Steady-state vibrations annoying				
2.5	Truck traffic on bumpy road at 16 m Large bulldozer at 7.6 m				
2.3					
1.9	Loaded trucks at 7.6 m				
1	Steady-state vibrations readily perceptible				
1	Transient vibrations barely perceptible				
0.8	Steady-state threshold for workshop				
0.8	Equivalent to walking on floor of residential structure				
0.8	Small bulldozer at 7.6 m				
0.76	Vehicle traffic at 16 m				
0.15 – 0.5	Steady-state vibrations threshold of perception				
0.4	Steady-state threshold for office				
0.3	Steady-state vibrations slightly perceptible				
0.2	Steady-state threshold for residence				
0.1	Steady-state threshold for hospital operating room				
0.025	Quiet background				



¹⁵ See references at conclusion of report text for cited vibration magnitudes, their causes and other thresholds.

Applicable Clause	Observation Period	Limit on the Average Peak Vibration Velocity (mm/s)					
		Daytime	Nighttime				
Vibration Limits for Frequent Impulses							
4(4)(b)	≤20 minutes	0.3	0.3				
4(4)(c)(i)	$20 < period \le 60 minutes$	0.6	0.3				
4(4)(c)(ii)	$60 < period \le 120 minutes$	1.0	0.3				
Vibration Limits for Infrequent Impulses							
4(4)(c)(iii)	120 minutes	10.0	0.3				

Table 17: Vibration Limits for Human Perception as Defined in Tables NPC-207-2 and NPC-207-3

Water quality testing was completed by AECOM (2017a and 2017b) on three occasions at Wells #1 and #2, identified by AECOM as wells PW1 and PW2, respectively and Well #3 (noted as MW2 by AECOM). These water quality testing events were identified by AECOM as "baseline", "pre-pile" and "post-pile" events. As related to the T42 test pile, AECOM concluded that all indicator parameter concentrations were within an acceptable range compared to baseline and pre-test conditions, with the exception of turbidity at Well #2 (PW-2). Elevated colour and turbidity values at this well were concluded to have potentially been the result of changes in well use and/or climatic conditions affecting water quality that could have occurred since the time of baseline sampling completed more than 2 months prior to the test pile and not the test pile driving. AECOM noted that Well #2 (PW-2) was a further distance to the test pile location as compared to Well #1 (PW-1) where turbidity and other water quality parameter concentrations were considered by AECOM to be stable. Related to the T5 test pile monitoring, evaluations of water quality data completed by AECOM concluded that water quality indicator parameters were within acceptable ranges as compared to baseline and pre-pile conditions, with the exception of colour and turbidity. AECOM concluded that elevated colour values could be a result of changes in well use and/or climatic conditions affecting water quality considering that the baseline and pre-test pile values show a larger percent difference compared to the pre-pile and post-pile results. Further, the turbidity concentrations at MW1 were found by AECOM to exceed the referenced water quality standards at the time of all sampling events (baseline, pre-pile and post-pile), while concentrations at MW2 (Well #3) remained below the water quality standards at the time of all sampling events. Elevated post-pile turbidity concentrations at MW1 were judged to have potentially been result of poor well construction and weather conditions and not related to the test pile driving. Because of well construction details, the well identified as MW1 by AECOM was not suitable for monitoring vibrations. The conclusion of AECOM that the test pile driving did not adversely affect water quality at the monitored wells is consistent with analysis and interpretation of the monitoring data that vibrations at the well locations were inconsequential.



9.0 CONCLUSIONS

Surface and subsurface vibration monitoring was completed at the NK1 turbine T42 and T5 sites prior to and during installation of test piles. Vibration data was collected at multiple distances from test pile T42 at the ground surface, within the soft silty clay soils (at about mid-depth) and within the bedrock near the glacial till/bedrock interface. Ground surface vibration measurements were also obtained at the T5 test pile site at multiple distances from the pile. Vibrations of various magnitudes were detected by all monitoring instruments. The resulting data has permitted an evaluation of distance-vibration magnitudes associated with other causes or published acceptable vibration thresholds for different activities and conditions, and as compared to background vibrations and deliberate actions to simulate day-to-day activities near the well locations, vibrations at the residential well locations during pile driving were below regulatory values for human perception of transient vibrations, other published thresholds related to residential uses and below commonly occurring background levels. It is our opinion that pile driving will not expose distant groundwater wells (e.g., more than 550 m) to vibrations in excess of those that the wells commonly experience otherwise. The conclusion of AECOM that the test pile driving did not adversely affect water quality at the monitored wells is consistent with analysis and interpretation of the monitoring data that vibrations at the well locations were inconsequential.

GOLDER ASSOCIATES LTD.

Jordon Tim

Jordan Kiss, M.E.Sc., E.I.T.

Danny da Silva, P.Eng. Principal

JK/SJB/JT/DD/cr

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Storer J. Boone, Ph.D., P.Eng. Principal

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Basis and Use of the Report: This report has been prepared for the specific site, design objective, development and purpose described to Golder by the Client. The factual data, interpretations and recommendations pertain to a specific project as described in this report and are not applicable to any other project or site location. Any change of site conditions, purpose, development plans or if the project is not initiated within eighteen months of the date of the report may alter the validity of the report. Golder can not be responsible for use of this report, or portions thereof, unless Golder is requested to review and, if necessary, revise the report.

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Unless otherwise stated, the suggestions, recommendations and opinions given in this report are intended only for the guidance of the Client in the design of the specific project. The extent and detail of investigations, including the number of test holes, necessary to determine all of the relevant conditions which may affect construction costs would normally be greater than has been carried out for design purposes. Contractors bidding on, or undertaking the work, should rely on their own investigations, as well as their own interpretations of the factual data presented in the report, as to how subsurface conditions may affect their work, including but not limited to proposed construction techniques, schedule, safety and equipment capabilities.

Soil, Rock and Groundwater Conditions: Classification and identification of soils, rocks, and geologic units have been based on commonly accepted methods employed in the practice of geotechnical engineering and related disciplines. Classification and identification of the type and condition of these materials or units involves judgment, and boundaries between different soil, rock or geologic types or units may be transitional rather than abrupt. Accordingly, Golder does not warrant or guarantee the exactness of the descriptions.



IMPORTANT INFORMATION AND LIMITATIONS OF THIS REPORT

Special risks occur whenever engineering or related disciplines are applied to identify subsurface conditions and even a comprehensive investigation, sampling and testing program may fail to detect all or certain subsurface conditions. The environmental, geologic, geotechnical, geochemical and hydrogeologic conditions that Golder interprets to exist between and beyond sampling points may differ from those that actually exist. In addition to soil variability, fill of variable physical and chemical composition can be present over portions of the site or on adjacent properties. The professional services retained for this project include only the geotechnical aspects of the subsurface conditions at the site, unless otherwise specifically stated and identified in the report. The presence or implication(s) of possible surface and/or subsurface contamination resulting from previous activities or uses of the site and/or resulting from the introduction onto the site of materials from off-site sources are outside the terms of reference for this project and have not been investigated or addressed.

Soil and groundwater conditions shown in the factual data and described in the report are the observed conditions at the time of their determination or measurement. Unless otherwise noted, those conditions form the basis of the recommendations in the report. Groundwater conditions may vary between and beyond reported locations and can be affected by annual, seasonal and meteorological conditions. The condition of the soil, rock and groundwater may be significantly altered by construction activities (traffic, excavation, groundwater level lowering, pile driving, blasting, etc.) on the site or on adjacent sites. Excavation may expose the soils to changes due to wetting, drying or frost. Unless otherwise indicated the soil must be protected from these changes during construction.

Sample Disposal: Golder will dispose of all uncontaminated soil and/or rock samples 90 days following issue of this report or, upon written request of the Client, will store uncontaminated samples and materials at the Client's expense. In the event that actual contaminated soils, fills or groundwater are encountered or are inferred to be present, all contaminated samples shall remain the property and responsibility of the Client for proper disposal.

Follow-Up and Construction Services: All details of the design were not known at the time of submission of Golder's report. Golder should be retained to review the final design, project plans and documents prior to construction, to confirm that they are consistent with the intent of Golder's report.

During construction, Golder should be retained to perform sufficient and timely observations of encountered conditions to confirm and document that the subsurface conditions do not materially differ from those interpreted conditions considered in the preparation of Golder's report and to confirm and document that construction activities do not adversely affect the suggestions, recommendations and opinions contained in Golder's report. Adequate field review, observation and testing during construction are necessary for Golder to be able to provide letters of assurance, in accordance with the requirements of many regulatory authorities. In cases where this recommendation is not followed, Golder's responsibility is limited to interpreting accurately the information encountered at the borehole locations, at the time of their initial determination or measurement during the preparation of the Report.

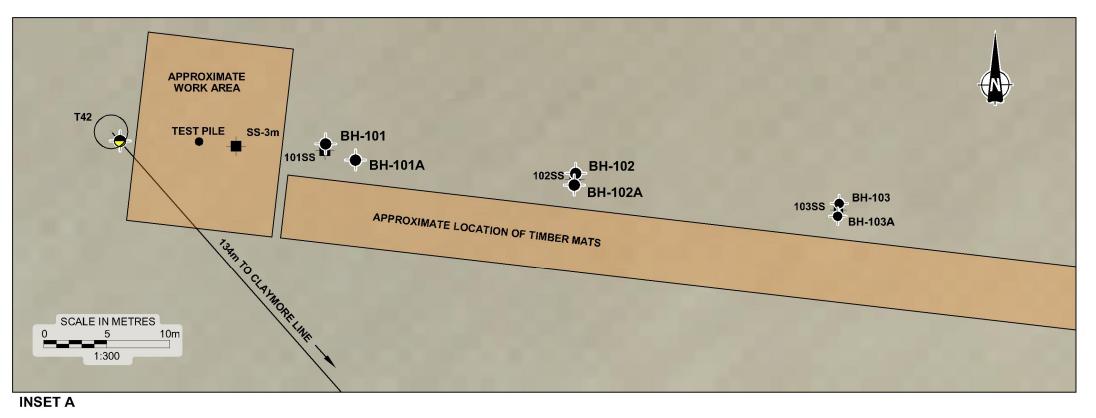
Changed Conditions and Drainage: Where conditions encountered at the site differ significantly from those anticipated in this report, either due to natural variability of subsurface conditions or construction activities, it is a condition of this report that Golder be notified of any changes and be provided with an opportunity to review or revise the recommendations within this report. Recognition of changed soil and rock conditions requires experience and it is recommended that Golder be employed to visit the site with sufficient frequency to detect if conditions have changed significantly.

Drainage of subsurface water is commonly required either for temporary or permanent installations for the project. Improper design or construction of drainage or dewatering can have serious consequences. Golder takes no responsibility for the effects of drainage unless specifically involved in the detailed design and construction monitoring of the system.









LEGEND

• BOREHOLE



SURFACE VIBRATION SENSOR

BOREHOLE (AMEC)

WATER WELL

REFERENCE

DRAWING BASED ON 2010 AERIAL IMAGERY PROVIDED BY THE MUNICIPALITY OF CHATHAM-KENT, INCLUDES MATERIAL c 2015 OF THE QUEEN'S PRINTER FOR ONTARIO; AND "FOUNDATION PLAN", ENTUITIVE, PROJECT No. C017-0190, DWG No. S002.

NOTES

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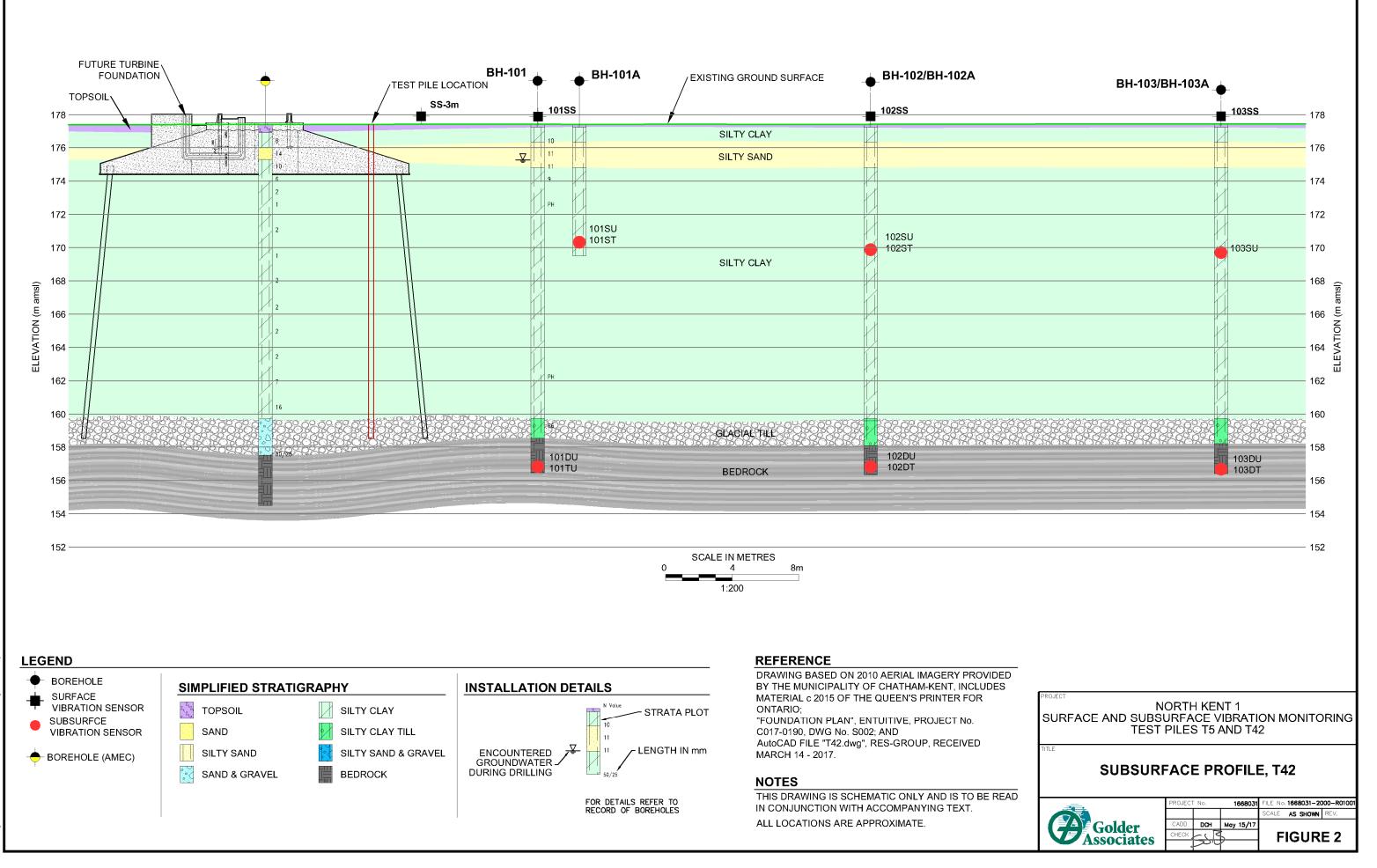
PROJECT

NORTH KENT 1 SURFACE AND SUBSURFACE VIBRATION MONITORING TEST PILES T5 AND T42

SITE AND INSTRUMENT LOCATION PLAN, T42

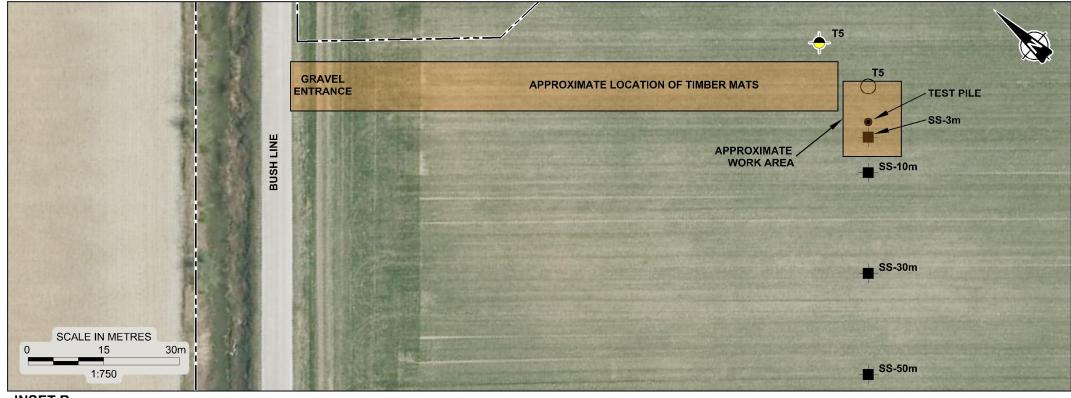


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			SCALE AS SHOWN	REV.	
CADD	DCH	May 12/17			
CHECK		×	FIGURE 1		
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LEGEND

SURFACE VIBRATION SENSOR

BOREHOLE (AMEC)

WATER WELL

REFERENCE

DRAWING BASED ON 2010 AERIAL IMAGERY PROVIDED BY THE MUNICIPALITY OF CHATHAM-KENT, INCLUDES MATERIAL c 2015 OF THE QUEEN'S PRINTER FOR ONTARIO; AND "FOUNDATION PLAN", ENTUITIVE, PROJECT No. C017-0190, DWG No. S002.

NOTES

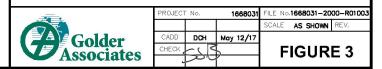
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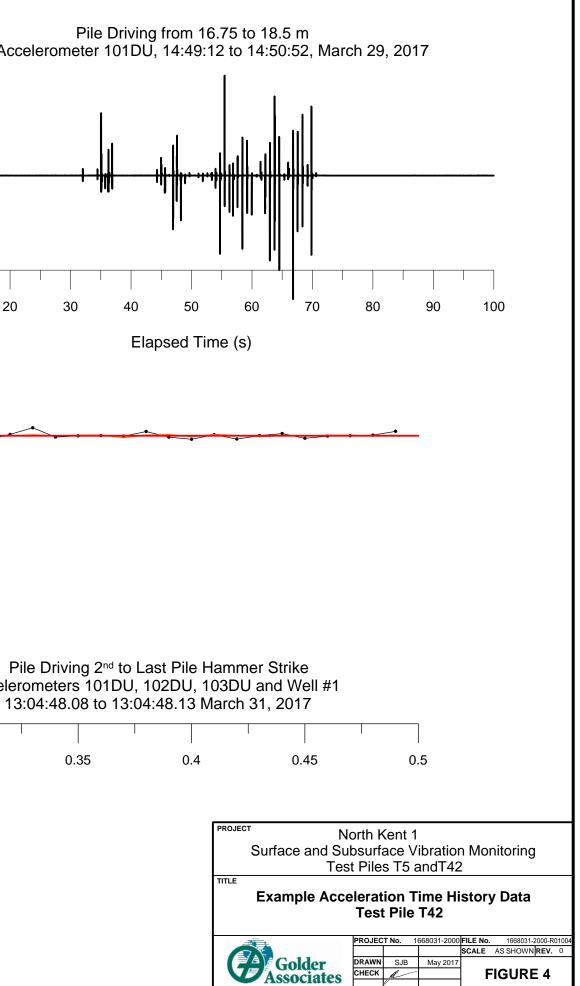
PROJECT

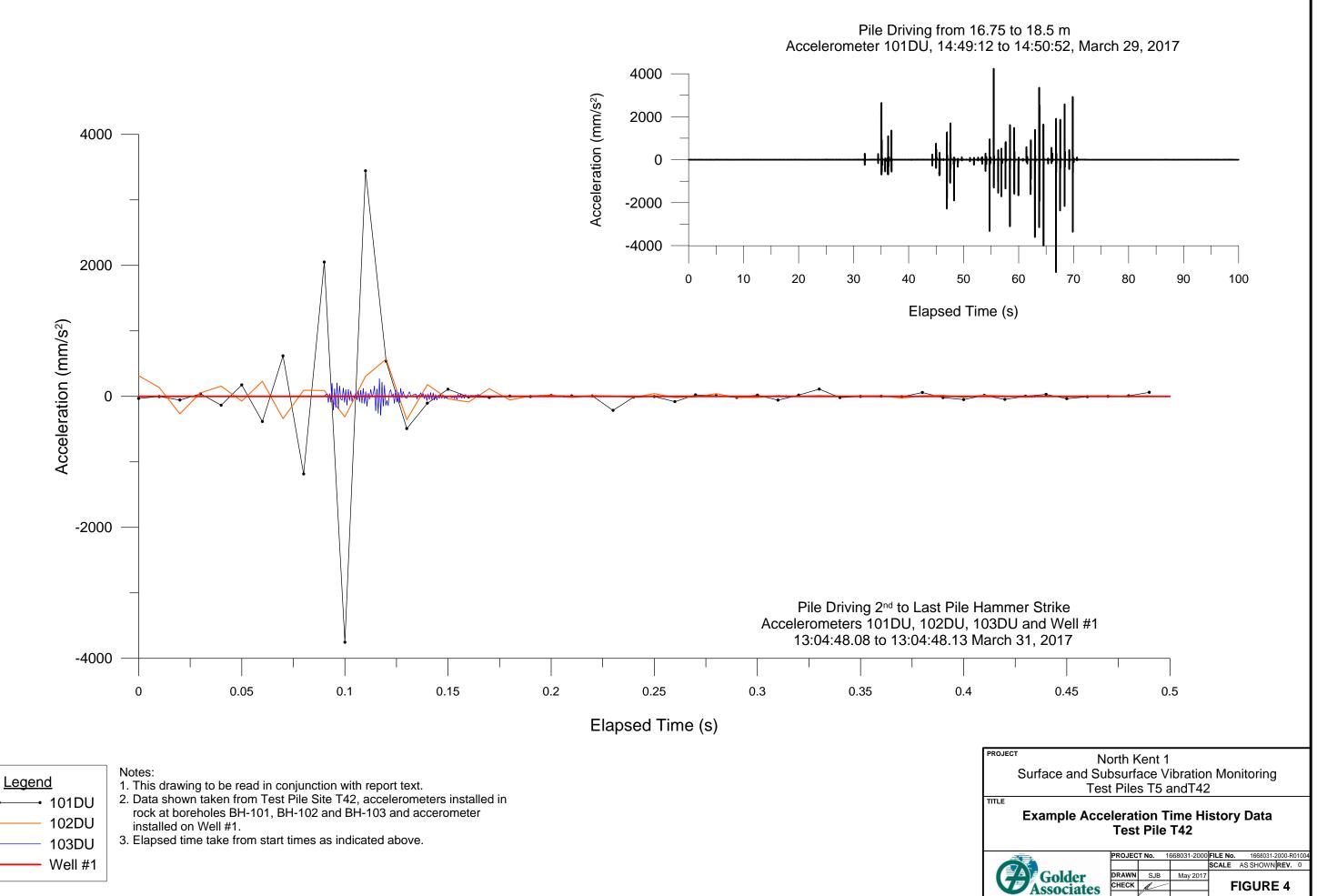
NORTH KENT 1 SURFACE AND SUBSURFACE VIBRATION MONITORING TEST PILES T5 AND T42

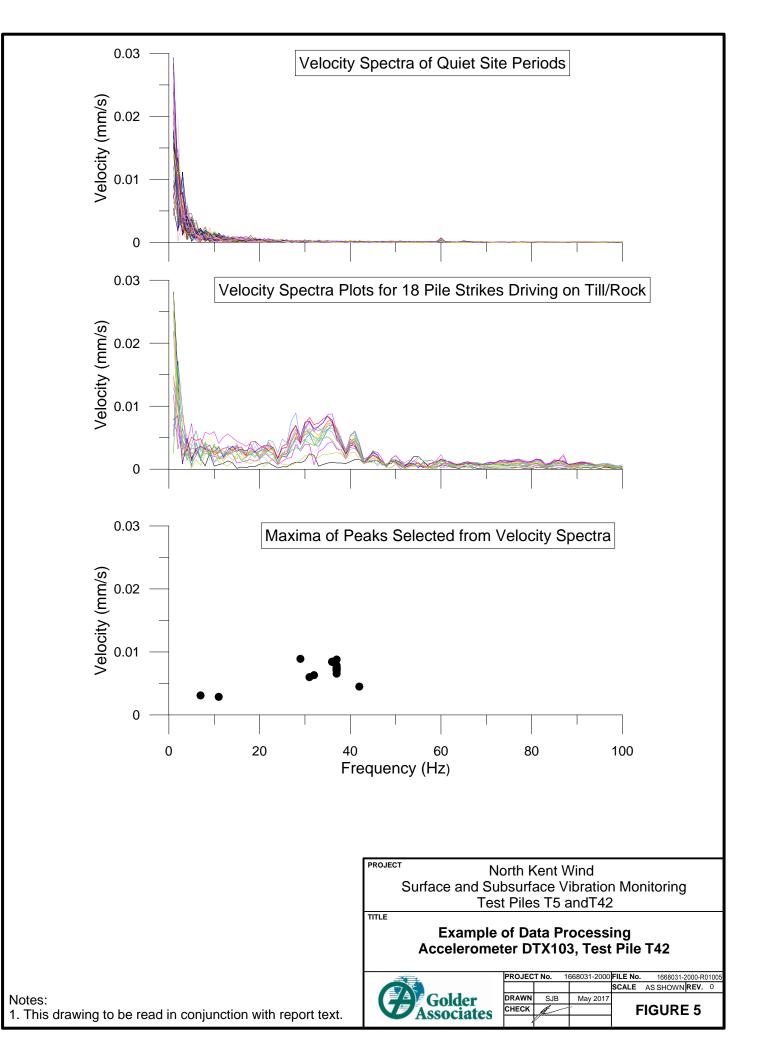
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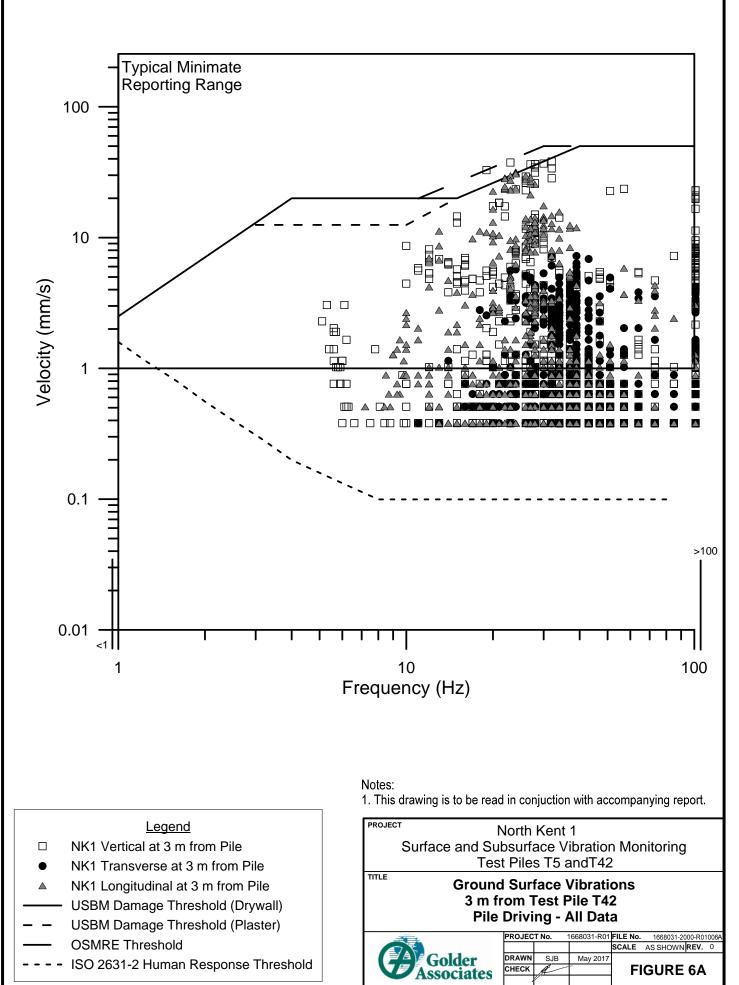
SITE AND INSTRUMENT LOCATION PLAN, T5













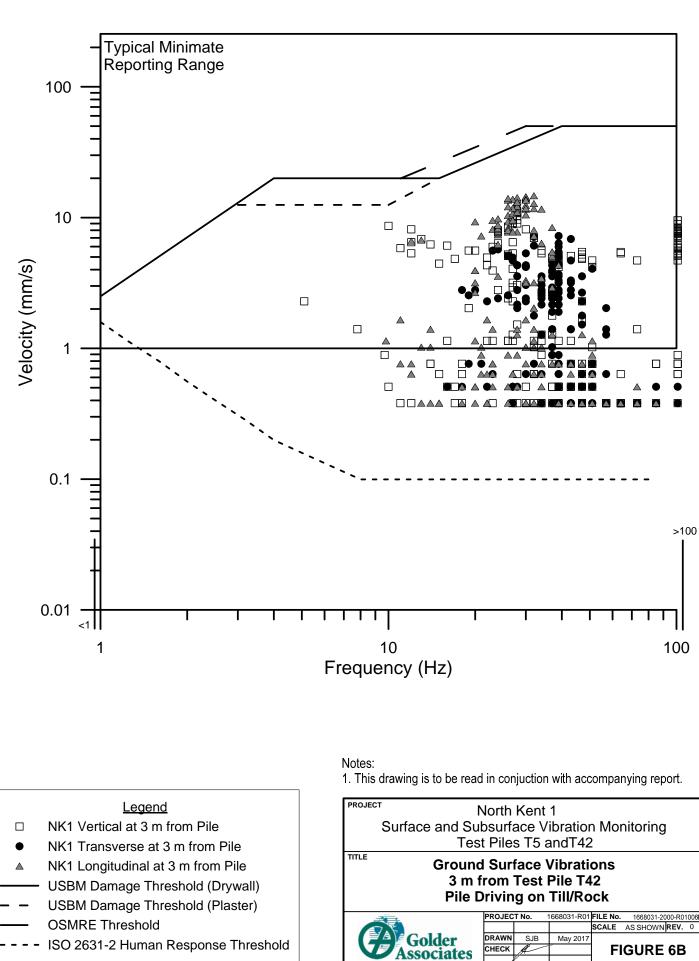
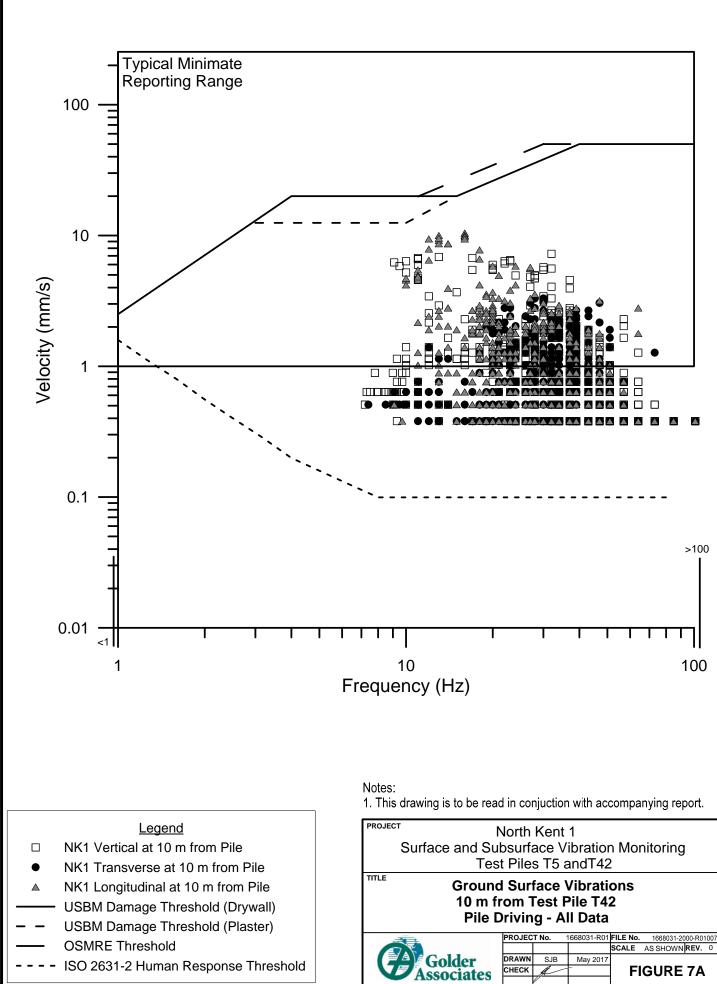
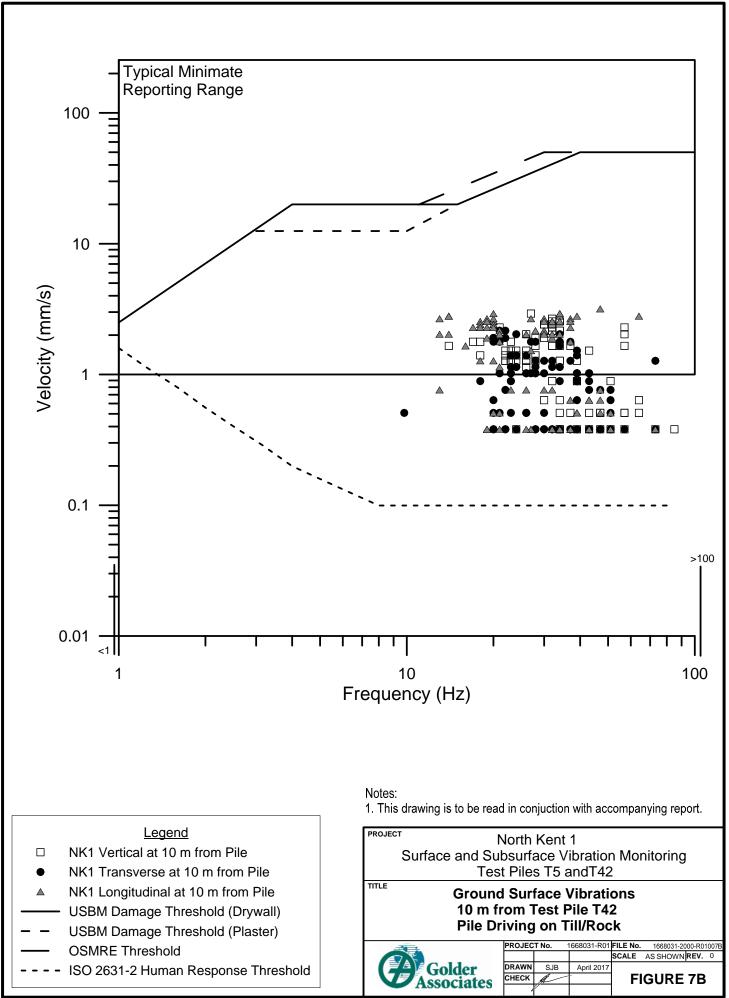
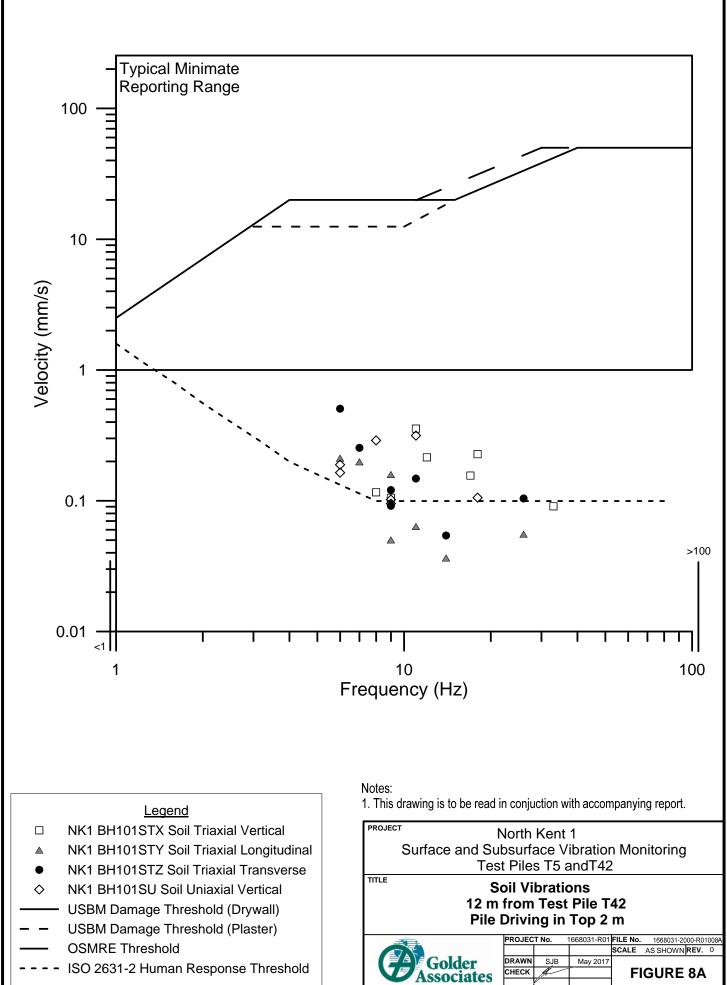
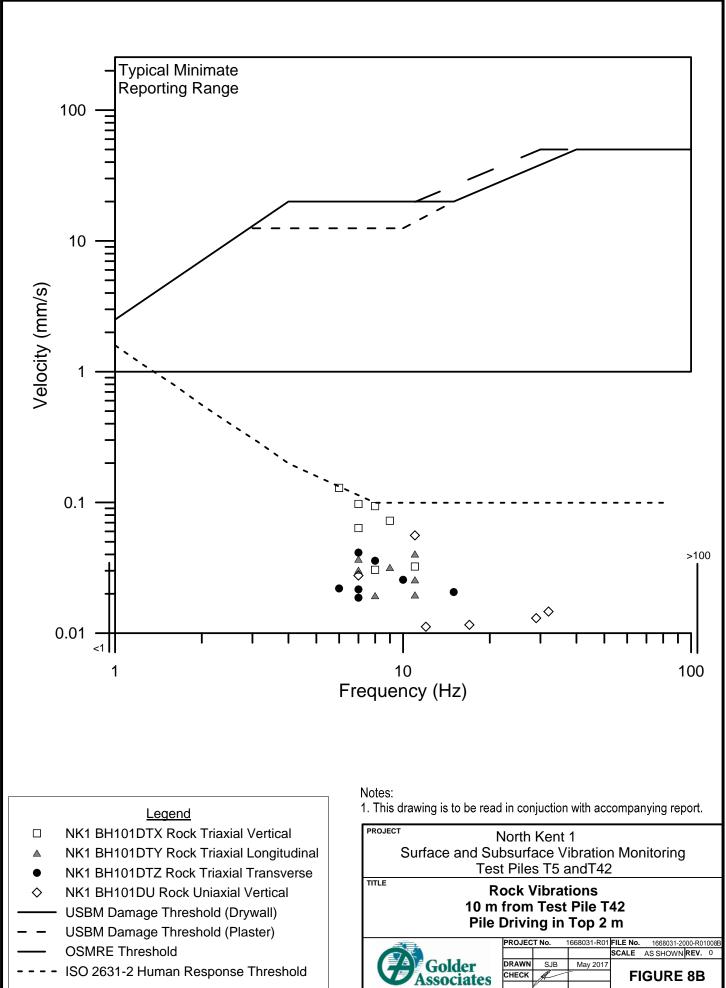


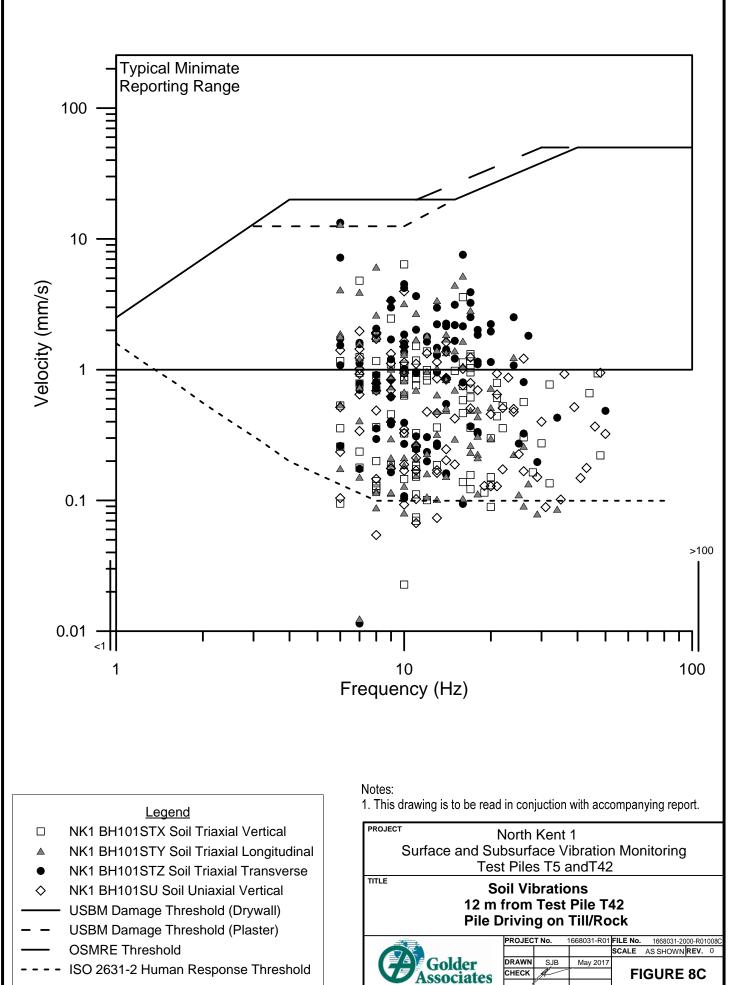
FIGURE 6B

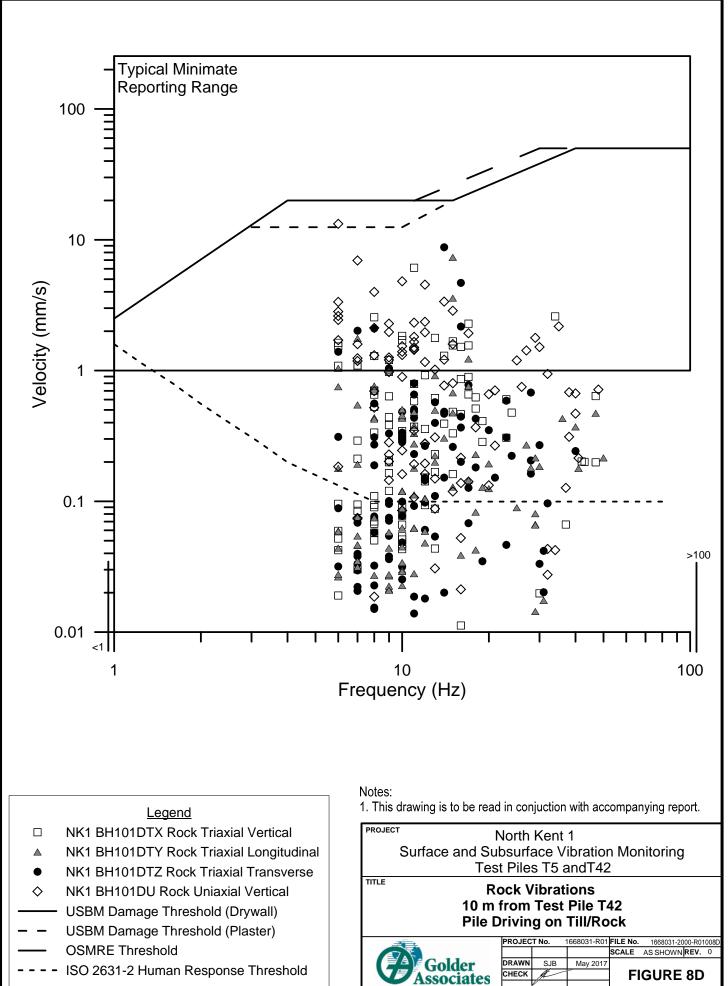


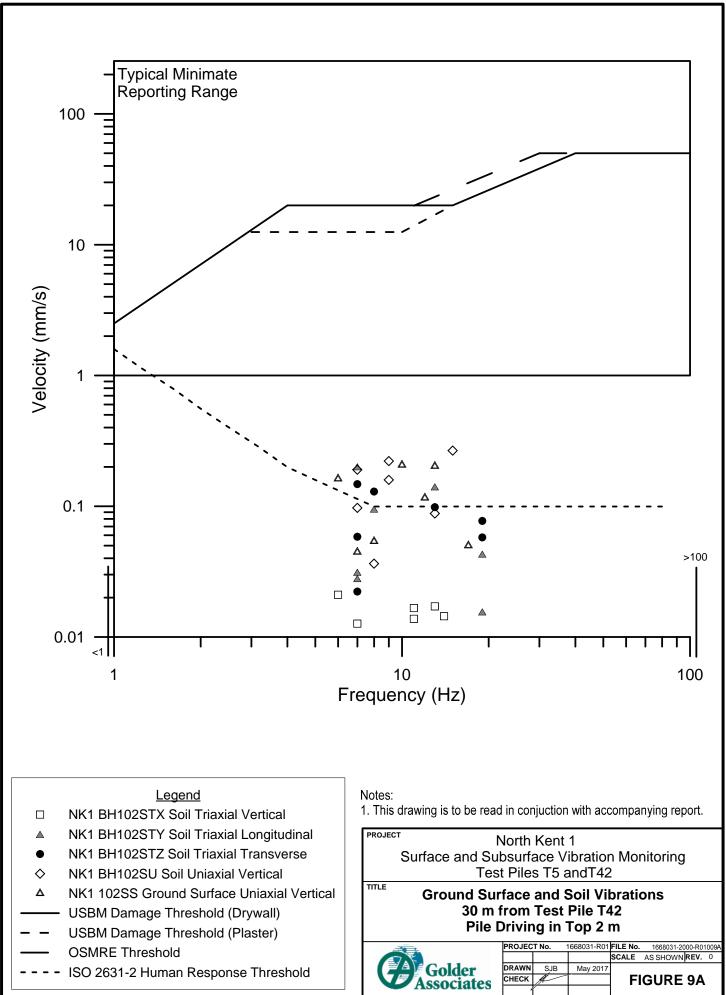


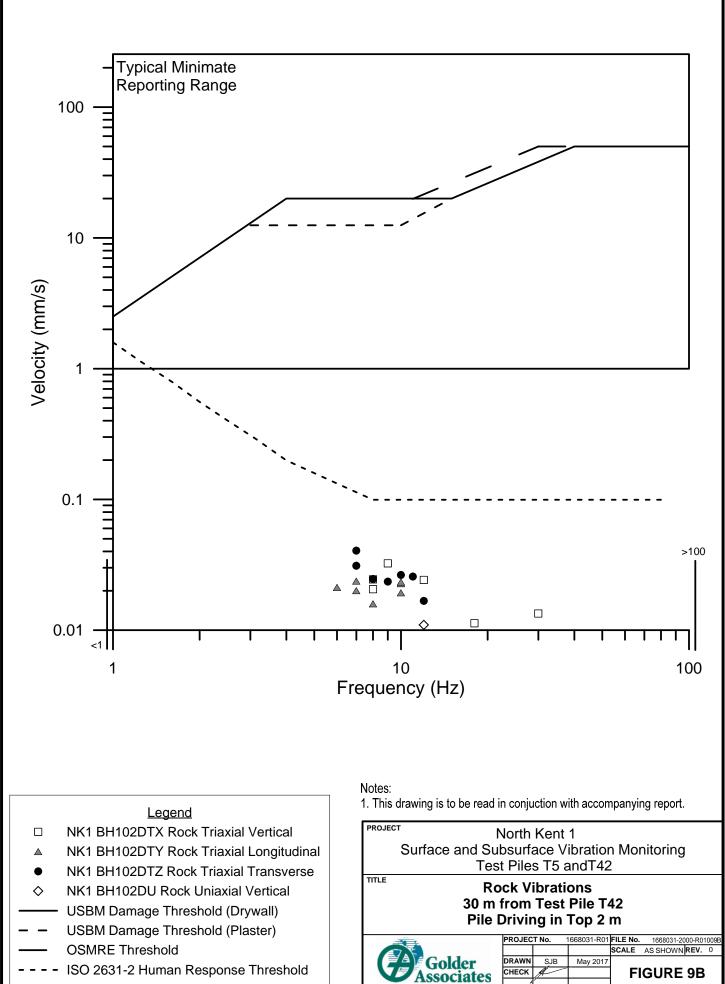


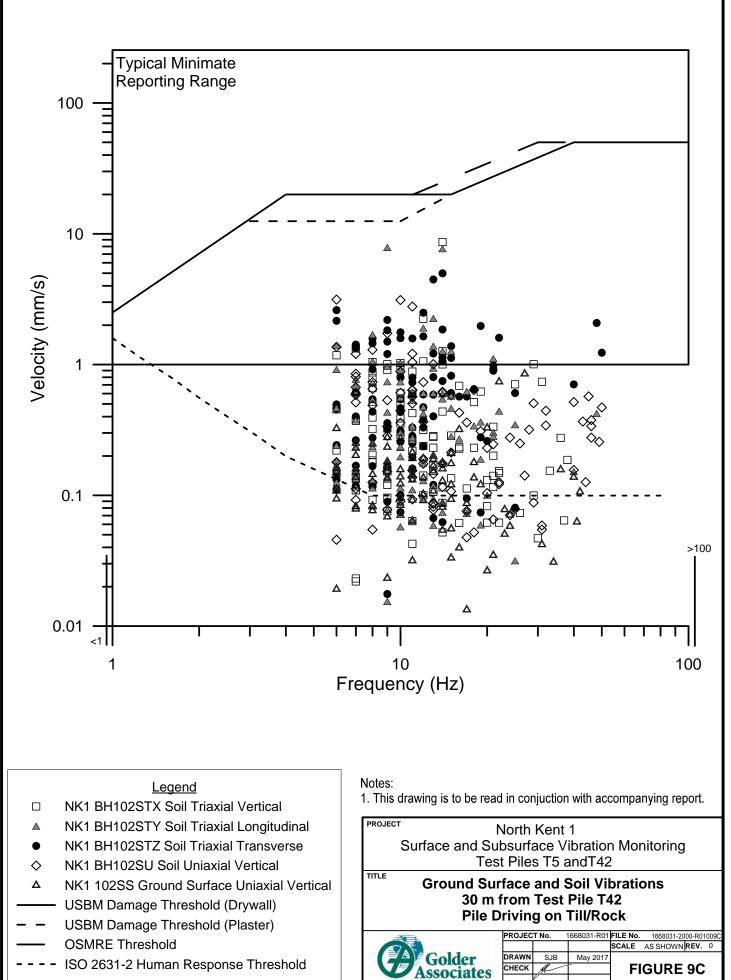


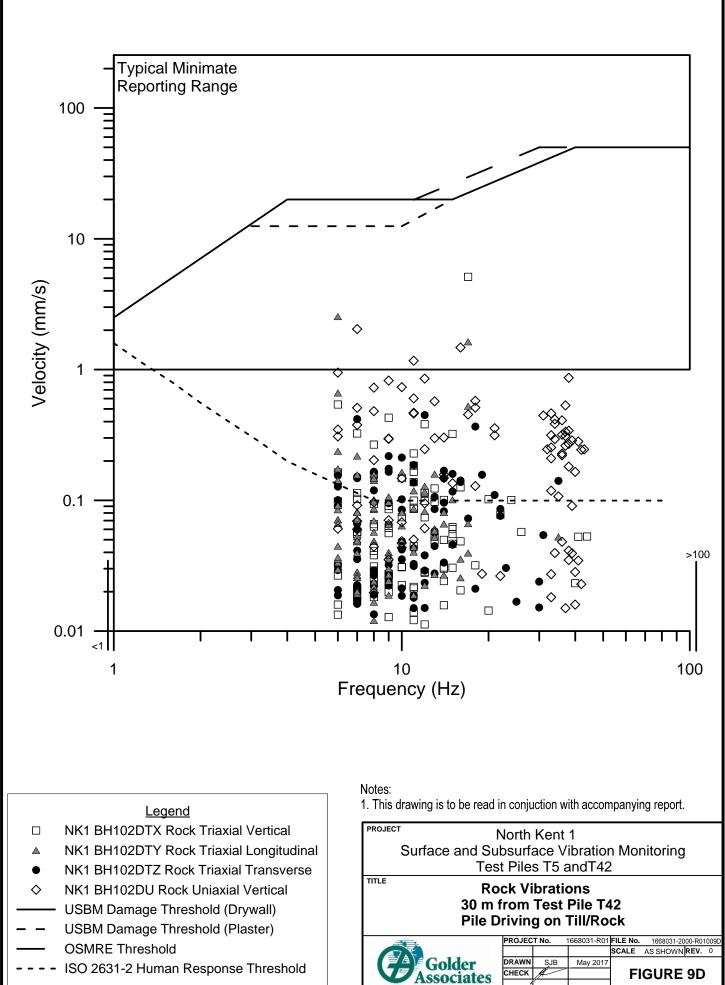


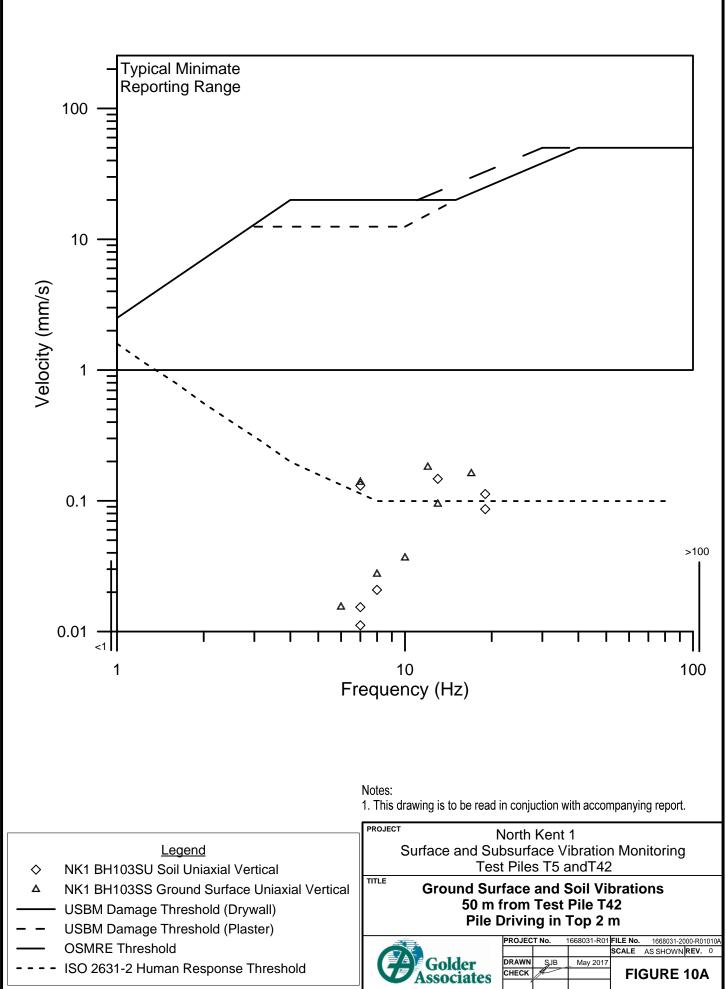


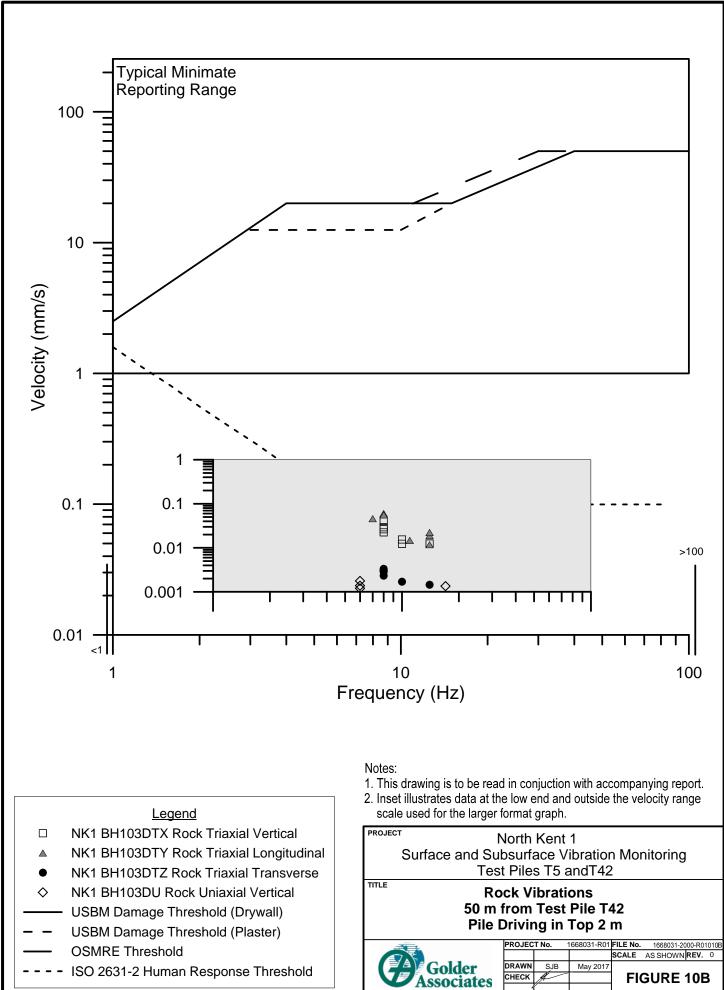


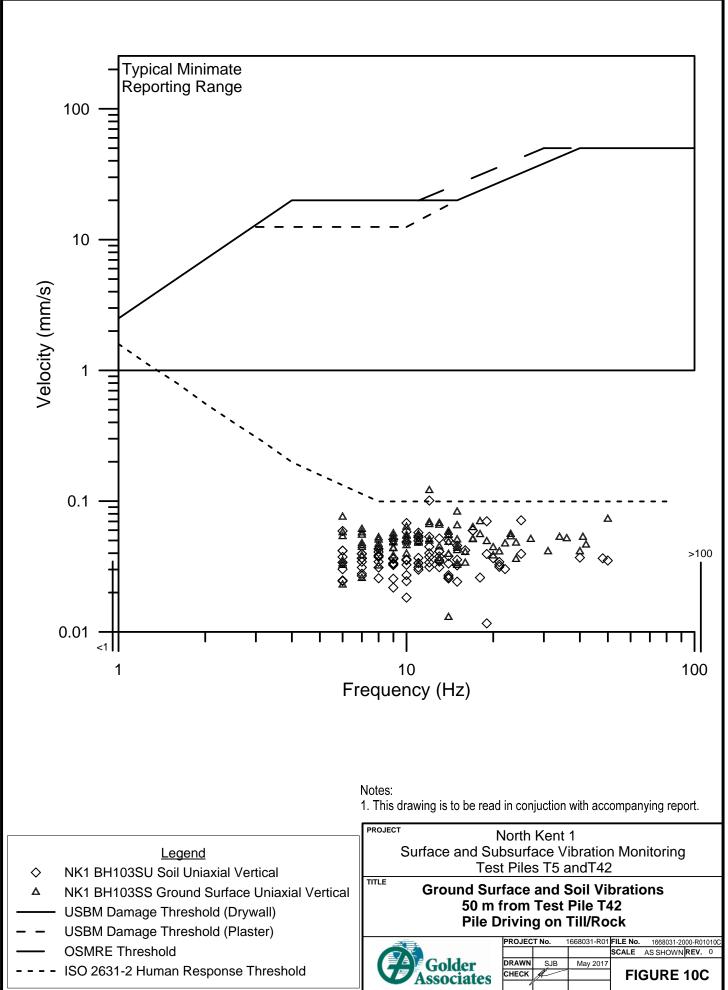


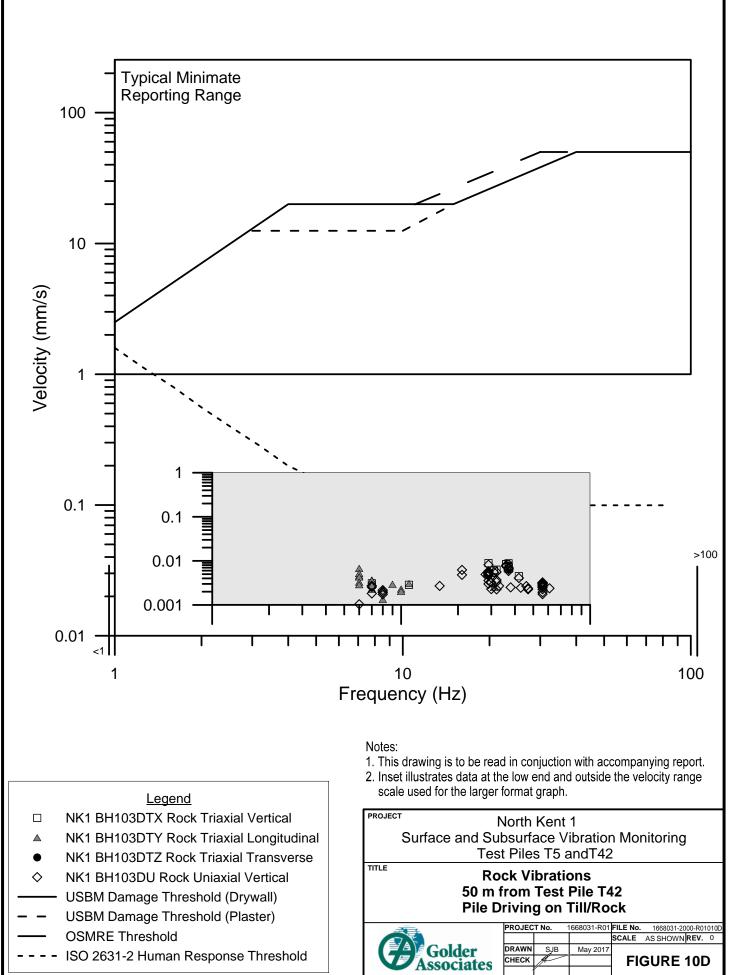


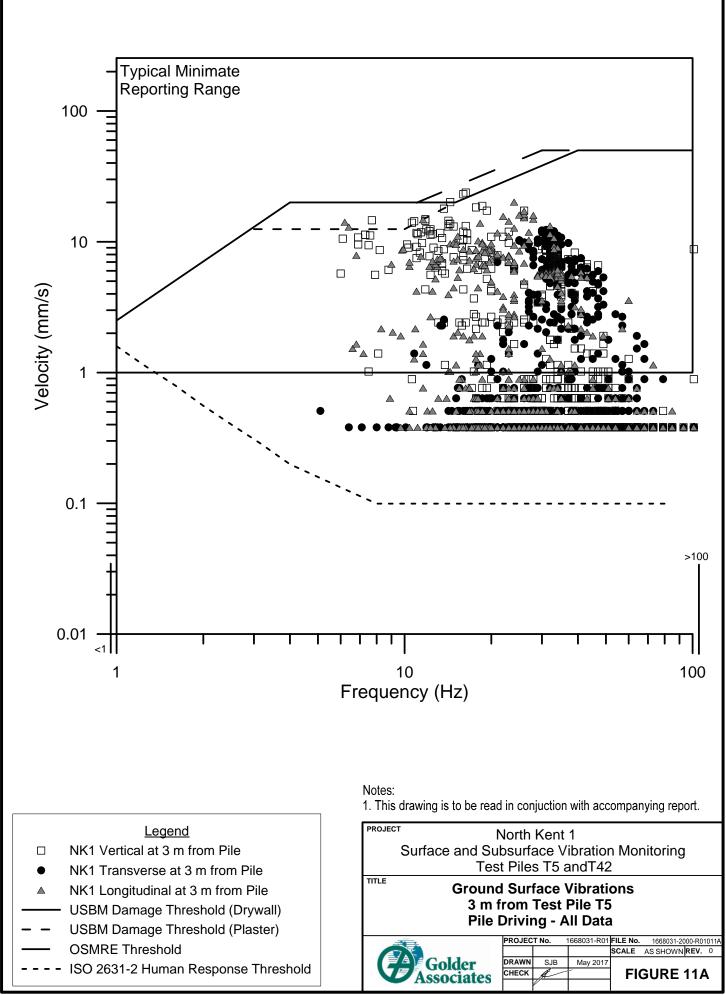


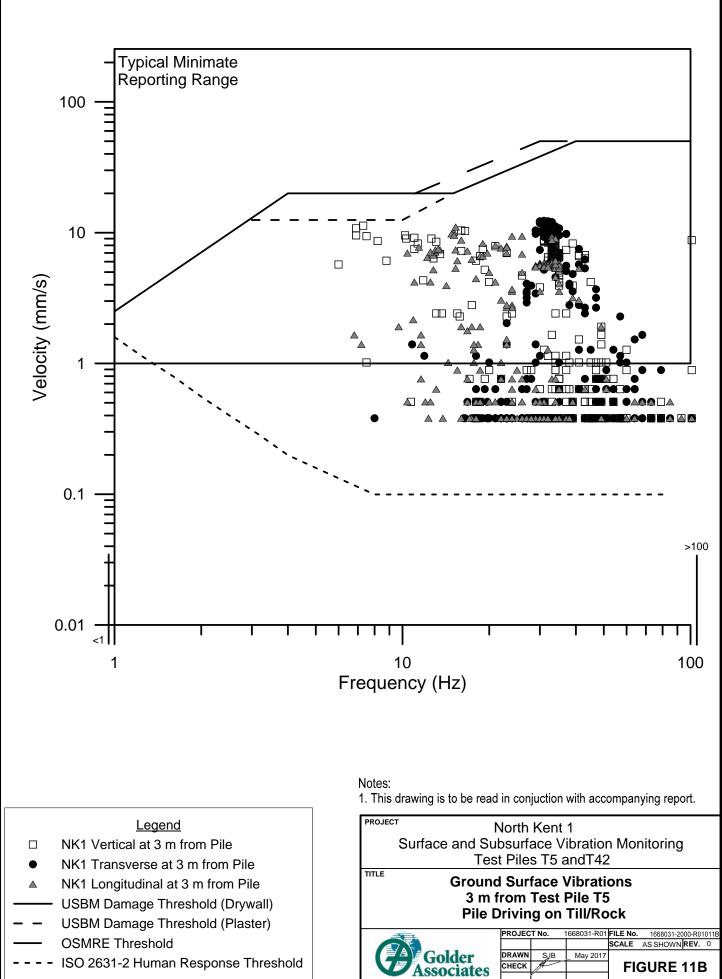


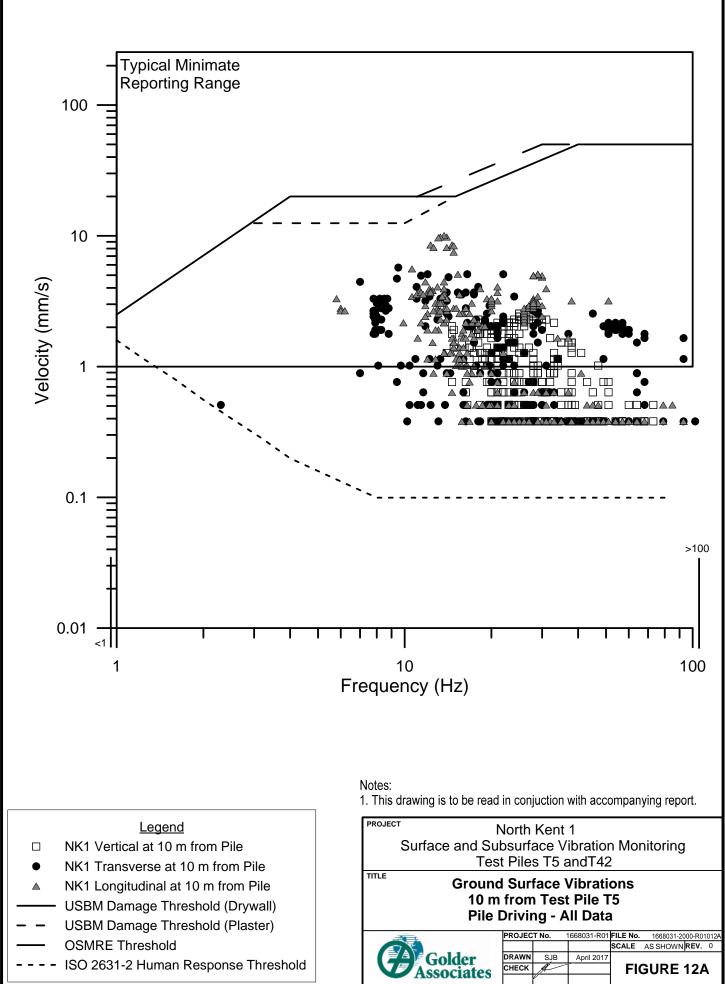


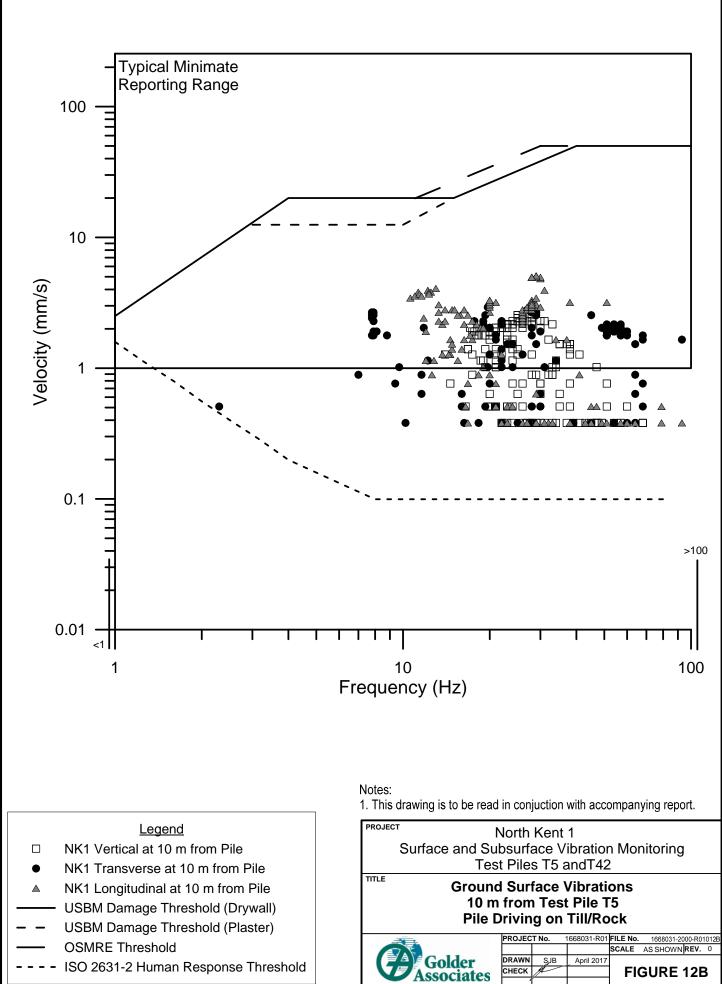


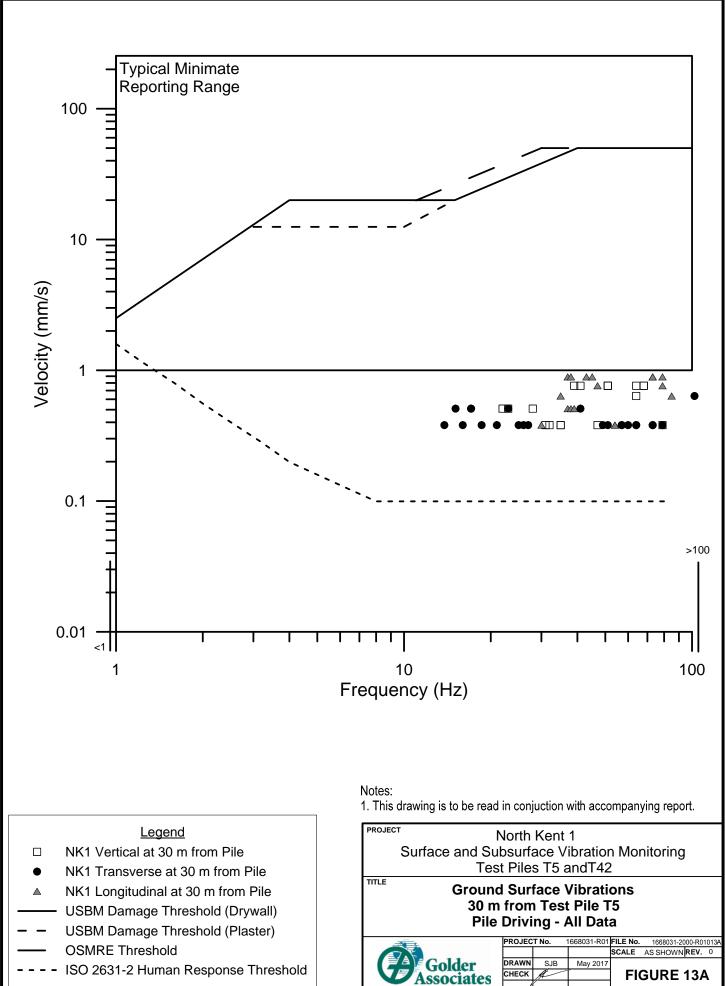


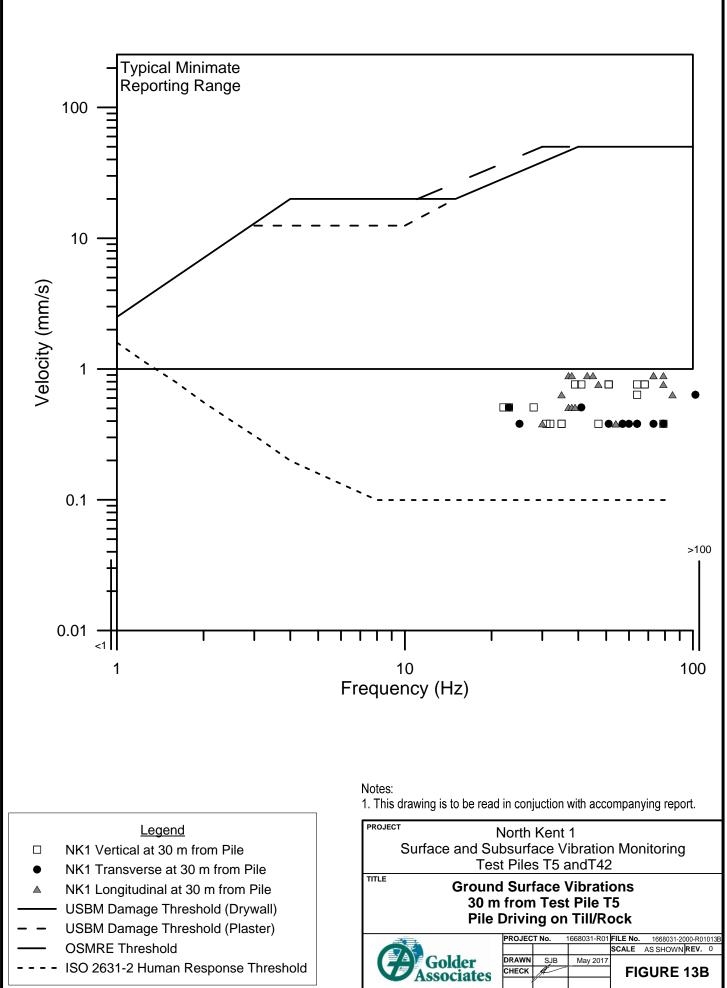


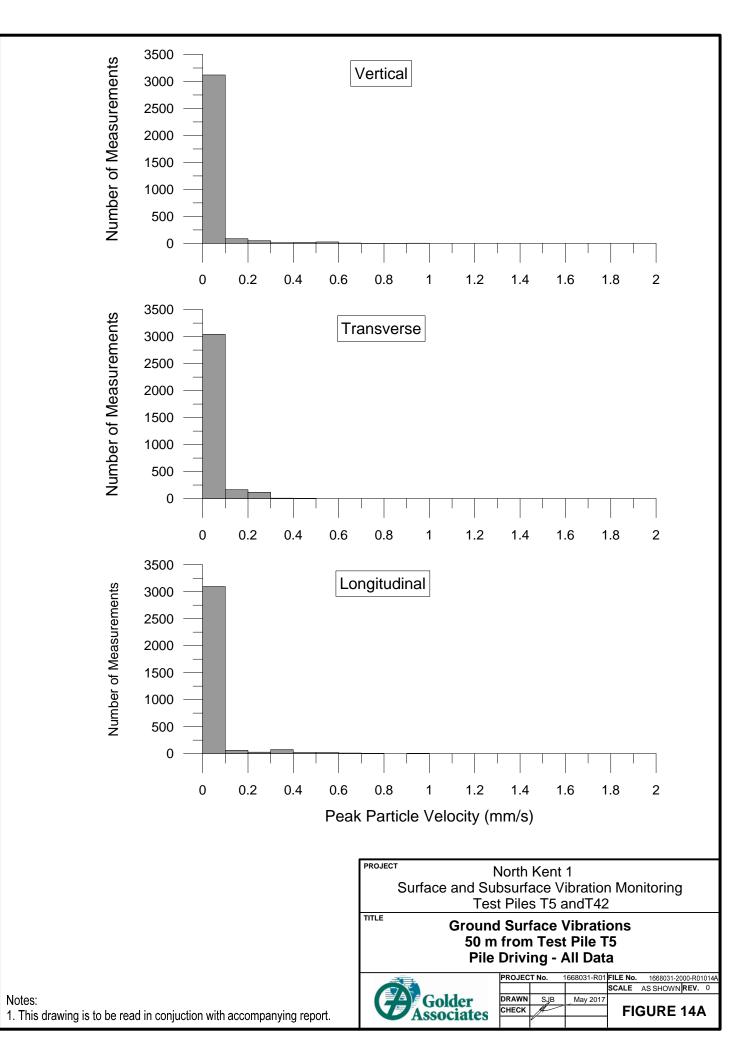




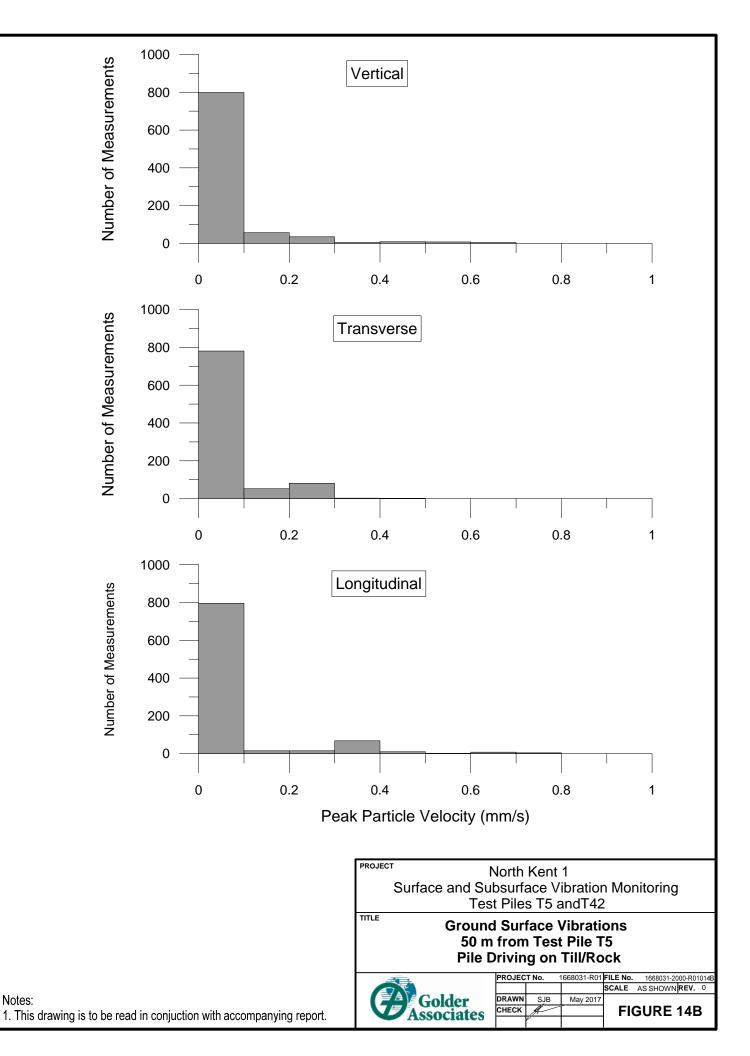




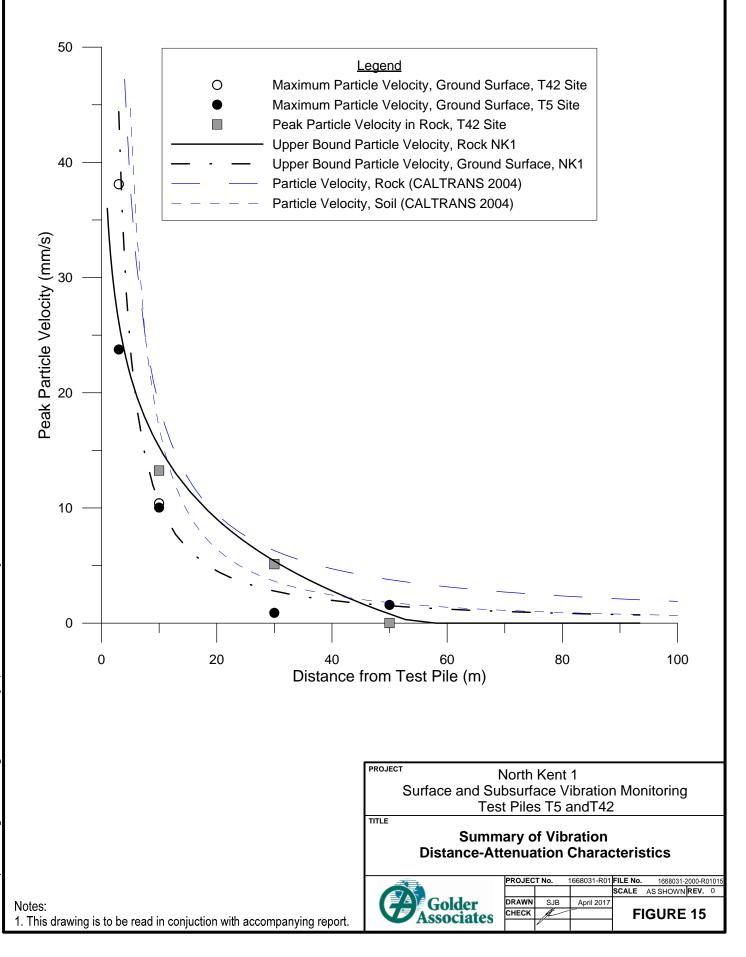




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APPENDIX A

Berminghammer Model B-32 Operational Specifications



Model B-32

Clean Series

2005

Features

- $\hfill\square$ Remote Throttle infinitely controlable energy
- Clean Combustion- Low Emissions
- $\hfill\square$ Fuel injection
- □ Easy Start in soft driving
- $\hfill\square$ Available with hydraulic trip
- □ Free-standing operation
- □ Specialty driving adapters
- Optional Kinetic Energy Monitor
- Optional Energy Control System (patented)
- □ Environmentally friendly (no-drip operation, bio-fuels and oils)

Operational Specifications

Ram mass: Rated Energy:

Stroke at Rated Energy:

Maximum Physical Stroke: Range of Operation:

Kinetic Energy at Rated Stroke:

Hammer Weight - bare hammer: Weight with Typical USA-Style Box Lead Guides:

Typical Direct-Drive Housing:

Total Typical Operating Weight:

Fuel Tank Capacity: Oil Tank Capacity:

Overall Length: Length including Direct-Drive Housing:

Minimum Box Lead size:

BERMINGHAM

FOUNDATION SOLUTIONS SINCE 1897

7,050 lbs (3 200 kg) 81,080 ft•lbs (110 kJ)

11.5 ft (3.5 m) 35 blows per minute 13.0 ft (4.0 m) 4.5-11.5 ft (1.4-3.5 m) 56-35 blows per minute 50,040 ft-lbs (67.8 kJ)

14,110 lbs (6 400 kg) 14,570 lbs (6 610 kg) 26 in (660 mm) guides 1,850 lbs (840 kg) 21 in (530 mm) opening 16,420 lbs (7 450 kg) (with guides, trip, and drive housing)

> 19.0 US Gal. (72 L) 6.5 US Gal. (25 L)

> > 20.1 ft (6.1 m) 21.7 ft (6.6 m)

26 in (660 mm)



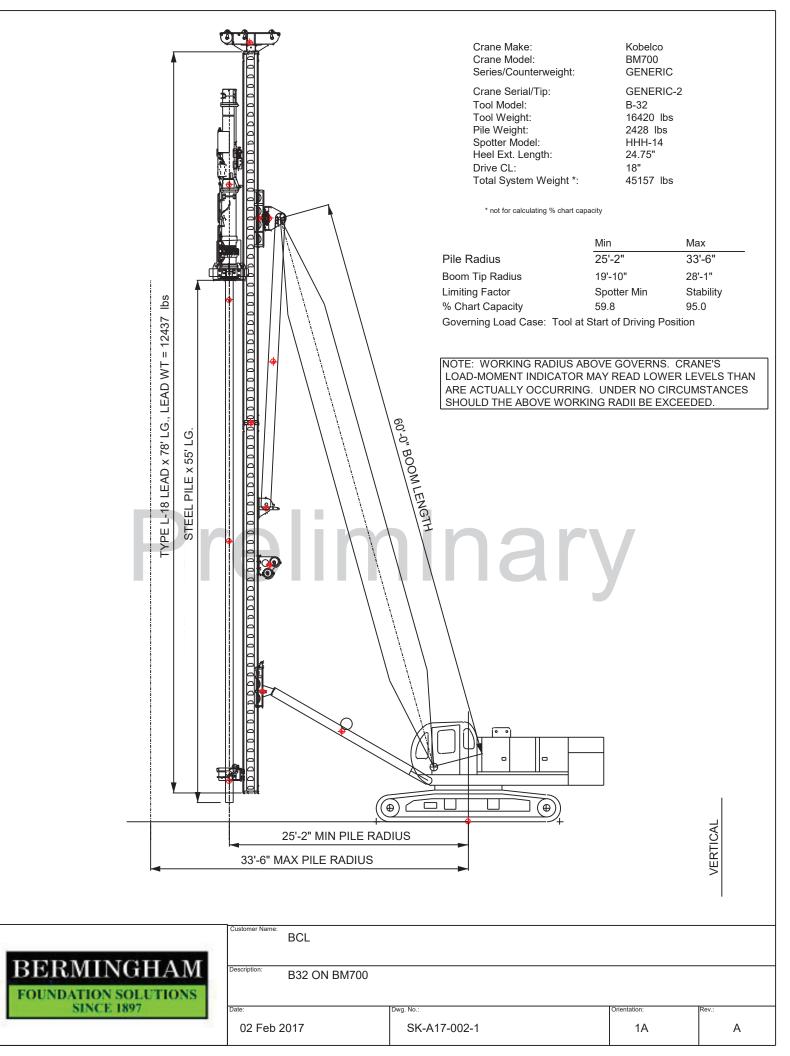
Wellington Street Marine Terminal, Hamilton, ON, Canada L8L 4Z9 Phone 1.905.528.0425 • Fax 1.905.528.6187 • Toll Free (Canada & USA 1.800.668.9432) www.berminghammer.com

English U	nits		
B-32		7,050 lb Pis	ton
BPM	Stroke	Potential Energy	Velocity
	(ft)	(ft•lb)	(ft/s)
35	11.8	83,190	22.5
36	11.2	78,960	22.0
37	10.6	74,730	21.5
38	10.0	70,500	21.0
39	9.5	66,980	20.5
40	9.1	64,160	20.0
41	8.6	60,630	19.5
42	8.2	57,810	19.0
43	7.8	54,990	18.5
44	7.5	52,880	18.0
45	7.2	50,760	17.5
46	6.9	48,650	17.0
47	6.6	46,530	16.5
48	6.3	44,420	16.0
49	6.0	42,300	15.5
50	5.8	40,890	15.0
51	5.6	39,480	14.6
52	5.4	38,070	14.2
53	5.2	36,660	13.8
54	5.0	35,250	13.4
55	4.8	33,840	13.0
56	4.6	32,430	12.6

SI Units	5		
B-32		3 200 kg Pis	ston
BPM	Stroke	Potential Energy	Velocity
	(m)	(kJ)	(m/s)
35	3.60	113	6.9
36	3.41	107	6.7
37	3.23	101	6.6
38	3.05	95.7	6.4
39	2.90	91.0	6.3
40	2.77	87.0	6.1
41	2.62	82.2	5.9
42	2.50	78.5	5.8
43	2.38	74.7	5.6
44	2.29	71.9	5.5
45	2.20	69.1	5.3
46	2.10	65.9	5.2
47	2.01	63.1	5.0
48	1.92	60.3	4.9
49	1.83	57.4	4.7
50	1.77	55.6	4.6
51	1.71	53.7	4.5
52	1.65	51.8	4.3
53	1.59	49.9	4.2
54	1.52	47.7	4.1
55	1.46	45.8	4.0
56	1.40	43.9	3.8



Stroke height is a function of soil resistance and may not be attainable in certain driving conditions. Standard Operating Range.





APPENDIX B

Records of Boreholes T42 and T5 (AMEC)



R	ECORD	OF BOREHOLE No. <u>T</u>	5							
Pro	ject Number:	SWW167102					Drillin	g Meth	150 mm O.D. Hollow Stem Augers	
Pro	oject Client:	North Kent Wind 1 GP Inc.					Drillin	g Mach	: <u>D-120</u>	
Pro	oject Name:	North Kent Wind 1 Project					Date	Started	15 May 2016 Date Completed: 15 May 2016 amec	
Pro	ect Location:	Municipality of Chatham-Kent, C	ntario				Logge	ed by:	ss Compiled by: ss foster	
Dri	lling Location:	N4708384, E391442					Revie	wed by	SM Revision No. 0	
									wheeler	
	LİTH		SC	IL SA	MPLI	NG			FIELD TESTING LAB TESTING	
ogy Plot		DESCRIPTION	le Type	le Number	/ery (%)	N' Value	(m) H.	ATION (m)	PenetrationTesting SPT DCPT Average Limits We W W Plastic Liquid Intact O Intact Remould O Remould O Moisture Content (%) Plastic Ciquid * Passing 75 um (%) O Moisture Content (%) COMMEN Built COMMEN COMMEN Built COMM	ZE

Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT 'N' Value	DEPTH (m)	elevation (m	○ SPT ● DCPT MTO Vane* Nilcon Vane* △ Intact ◇ △ Remould Remould ■ Undrained Shear Strength (kPa) (from P, Penetrometer tests) 20 20 40 60	Plastic Liquid * Passing 75 um (%) O Moisture Content (%) * Unit Weight (KN/m3) 20 40 60 80	INSTRUMENTA INSTALLATION	GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
	Local Ground Surface Elevation: TOPSOIL (380mm thick)	0	0		0)	-	<u> </u>	20 40 60 80	20 40 80 80		
	SILTY CLAY Trace sand Mottled brown and grey Firm	0.4				-					
		ss	1	94	6	- - 1 -		O	o ²⁴		
77	FINE SAND Trace silt	1.4				-					
	Brown Compact	SS	2	67	12	- - - -		0	o ²³		
\mathbb{X}	SILTY CLAY Trace sand	2.1				-					
	Grey Firm	SS	3	100	6	-		0	o ²⁴		
						- 		· · · · · · · · · · · · · · · · · · ·			
	Silt seams	ss	4	100	6	-		0	o ²⁷		
	Soft					-					
		SS	5	100	2	- 4 		Þ	o ³⁴		
						-					
		SS	6	100	2	- 5 		D	o ³⁴		
						-					
						- - - 6					
		TW	7	100		-					
		VT				-		21 ↑ △			
						- 7 - -					
	ec Foster Wheeler	freestanding g	Iroundw	ater obs	served in	n open bo	orehol	e upon completion of drilling.			
118 Win	65 County Road 42 dsor, ON N8N 2M1	sholo dataila ar	ronorte d	do not c	notit-to -	thorough	undo	tanding of all notantial any distance of	report and requires interesting -	-	from a sublified
Fax: www	510 735 0660 Geol	ehole details, as pr technical Enginee ompanying 'Explai	r Also,b	orehole ir	nformatio	n shou l d be	e read i	tanding of all potential conditions pr n conjunction with the geotechnical	resent and requires interpretive a I report for which it was commission	oned and t	from a qualified the Page: 1 of 3

RECORD OF BOREHOLE No. 15 Project Number: SWW167102 Drilling Method: 150 mm O.D. Hollow Stem Augers North Kent Wind 1 GP Inc. Project Client: Drilling Machine: D-120 amec foster wheeler Project Name: North Kent Wind 1 Project Date Started: <u>15 May 2016</u> Date Completed: <u>15 May 2016</u> Project Location: Municipality of Chatham-Kent, Ontario Logged by: SS Compiled by: SS ____ ___ Drilling Location: N4708384, E391442 Reviewed by: SM Revision No.: 0

	LITHOLOGY PROFILE		SOIL S	AMPL	NG			FIELD TESTING	LAB TESTING					
Lithology Plot	DESCRIPTION	Samula Tune	Sample Number	Recovery (%)	SPT 'N' Value	DEPTH (m)	ELEVATION (m)	PenetrationTesting ○ SPT ● DCPT MTO Vane* Nilcon Vane* △ Intact ◇ Intact ■ Remould ● Remould Undrained Shear Strength (kPa) (from P. Penetrometer tests) 20 40 60 80	Atterberg Limits W _P W W _L Plastic Liquid * Passing 75 um (%) O Moisture Content (%) * Unit Weight (KN/m3)	INSTRUMENTATION INSTALLATION		COMMI & GRAIN DISTRIB (%)	SIZE UTION	CL
	SILTY CLAY Trace sand						Ξ	20 40 60 80	20 40 60 80		GR	54	51	
	Grey	S	5 8	100	1	- 8	(D	o ³⁷					
						-								
						E								
						- - 9								
						Ē								
		S	S 9	100	3	-		Ο	o ³⁸					
						_		20						
		V	r 🔤			- 10 -		2 0 7 ▲						
						-								
		S	S 10	100	2	- 11		D	o ⁴²					
						-								
						-								
						- - - 12								
						- - -								
		S	5 11	100	2				o ⁴⁷					
								15						
						- 13 - -		15 7 [∆]		1				
						-								
						E								
		s	5 12	100	2	- 14		D	o ⁴⁷					
						Ē								
	SAND AND GRAVEL TILL Trace silt, trace clay	14.6				E								
	Grey Loose					- 15		· · · · · · · · · · · · · · · · · · ·						
	nec Foster Wheeler	Z No freestanding	g groundv	vater obs	 served i	n open b	orehol	e upon completion of drilling.		I				
119	65 County Road 42 dsor, ON N8N 2M1 519-735-2499													
⊢ax	519-735-2499 : 519-735-9669 w.amecfw.com	Borehole details, as Geotechnical Engir accompanying 'Exp	eer Also,	borehole i	nformatio	a thorough on shou l d b	i unders be read i	tanding of all potential conditions p n conjunction with the geotechnica	resent and requires interpretive a report for which it was commissi	ssistance f oned and t	rom a qualif he	ied	Page: 3	2 of 3
Cont	inued on Next Page													

RECORD	OF BOREHOLE No. <u>T5</u>					
Project Number:	SWW167102	Drilling Method:	150 mm O.D.	Hollow Stem Aug	ers	-
Project Client:	North Kent Wind 1 GP Inc.	Drilling Machine:	D-120			
Project Name:	North Kent Wind 1 Project	Date Started:	<u>15 May 2016</u>	Date Completed:	<u>15 May 2016</u>	amec
Project Location:	Municipality of Chatham-Kent, Ontario	Logged by:	SS	Compiled by:	SS	foster
Drilling Location:	N4708384, E391442	Reviewed by:	SM	Revision No.:	<u>0</u>	wheeler

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Wileter												WITCELET
<u> </u>			SOIL	SA	MPLI	١G			FIELD TESTING	LAB TESTING	7	
Lithology Plot	DESCRIPTION	Samnle Tyne			Recovery (%)	SPT 'N' Value	DEPTH (m)	ELEVATION (m)	PenetrationTesting ○ SPT DCPT MTO Vane* Nilcon Vane* △ Intact Intact ▲ Remould Remould Undrained Shear Strength (kPa) (from P. Penetrometer tests) 80	Atterberg Limits W₀ W W₀ ■ ● ● Plastic Liquid # × Passing 75 um (%₀) o Moisture Content (%₀) ∨ Unit Weight (KN/m3) 20 40 60 80	INSTRUMENTATION INSTALLATION	COMMENTS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
	SAND AND GRAVEL TILL Trace silt, trace clay Grey Loose	s	5 1	3	100	8			0	o ²⁴		
	Rock fragments						- - 16 - -					
	FINE GRAINED SHALE Black alternating Dark grey	16.8S	s 1	4	100>	50/25m	- - - - - - 17			o ¹³		
		R	C 1	5	100		-					TCR = 100% SCR = 76% RQD = 60%
							- - - 18 - -					
		R	C 1	6	99		- - - - - - - - - - -					TCR = 95% SCR = 92% RQD = 91%
	END OF BOREHOLE		C 1	7	95		- - - - - - - - - -					TCR = 99% SCR = 99% RQD = 99%
	END OF BOREHOLE	20.3					- - - - - - - -					
							- - - - - - - - -					
En		freestanding	g groun	ıdwa	ter obs	erved ir	- - n open t	orehol	le upon completion of drilling.			
Fax	65 County Road 42 rdsor, ON N8N 2M1 519-735-2499 Bore : 519-735-9669 Geot w.amecfw.com acco	hole details, as echnical Engir mpanying 'Exp	s presen leer Als lanation	ted, d o, boi i of Bo	o not coi rehole in orehole L	nstitute a formatio .og'-	thoroug n shou l d	n unders be read	standing of all potential conditions p in conjunction with the geotechnical	resent and requires interpretive as report for which it was commission	ssistance f oned and t	rom a qualified he Page: 3 of 3

RECORD OF	F BOREHOLE No. <u>T42</u>					
Project Number: SV	WW167102	Drilling Method:	150 mm O.D.	Hollow Stem Aug	ers	
Project Client: No	orth Kent Wind 1 GP Inc.	Drilling Machine:	D-120			
Project Name: No	orth Kent Wind 1 Project	Date Started:	<u>11 May 2016</u>	Date Completed:	12 May 2016	amec 🏹
Project Location: Mu	unicipality of Chatham-Kent, Ontario	Logged by:	<u>ss</u>	Compiled by:	SS	foster
Drilling Location: <u>N4</u>	4701243, E393628	Reviewed by:	<u>SM</u>	Revision No.:	<u>0</u>	wheeler

	LITHOLOGY PROFILE	sc	SOIL SAMPLING					FIELD TESTING	LAB TESTING	_				
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT 'N' Value	DEPTH (m)	ELEVATION (m)	PenetrationTesting ○ SPT ● DCPT MTO Vane* Nilcon Vane* △ Intact ● Intact ■ Remould ● Remould Undrained Shear Strength (kPa) (from P. Penetrometer tests) 20 40 60 80	Atterberg Limits Wp W W, Plastic Liquid * Passing 75 um (%) O Moisture Content (%) * Unit Weight (KN/m3)	INSTRUMENTATION INSTALLATION	[COMMI & GRAIN STRIB (%	SIZE UTION)	
	Local Ground Surface Elevation: TOPSOIL (355mm thick)	Sa	Sa	Re	SP			20 40 60 80	20 40 60 80	źź	GR	SA	SI	CL
	()													
	SILTY CLAY Trace sand Mottled brown and grey Firm	0.4				- - - -								
		SS	1	61	6	- - 1 - -		0	o ²³					
	FINE SAND Trace silt Brown Compact	1.4 SS	2	83	14	- - - -		0	_o 24					
						- 2								
	SILTY CLAY Trace sand	2.1				-								
	Grey Stiff	SS	3	100	10			0	o ²⁴					
	Firm					- 3								
		SS	4	100	6	- - - -		0	o ²⁴					
						-								
	Soft	SS	5	100	2	- - 4 -		p	o ²⁸					
						-								
		SS	6	100	1	- - - 5		þ	o ³¹					
						-								
						- - - - 6								
		SS	7	100	2			p	o ³⁷					
						F		21						
		VT				ŀ								
						- 7 - -								
						_								
	ec Foster Wheeler vironment & Infrastructure $\sum_{n=1}^{\infty} No \text{ free}$	standing g	roundw	ater obs	served i	n open l	borehol	e upon completion of drilling.						
Win Tel: Fax: www	65 County Road 42 dsor, ON N8N 2M1 519-735-2499 Borehole 519-735-9669 Geotechn w.amecfw.com accompa inued on Next Page	details, as pr ical Engineer nying 'Explan	esented, Also, be ation of E	do not co orehole ir 3orehole	onstitute a nformatic Log'-	a thoroug on shou l d	h unders be read i	tanding of all potential conditions p n conjunction with the geotechnical	resent and requires interpretive a report for which it was commissi	ssistance f oned and ti	rom a qualifi ne	ed	Page:	1 of 4

RECORD OF BOREHOLE No. T42

Project Number:	SWW167102	Drilling Method:	150 mm O.D.	Hollow Stem Aug	ers
Project Client:	North Kent Wind 1 GP Inc.	Drilling Machine:	D-120		
Project Name:	North Kent Wind 1 Project	Date Started:	<u>11 May 2016</u>	Date Completed:	<u>12 May 2016</u>
Project Location:	Municipality of Chatham-Kent, Ontario	Logged by:	SS	Compiled by:	SS
Drilling Location:	N4701243, E393628	Reviewed by:	SM	Revision No.:	<u>0</u>



DESCRIPTION SILTY CLAY Trace sand Grey Soft	SS Sample Type	Sample Number	Recovery (%)	SPT 'N' Value	(m)	(m) NO	PenetrationTesting ○ SPT ● DCPT MTO Vane* Nilcon Vane*	Atterberg Limits W _P W W _L ■ O ● Plastic Liquid	ENTATIO		COMMI & GRAIN	SIZE	
Trace sand Grey	SS		Rec	N' TYS	DEPTH (m)	ELEVATION	A Intact A Intact Remould Arrow Intact Remould Arrow Intact Arrow Intact	 ※ Passing 75 um (%) ○ Moisture Content (%) ★ Unit Weight (KN/m3) 20 40 60 80 	INSTRUMENTATION INSTALLATION	GR	DISTRIB (%)		с
		8	100	1	- - - - - - - - -		Þ	o ³³					
	SS	9	100	2	- - 9 - - - -		Þ	o ³⁷					
	VT				- - - 10 - - -		7 ²⁰						
	TW	10	100		- - - - - - - -		18						
	VT				- - - - - - - - -		↑ ¹⁸						
	SS VT	11	100	2	- - - - - - - - - - -								
	SS	12	100	2	- - - - - - - - - - - - -		p						
nec Foster Wheeler					- - - - - - - 15		e upon completion of drilling.						

RECORD OF BOREHOLE No. T42 Project Number: SWW167102 Drilling Method: 150 mm O.D. Hollow Stem Augers North Kent Wind 1 GP Inc. Project Client: Drilling Machine: D-120 _ Project Name: North Kent Wind 1 Project Date Started: 11 May 2016 Date Completed: 12 May 2016 Project Location: Municipality of Chatham-Kent, Ontario Logged by: SS Compiled by: SS ____ ____ Drilling Location: N4701243, E393628 0 Reviewed by: SM Revision No.:



LITHOLOGY PROFILE SOIL SAMPLING FIELD TESTING LAB TESTING												
		SC	ML SA	MPLI	NG					z	COMMENTS	
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT 'N' Value	DEPTH (m)	ELEVATION (m)	PenetrationTesting ○ SPT ● DCPT MTO Vane* Nilcon Vane* △ Intact ◇ Intact ▲ Remould ● Remould Undrained Shear Strength (kPa) (from P. Penetrometer tests) 20 40 60 80	Atterberg Limits W _p W W _L Plastic Liquid * Passing 75 um (%) O Moisture Content (%) * Unit Weight (KN/m3)	INSTRUMENTATION INSTALLATION	& GRAIN SIZE DISTRIBUTION (%)	
<u>d</u> Lit		Sai	Sai	Ř	SP	B	Ц	20 40 60 80	20 40 60 80	žž	GR SA SI CL	
	SILTY CLAY Trace sand Grey Soft	SS	13	100	2	-		D				
		VT				- - 16 -		13 8				
	Increased sand content					-						
	Firm					-						
		SS	14	100	7	- 17 		0				
	SAND AND GRAVEL TILL 17.7 Trace silt, trace clay Grey	-				- - - - - 18						
	Compact					-						
		SS	15	100	16	-		0				
						- 19 -						
	Rock fragments Saturated Very dense					-						
	FINE GRAINED SHALE 19.9 Black and light grey	S	16	100>	50/25m	m 20 						
		RC	17	100		-					TCR = 100% SCR = 95% RQD = 95%	
						- - 21						
						-					TCR = 100%	
		RC	18	100		- - 22					SCR = 93% RQD = 93%	
						-						
En	vironinent & milastructure –	nding gr	oundwa	ater obs	erved ir	n open b	orehol	e upon completion of drilling.				
Tel: Fax		ails, as pro Engineer	esented, Also, bo	do not co prehole in	nstitute a	thorough n shou l d b	unders be read i	tanding of all potential conditions p n conjunction with the geotechnica	present and requires interpretive as I report for which it was commission	ssistance f	rom a qualified 10	
	w.amecfw.com	a robian			9-						Page: 3 of 4	

RECORD OF BOREHOLE No. <u>T42</u>

Project Number: SWW167102 Drilling Method: 150 mm O.D. Hollow Stem Augers Project Client: North Kent Wind 1 GP Inc. Drilling Machine: D-120 Project Name: North Kent Wind 1 Project Date Started: <u>**11 May 2016**</u> Date Completed: <u>**12 May 2016**</u> Project Location: Municipality of Chatham-Kent, Ontario Logged by: SS Compiled by: SS ____ ____ Drilling Location: N4701243, E393628 Reviewed by: SM Revision No.: 0



											wheeler
		SC	IL SA	MPLI	NG			FIELD TESTING	LAB TESTING	-	
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT 'N' Value	DEPTH (m)	ELEVATION (m)	PenetrationTesting ○ SPT DCPT MTO Vane* Nilcon Vane* △ Intact > Intact ▲ Remould Remould Undrained Shear Strength (kPa) (from P. Penetrometer tests) 20 20 40 60	Atterberg Limits Wp W ■ ● Plastic Liquid ★ Passing 75 um (%) 0 O Moisture Content (%) • ★ Unit Weight (KN/m3) 20 40 60 80	INSTRUMENTATION INSTALLATION	COMMENTS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
	END OF BOREHOLE 22.9					- - 23 - - -					
						- - 24 - - - -					
						- - - 25 - - - -					
						- - - 26 - - -					
						- - - - - - - -					
						- - - - - - - - - -					
						- - - - - - - - -					
-						- - - - - - -					
En 118 Wir Tel Fax	365 County Road 42							e upon completion of drilling tanding of all potential conditions n conjunction with the geotechnic	present and requires interpretive a al report for which it was commissi	ssistance f oned and tl	rom a qualified 10 Page: 4 of 4









METHOD OF SOIL CLASSIFICATION

Organic or Inorganic	Soil Group	Type of	f Soil	Gradation or Plasticity	Cu	$=\frac{D_{60}}{D_{10}}$		$Cc = \frac{(D)}{D_{10}}$	$(xD_{60})^2$	Organic Content	USCS Group Symbol	Group Name		
	_	nm) is	Gravels with ≤12%	Poorly Graded		<4		≤1 or 3	≥3		GP	GRAVEL		
(ss	5 mm)	VELS / mass action 14.75 I	fines (by mass)	Well Graded		≥4		1 to 3	3		GW	GRAVEL		
by ma	SOILS an 0.07	GRAVELS (>50% by mass of coarse fraction is larger than 4.75 mm)	Gravels with >12%	Below A Line			n/a				GM	SILTY GRAVEL		
SANIC t ≤30%	AINED rger th		fines (by mass)	Above A Line			n/a			<20%	GC	CLAYEY GRAVEL		
INORGANIC (Organic Content ≤30% by mass)	COARSE-GRAINED SOILS (>50% by mass is larger than 0.075 mm)	of is mm)	Sands with ≤12%	Poorly Graded		<6		≤1 or i	≥3	≤30%	SP	SAND		
ganic (COARS by mai	SANDS 6 by mass se fraction than 4.75	fines (by mass)	Well Graded		≥6		1 to 3	3		SW	SAND		
(Or	(>50%	SANDS (≥50% by mass of coarse fraction is smaller than 4.75 mm)	Sands with	Below A Line			n/a				SM	SILTY SAND		
		smal	>12% fines (by mass)	Above A Line			n/a				SC	CLAYEY SAND		
Organic						I	Field Indica	tors						
or Inorganic	Soil Group	Type of	f Soil	Laboratory Tests	Dilatancy	Dry Strength	Shine Test	Thread Diameter	Toughness (of 3 mm thread)	Organic Content	Symbol	Primary Name		
		- plot		I founded to be to	Rapid	None	None	>6 mm	N/A (can't roll 3 mm thread)	<5%	ML	SILT		
	5 mm)	and LL	sity ow)	Liquid Limit <50	Slow	None to Low	Dull	3mm to 6 mm	None to low	<5%	ML	CLAYEY SIL		
by ma	OILS an 0.07	SILTS SILTS (Non-Plastic or Pl and LL plot bolow A 1 inc.	below A-Line on Plasticity Chart below)		Slow to very slow	Low to medium	Dull to slight	3mm to 6 mm	Low	5% to 30%	OL	ORGANIC SILT		
INORGANIC ic Content ≤30% by m	JED SC aller th	ו-Plasti	e e e	Liquid Limit	Slow to very slow	Low to medium	Slight	3mm to 6 mm	Low to medium	<5%	МН	CLAYEY SIL		
	FINE-GRAINED SOILS mass is smaller than 0	(Nor		≥50	None	Medium to high	Dull to slight	1 mm to 3 mm	Medium to high	5% to 30%	ОН	ORGANIC SILT		
ganic (FINE by mas	olot	ant art	Liquid Limit <30	None	Low to medium	Slight to shiny	~ 3 mm	Low to medium	0%	CL	SILTY CLAY		
D.	FINE-GRAINED SOILS (250% by mass is smaller than 0.075 mm)	CLAYS and LL p	above A-Line on Plasticity Chart below)	Liquid Limit 30 to 50	None	Medium to high	Slight to shiny	1 mm to 3 mm	Medium	to 30%	CI	SILTY CLAY		
		(Pla	Plast	Liquid Limit ≥50	None	High	Shiny	<1 mm	High	(see Note 2)	Symbol GP GW GM GC SP SW SM SC USCS Group Symbol ML OL MH OL CL CI CH CH CH PT two symbols SW-SC and C ymbols must H 12% fines (i. lean" and "d	CLAY		
		Peat and mi mixtu					30% to 75%		SILTY PEAT SANDY PEA					
HIGHLY ORGANIC SOILS	by mai	Predominar may conta mineral soil, amorpho	in some fibrous or							75% to 100%	PT	PEAT		
40 30 ((d) X4	40 Low Plasticity Medium Plasticity High Plasticity all Foc 30 CLAY CH CH GROWTH Transformed Foc								Dual Symbol — A dual symbol is two symbols separated by a hyphen, for example, GP-GM, SW-SC and CL-ML. For non-cohesive soils, the dual symbols must be used when the soil has between 5% and 12% fines (i.e. to identify transitional material between "clean" and "dirty" sand or gravel. For cohesive soils, the dual symbol must be used when the					
Plasticity Index (PI) 05 -				Aline				liquid limit and plasticity index values plot in the CL-ML are of the plasticity chart (see Plasticity Chart at left).						

Borderline Symbol — A borderline symbol is two symbols separated by a slash, for example, CL/CI, GM/SM, CL/ML. A borderline symbol should be used to indicate that the soil has been identified as having properties that are on the transition between similar materials. In addition, a borderline symbol may be used to indicate a range of similar soil types within a stratum.

Liquid Limit (LL) Note 1 - Fine grained materials with PI and LL that plot in this area are named (ML) SILT with slight plasticity. Fine-grained materials which are non-plastic (i.e. a PL cannot be measured) are named SILT.

CLAYEY SILT ML ORGANIC SILT OL

SILTY CLAY

20 25.5

SILTY CLAY-CLAYEY SILT, CL-MI

10

SILT ML (See Note 1)

Note 2 – For soils with <5% organic content, include the descriptor "trace organics" for soils with between 5% and 30% organic content include the prefix "organic" before the Primary name.



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ABBREVIATIONS AND TERMS USED ON RECORDS OF BOREHOLES AND TEST PITS

PARTICLE SIZES OF CONSTITUENTS

Soil Constituent	Particle Size Description	Millimetres	Inches (US Std. Sieve Size)								
BOULDERS	Not Applicable	>300	>12								
COBBLES	Not Applicable	75 to 300	3 to 12								
GRAVEL	Coarse Fine	19 to 75 4.75 to 19	0.75 to 3 (4) to 0.75								
SAND	Coarse Medium Fine	2.00 to 4.75 0.425 to 2.00 0.075 to 0.425	(10) to (4) (40) to (10) (200) to (40)								
SILT/CLAY	Classified by plasticity	<0.075	< (200)								

MODIFIERS FOR SECONDARY AND MINOR CONSTITUENTS

Percentage by Mass	Modifier
>35	Use 'and' to combine major constituents (<i>i.e.</i> , SAND and GRAVEL, SAND and CLAY)
> 12 to 35	Primary soil name prefixed with "gravelly, sandy, SILTY, CLAYEY" as applicable
> 5 to 12	some
≤ 5	trace

PENETRATION RESISTANCE

Standard Penetration Resistance (SPT), N:

The number of blows by a 63.5 kg (140 lb) hammer dropped 760 mm (30 in.) required to drive a 50 mm (2 in.) split-spoon sampler for a distance of 300 mm (12 in.).

Cone Penetration Test (CPT)

An electronic cone penetrometer with a 60° conical tip and a project end area of 10 cm² pushed through ground at a penetration rate of 2 cm/s. Measurements of tip resistance (q_t), porewater pressure (u) and sleeve frictions are recorded electronically at 25 mm penetration intervals.

Dynamic Cone Penetration Resistance (DCPT); Nd:

The number of blows by a 63.5 kg (140 lb) hammer dropped 760 mm (30 in.) to drive uncased a 50 mm (2 in.) diameter, 60° cone attached to "A" size drill rods for a distance of 300 mm (12 in.).

- PH: Sampler advanced by hydraulic pressure
- PM: Sampler advanced by manual pressure
- WH: Sampler advanced by static weight of hammer
- WR: Sampler advanced by weight of sampler and rod

NON-COHESIVE (COHESIONLESS) SOILS

	Comp	actness ²							
Term SPT 'N' (blows/0.3m) ¹									
Very Loose 0 - 4									
Loose 4 to 10									
	Compact	10 to 30							
	Dense	30 to 50							
١	/ery Dense	>50							
	Field Meint	ure Condition							
Term									
Term	L	Description							
Dry	Soil flows freely thre	ough fingers.							
Moist	Soils are darker tha may feel cool.	an in the dry condition and							
Moist Wet	may feel cool.	an in the dry condition and ree water forming on hands							

S V	MPI	ES
SA		LEG

SAMPLES	
AS	Auger sample
BS	Block sample
CS	Chunk sample
DO or DP	Seamless open ended, driven or pushed tube sampler – note size
DS	Denison type sample
FS	Foil sample
GS	Grab Sample
RC	Rock core
SC	Soil core
SS	Split spoon sampler – note size
ST	Slotted tube
ТО	Thin-walled, open – note size
TP	Thin-walled, piston – note size
WS	Wash sample

SOIL TESTS

SUIL TESTS	
w	water content
PL, w _p	plastic limit
LL, wL	liquid limit
С	consolidation (oedometer) test
CHEM	chemical analysis (refer to text)
CID	consolidated isotropically drained triaxial test ¹
CIU	consolidated isotropically undrained triaxial test with porewater pressure measurement ¹
D _R	relative density (specific gravity, Gs)
DS	direct shear test
GS	specific gravity
М	sieve analysis for particle size
МН	combined sieve and hydrometer (H) analysis
MPC	Modified Proctor compaction test
SPC	Standard Proctor compaction test
OC	organic content test
SO ₄	concentration of water-soluble sulphates
UC	unconfined compression test
UU	unconsolidated undrained triaxial test
V (FV)	field vane (LV-laboratory vane test)
γ	unit weight
1. Tests whi	ch are anisotropically consolidated prior to shear are show

Tests which are anisotropically consolidated prior to shear are shown as CAD, CAU. COHESIVE SOILS

CONLOIVE C

Consistency										
Term	Undrained Shear Strength (kPa)	SPT 'N' ^{1,2} (blows/0.3m)								
Very Soft	<12	0 to 2								
Soft	12 to 25	2 to 4								
Firm	25 to 50	4 to 8								
Stiff	50 to 100	8 to 15								
Very Stiff	100 to 200	15 to 30								
Hard	>200	>30								

 SPT 'N' in accordance with ASTM D1586, uncorrected for overburden pressure effects; approximate only.

effects; approximate only.

 SPT 'N' values should be considered ONLY an approximate guide to consistency; for sensitive clays (e.g., Champlain Sea clays), the N-value approximation for consistency terms does NOT apply. Rely on direct measurement of undrained shear strength or other manual observations.

Water Content										
Term	Description									
w < PL	Material is estimated to be drier than the Plastic Limit.									
w ~ PL	Material is estimated to be close to the Plastic Limit.									
w > PL	Material is estimated to be wetter than the Plastic Limit.									





Unless otherwise stated, the symbols employed in the report are as follows:

I.	GENERAL	(a) w	Index Properties (continued) water content
π In x Iog ₁₀ g t	3.1416 natural logarithm of x x or log x, logarithm of x to base 10 acceleration due to gravity time	w _I or LL w _p or PL I _p or PI Ws I _L IC emax emin	liquid limit plastic limit plasticity index = $(w_l - w_p)$ shrinkage limit liquidity index = $(w - w_p) / I_p$ consistency index = $(w_l - w) / I_p$ void ratio in loosest state void ratio in densest state
П.	STRESS AND STRAIN	ID	density index = $(e_{max} - e) / (e_{max} - e_{min})$ (formerly relative density)
$\gamma \Delta$	shear strain change in, e.g. in stress: $\Delta \sigma$	(b) h	Hydraulic Properties hydraulic head or potential
8 Ev n	linear strain volumetric strain coefficient of viscosity	q v i	rate of flow velocity of flow hydraulic gradient
η υ σ	Poisson's ratio total stress	k	hydraulic conductivity (coefficient of permeability)
σ΄ σ΄ _{νο}	effective stress ($\sigma' = \sigma - u$) initial effective overburden stress principal stress (major, intermediate,	j	seepage force per unit volume
01, 02, 03	minor)	(c) C _c	Consolidation (one-dimensional) compression index
σ_{oct}	mean stress or octahedral stress = $(\sigma_1 + \sigma_2 + \sigma_3)/3$	Cr	(normally consolidated range) recompression index
τ u	shear stress porewater pressure	Cs	(over-consolidated range) swelling index
E G	modulus of deformation shear modulus of deformation	Cα mv	secondary compression index coefficient of volume change
ĸ	bulk modulus of compressibility	Cv	coefficient of consolidation (vertical direction)
		Ch T	coefficient of consolidation (horizontal direction)
III.	SOIL PROPERTIES	Tv U	time factor (vertical direction) degree of consolidation
(a) ρ(γ)	Index Properties bulk density (bulk unit weight)*	σ΄ _Ρ OCR	pre-consolidation stress over-consolidation ratio = $\sigma'_{P} / \sigma'_{vo}$
ρ(γ) ρ _d (γ _d)	dry density (dry unit weight)	(d)	Shear Strength
ρw(γw) ρs(γs) γ΄	density (unit weight) of water density (unit weight) of solid particles unit weight of submerged soil	τ _ρ , τ _r φ΄ δ	peak and residual shear strength effective angle of internal friction angle of interface friction coefficient of friction = tan δ
DR	$(\gamma' = \gamma - \gamma_w)$ relative density (specific gravity) of solid particles (D _R = ρ_s / ρ_w) (formerly G _s)	μ C΄ Cu, Su	effective cohesion undrained shear strength ($\phi = 0$ analysis)
e n S	void ratio porosity degree of saturation	p p' q q _u St	mean total stress $(\sigma_1 + \sigma_3)/2$ mean effective stress $(\sigma'_1 + \sigma'_3)/2$ $(\sigma_1 - \sigma_3)/2$ or $(\sigma'_1 - \sigma'_3)/2$ compressive strength $(\sigma_1 - \sigma_3)$ sensitivity
where	ty symbol is ρ . Unit weight symbol is $\gamma = \rho g$ (i.e. mass density multiplied by eration due to gravity)	Notes: 1 2	τ = c' + σ' tan φ' shear strength = (compressive strength)/2



LOCATION: REFER TO LOCATION PLAN

RECORD OF BOREHOLE BH-101\BH-101A

BORING DATE: March 14, 2017 DRILLING CONTRACTOR: WALKER DRILLING SHEET 1 OF 3

DATUM: GEODETIC

日어	SOIL PROFILE	1.		SA	MPL	-	z	RESIS	MIC PEN TANCE,	BLOW	ION S/0.3m	l	HYDR	AULIC C k, cm/s	ONDUCT	IIVIIY,	T	βĻ	INSTALLATION
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SING	DESCRIPTION	ATA F	ELEV.	NUMBER	TYPE	BLOWS/0.3m	ELEV.	SHEA Cu, kP	R STREM	NGTH	nat V. + rem V. €	Q - ● U - O	W		ONTENT			B. TI	OBSERVATIONS
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							178												
		2 22	177.40 177:22																888
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UGER LOW STEM	(SM) SILTY FINE SAND ; brown; compact		1.07	2	ss	11	176												Enc WL
POWER AUGER 210mm OD HOLLOW STEM			174.81	4	ss	9	174												
	(CL) - SILTY CLAY , some sand, with sand lenses; grey; stiff to soft			5	ss	PH	172	•	++										
TRI-CONE WITH MUD UNCASED							170											-	Instruments in BH-101A 101ST 101SU Bottom of BH-101A
							169												
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EPTH \$: 50	SCALE						(Ĵ	G Ass	olde ocia	er ates								LOGGED: MR CHECKED: SS

LOCATION: REFER TO LOCATION PLAN

RECORD OF BOREHOLE BH-101\BH-101A

BORING DATE: March 14, 2017 DRILLING CONTRACTOR: WALKER DRILLING

SHEET 2 OF 3

DATUM: GEODETIC

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			(CL) - SILTY CLAY, some sand, some gravel, with shale fragments; grey, TILL; hard	6					159												
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LOCATION: REFER TO LOCATION PLAN

RECORD OF BOREHOLE BH-101\BH-101A

TION PLAN

BORING DATE: March 14, 2017 DRILLING CONTRACTOR: WALKER DRILLING SHEET 3 OF 3

DATUM: GEODETIC

Щ		Ę	SOIL PROFILE	1		SA	AMPL	ES	7	DYN/ RESI	AMIC STAN	PEN VCE,	ETRA1 BLOW	10N S/0.3	ßm	l	HYDR/	AULIC C k, cm/s		TIVITY,	T	QL QL	INSTALLATION
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	╉			S		┝					20	4	0	60	80	0	1	0	20 ; 	30	40		
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-	DRILING								158	(%)	(%)		(%)										
- - 2 -	MUD ROTARY DRILLING	PORC	fine grained SHALE (bedrock); grey			9	PQ RC			T.C.R. (%) ம	4 S.C.R. (%)	81	R.Q.D. (%)	7									
-	M								157														101DT
- 2	₁┝		END OF BOREHOLE		156.46						-												101DU
					20.0				156														Groundwater encountered during drilling at about 2.13 mbgs on March 14, 2017
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LOCATION: REFER TO LOCATION PLAN

RECORD OF BOREHOLE BH-102\BH-102A

BORING DATE: March 14 - 17, 2017 DRILLING CONTRACTOR: WALKER DRILLING SHEET 1 OF 3

DATUM: GEODETIC

Image: solution of the	SOIL DESCRIPTION FROM 0 TO 17.83m INFERRED FROM BH-101 178 GROUND SURFACE 177.40 TOPSOIL, clayey; dark brown 179.92	INSTALLATI AND GROUNDWA OBSERVATIO
Sol, DeSCHEFICINE FROM 61-01 177.6 GROUND SUPPACE 177.6 GROUND SUPPACE 177.6 (C). SUPP CLY, come and, with and leader, pay, off to add. 174.81 (C). SUPY CLY, some and, with and leader, pay, off to add. 174.81 (C). SUPY CLY, some and, with and leader, pay, off to add. 174.81 (C). SUPY CLY, some and, with and leader, pay, off to add. 174.81 (C). SUPY CLY, some and, with and leader, pay, off to add. 174.81 (C). SUPY CLY, some and, with and leader, pay, off to add. 174.81 (C). SUPY CLY, some and, with and leader, pay, off to add. 174.81 (C). SUPY CLY, some and, with and leader, pay, off to add. 174.81 (C). SUPY CLAY, some and, with a dd. 174.81 (C). SUPY CLAY, some and, with a dd. 174.81 (C). SUPY CLAY, some and, with a dd. 174.81 (C). SUPY CLAY, some and, with a dd. 175.81 (C). SUPY CLAY, some and, with a dd. 175.81 (C). SUPY CLAY, some and, with a dd. 175.81 (C). SUPY CLAY, some and, with a dd. 175.81 (C). SUPY CLAY, some and, with a dd. 175.81 (C). SUPY CLAY, some and, with a dd. 177.9	SOIL DESCRIPTION FROM 0 TO 17.83m INFERRED FROM BH-101 178 GROUND SURFACE 177.40 TOPSOIL, clayey; dark brown 4,54,5,177.22	OBSERVATI
Sol, DeSCHEFICINE FROM 61-01 177.6 GROUND SUPPACE 177.6 GROUND SUPPACE 177.6 (C). SUPP CLY, come and, with and leader, pay, off to add. 174.81 (C). SUPY CLY, some and, with and leader, pay, off to add. 174.81 (C). SUPY CLY, some and, with and leader, pay, off to add. 174.81 (C). SUPY CLY, some and, with and leader, pay, off to add. 174.81 (C). SUPY CLY, some and, with and leader, pay, off to add. 174.81 (C). SUPY CLY, some and, with and leader, pay, off to add. 174.81 (C). SUPY CLY, some and, with and leader, pay, off to add. 174.81 (C). SUPY CLY, some and, with and leader, pay, off to add. 174.81 (C). SUPY CLAY, some and, with a dd. 174.81 (C). SUPY CLAY, some and, with a dd. 174.81 (C). SUPY CLAY, some and, with a dd. 174.81 (C). SUPY CLAY, some and, with a dd. 175.81 (C). SUPY CLAY, some and, with a dd. 175.81 (C). SUPY CLAY, some and, with a dd. 175.81 (C). SUPY CLAY, some and, with a dd. 175.81 (C). SUPY CLAY, some and, with a dd. 175.81 (C). SUPY CLAY, some and, with a dd. 177.9	SOIL DESCRIPTION FROM 0 TO 17.83m INFERRED FROM BH-101 178 GROUND SURFACE 177.40 TOPSOIL, clayey; dark brown 4,54,5,177.22	
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17.8bm NFERRED FROM BH 101 179 GROUND SURFACE 177.4bm TOPSOL, Surgar, CE 177.4bm (C), SULTY CLAY, some sand; brown 178 (C), SULTY FRE SAND, brown; 178 (C), SULTY CLAY, some sand; with and kreas; gray; suff to soft. 178 (C), SULTY CLAY, some sand; with and kreas; gray; suff to soft. 179 (C), SULTY CLAY, some sand; with and kreas; gray; suff to soft. 179 (C), SULTY CLAY, some sand; with and kreas; gray; suff to soft. 179 (C), SULTY CLAY, some sand; with and kreas; gray; suff to soft. 179 (C), SULTY CLAY, some sand; with and kreas; gray; suff to soft. 179 (C), SULTY CLAY, some sand; with and kreas; gray; suff to soft. 179 (C), SULTY CLAY, some sand; with and kreas; gray; suff to soft. 179	17.83m INFERRED FROM BH-101 1/8 GROUND SURFACE 177.40 TOPSOIL, clayey; dark brown 177.22	
TOPBOL carey: dark rown 5.5 17928 (CL) SILTY CLAY, some sand, brown 17033 177 170 170 (SM) SILTY FRE SAND; trown: 17033 177 170 170 170 (SM) SILTY FRE SAND; trown: 178 51 177 170 170 170 170 (SM) SILTY FRE SAND; trown: 178 51 177 170 170 170 170 (SM) SILTY FRE SAND; trown: 178 51 177 170	TOPSOIL, clayey; dark brown 4, 17.22	
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Image: Construction of the second s	174.81 2.59	
Image: clipped with send lenses, grey, stiff to soft 173 172 172 172 171 172 172 172 172 172 172 171 172 172 172 172 172 172 172 171 172 172 172 172 172 171 172 173 174 175 172 172 170 170 170 170 172 172 172 172 170 170 170 172		
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(CL) - SILTY CLAY, some sand, with sand lenses; grey, stiff to soft		
sand lenses; grey; stiff to soft		
NUMBROOT	sand lenses; grey; stiff to soft	
BH-102A 170 169 169 169 169 169 169 169 169		
BH-102A 170 169 169 169 169 169 169 169 169		
Bottom of BH-102A		BH-102A 102ST
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LOCATION: REFER TO LOCATION PLAN

RECORD OF BOREHOLE BH-102\BH-102A

BORING DATE: March 14 - 17, 2017 DRILLING CONTRACTOR: WALKER DRILLING SHEET 2 OF 3

DATUM: GEODETIC

METRES		SOIL PROFILE			SA	MPL		z	DYNA RESIS	MIC PEN STANCE,	BLOW	ION S/0.3m	l	HYDR/	AULIC Co k, cm/s		TIVITY,	T	μĜ	INSTALLATION
			STRATA PLOT		۲.		J.3m	ELEVATION					30					10 ⁻³	ADDITIONAL LAB. TESTING	AND GROUNDWATE
	או	DESCRIPTION	VTA F	ELEV. DEPTH	NUMBER	TYPE	BLOWS/0.3m	ILEV,	SHEA Cu, kF	R STREM	NGTH	nat V. + rem V. ⊕	Q - ● U - O	W	ATER C				B. TE	OBSERVATION
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		(CL) - SILTY CLAY , some sand, with sand lenses; grey; stiff to soft	K	ų –				164										──		
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LOCATION: REFER TO LOCATION PLAN

RECORD OF BOREHOLE BH-102\BH-102A

BORING DATE: March 14 - 17, 2017 DRILLING CONTRACTOR: WALKER DRILLING SHEET 3 OF 3

DATUM: GEODETIC

	Γ	ш	DO	SOIL PROFILE			SA	MPL	.ES		DYNA	MIC PEN TANCE,		ON 3/0.3m		HYDR	AULIC C k, cm/s	ONDUCT	FIVITY,	Т	. (7)	
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RECORD OF BOREHOLE BH-103\BH-103A

BORING DATE: March 14 - 18, 2017 DRILLING CONTRACTOR: WALKER DRILLING

SHEET 1 OF 3

DATUM: GEODETIC

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LOCATION: REFER TO LOCATION PLAN

RECORD OF BOREHOLE BH-103\BH-103A

BORING DATE: March 14 - 18, 2017 DRILLING CONTRACTOR: WALKER DRILLING SHEET 2 OF 3

DATUM: GEODETIC

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RECORD OF BOREHOLE BH-103\BH-103A

BORING DATE: March 14 - 18, 2017 DRILLING CONTRACTOR: WALKER DRILLING SHEET 3 OF 3

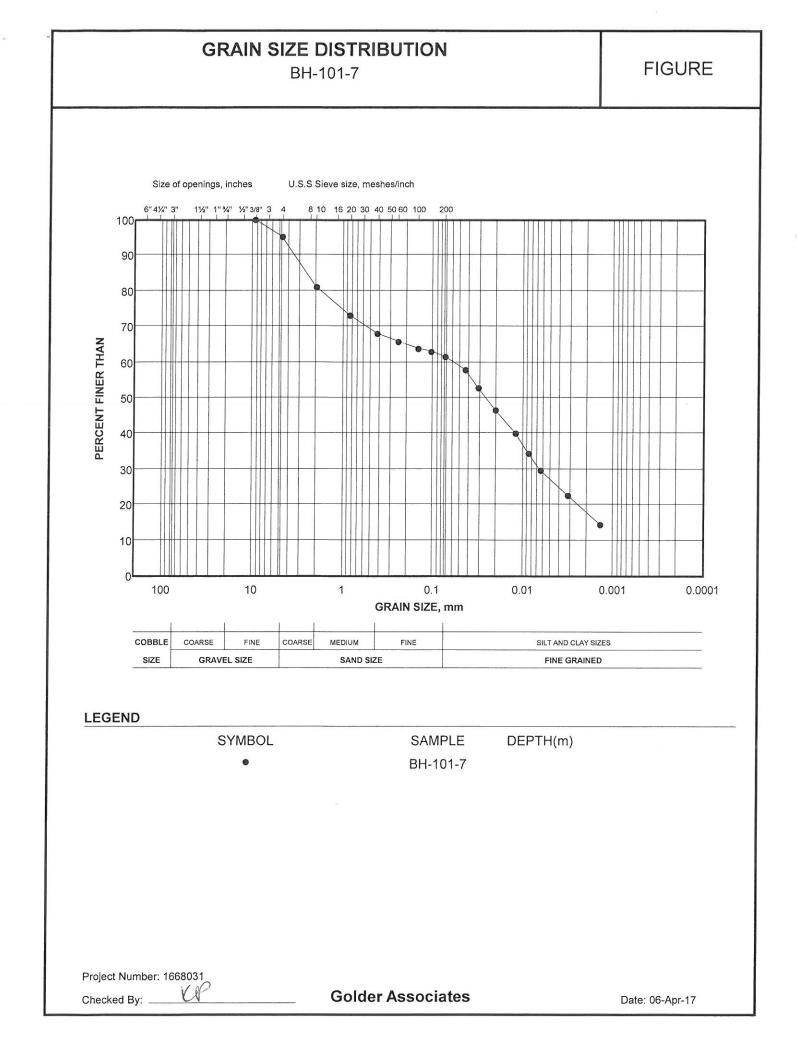
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SOIL SIEVE AND HYDROMETER ANALYSIS

Initial weight of dry sample Weight measured for back sieving Weight of Sample for Hydrometer = 285(g) = 50.09(g) = 50.09(g)

COARSE SIEVING

SIEVE	CUM. MASS RETAINED (g)	% RETAINED	PARTICLE SIZE(mm)	% PASSING
75mm	0.00	0.00	75.00	100.0
63mm	0.00	0.00	63.00	100.0
53mm	0.00	0.00	53.00	100.0
37.5mm	0.00	0.00	37.50	100.0
26.5mm	0.00	0.00	26.50	100.0
19.0mm	0.00	0.00	19.00	100.0
13.2mm	0.00	0.00	13.20	100.0
9.5mm	0.00	0.00	9.50	100.0
4.75mm	14.00	4.91	4.75	95.1
2.00mm	53.55	13.88	2.00	81.2
PAN	231.45	81.21	0.00	0.0

HYDROMETER BACK SIEVING

SIEVE	CUM. MASS RETAINED (g)	% RETAINED	PARTICLE SIZE(mm)	% PASSING
850µm	4.97	8.06	0.85	73.2
425µm	8.12	5.11	0.43	68.0
250µm	9.59	2.38	0.25	65.7
150µm	10.73	1.85	0.15	63.8
106µm	11.23	0.81	0.11	63.0
75µm	12.04	1.31	0.08	61.7

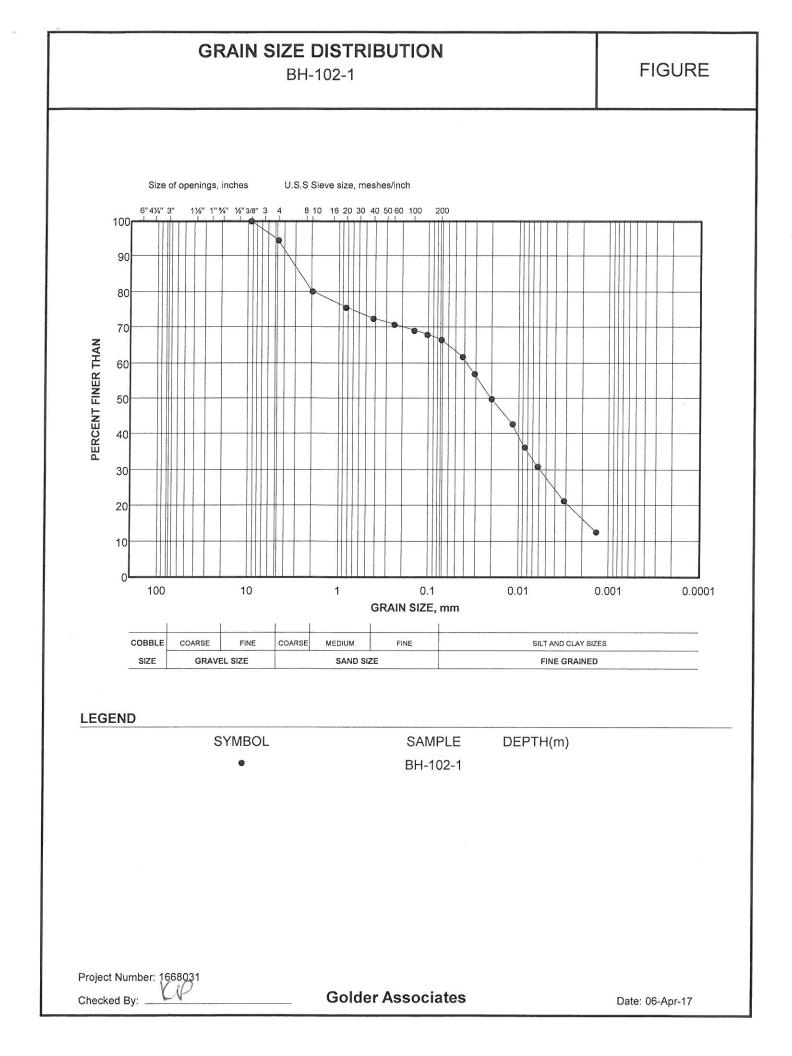
HYDROMETER

Started : Finished :	DATE (MM\DD\YYYY) 4/6/2017 4/6/2017	TIME (HH:MM:SS) Null Null					
Elapsed Time (min)	HYDROMETER READING	DEFLOCCULANT CORRECTION	WATER TEMP (°C)	CORRECTED HYDROMETER READING	PARTICLE SIZE (mm)	% PASSING	PLOT
1.00 2.00 5.00 15.00 30.00 60.00 250.00 1440.00	40.00 37.00 33.00 29.00 25.50 22.50 18.00 13.00	4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0	22.6 22.6 22.6 22.6 22.6 22.6 22.6 22.6	36.00 33.00 29.00 25.00 21.50 18.50 14.00 9.00	0.0445 0.0321 0.0208 0.0123 0.0088 0.0064 0.0032 0.0014	57.8 53.0 46.5 40.1 34.5 29.7 22.5 14.4	True True True True True True True
Project Numb Project Task	1000			Depth Units Testing Date		Metric 4/6/2017 11:39:25 AI	vi

Sample Number Checked By BH-101-7

Units Testing Date Tested By LabID Metric 4/6/2017 11:39:25 AM Sieve - KP, Hydrometer - KP 17-155

Golder Associates



Initial weight of dry sample Weight measured for back sieving Weight of Sample for Hydrometer

= 186.33(g) = 50.01(g) = 50.01(g)

COARSE SIEVING

SIEVE	CUM. MASS RETAINED (g)	% RETAINED	PARTICLE SIZE(mm)	% PASSING
75mm	0.00	0.00	75.00	100.0
63mm	0.00	0.00	63.00	100.0
53mm	0.00	0.00	53.00	100.0
37.5mm	0.00	0.00	37.50	100.0
26.5mm	0.00	0.00	26.50	100.0
19.0mm	0.00	0.00	19.00	100.0
13.2mm	0.00	0.00	13.20	100.0
9.5mm	0.00	0.00	9.50	100.0
4.75mm	10.15	5.45	4.75	94.6
2.00mm	36.82	14.31	2.00	80.2
PAN	149.51	80.24	0.00	0.0

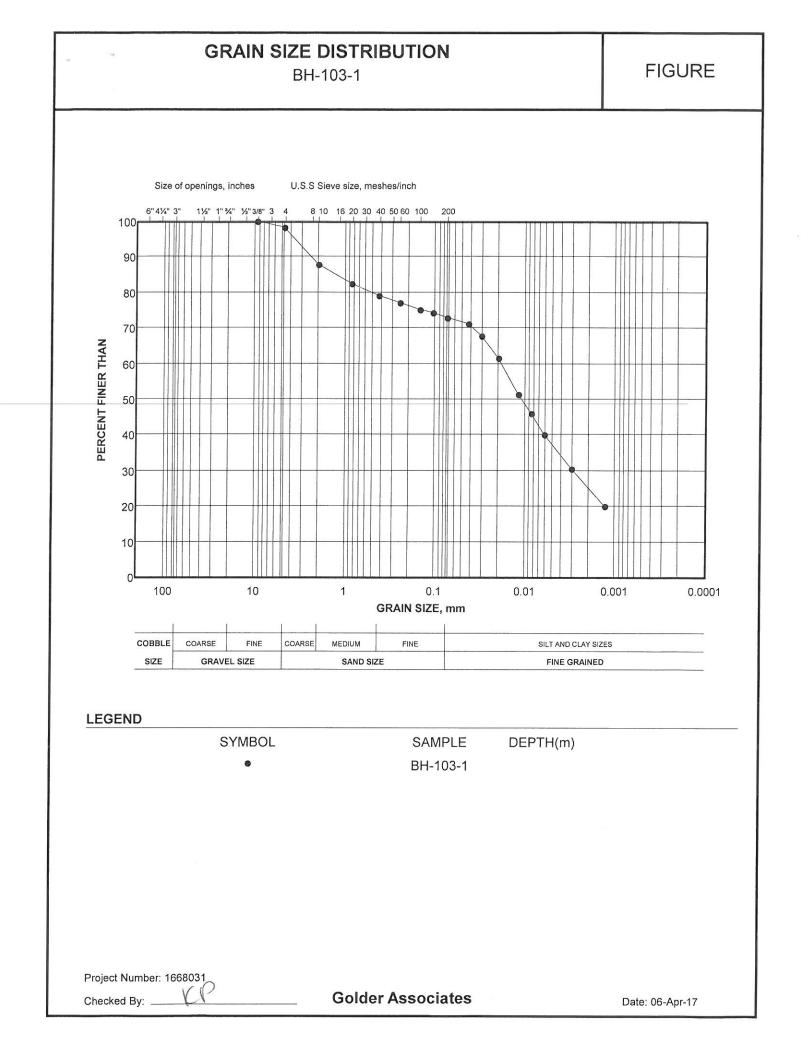
HYDROMETER BACK SIEVING

SIEVE	CUM. MASS RETAINED (g)	% RETAINED	PARTICLE SIZE(mm)	% PASSING
850µm	2.85	4.57	0.85	75.7
425µm	4.68	2.94	0.43	72.7
250µm	5.82	1.83	0.25	70.9
150µm	6.93	1.78	0.15	69.1
106µm	7.57	1.03	0.11	68.1
75µm	8.44	1.40	0.08	66.7

HYDROMETER

Project Numb Project Task Sample Numb Checked By	1	668031 1000 H-102-1		Depth Units Testing Date Tested By LabID	Ð	Metric 4/6/2017 11:37:30 Al Sieve - KP, Hydrome 17-156	5.5 (C.S.)	
1.00 2.00 5.00 15.00 30.00 60.00 250.00 1440.00	43.00 40.00 35.50 31.00 27.00 23.50 17.50 12.00	4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0	22.6 22.6 22.6 22.6 22.6 22.6 22.6 22.6	39.00 36.00 31.50 27.00 23.00 19.50 13.50 8.00	0.0436 0.0315 0.0205 0.0121 0.0088 0.0063 0.0032 0.0014	61.9 57.2 50.0 42.9 36.5 31.0 21.4 12.7	True True True True True True True	
Started : Finished : Elapsed Time (min)	(MM\DD\YYYY 4/6/2017 4/6/2017 HYDROMETEF READING	Null Null	WATER TEMP (°C)	CORRECTED HYDROMETER READING	PARTICLE SIZE (mm)	% PASSING	PLOT	
Finished :	4/6/2017	Null Null						

Golder Associates



Initial weight of dry sample Weight measured for back sieving Weight of Sample for Hydrometer = 207.6(g) = 50.06(g) = 50.06(g)

COARSE SIEVING

SIEVE	CUM. MASS RETAINED (g)	% RETAINED	PARTICLE SIZE(mm)	% PASSING
75mm	0.00	0.00	75.00	100.0
63mm	0.00	0.00	63.00	100.0
53mm	0.00	0.00	53.00	100.0
37.5mm	0.00	0.00	37.50	100.0
26.5mm	0.00	0.00	26.50	100.0
19.0mm	0.00	0.00	19.00	100.0
13.2mm	0.00	0.00	13.20	100.0
9.5mm	0.00	0.00	9.50	100.0
4.75mm	3.51	1.69	4.75	98.3
2.00mm	25.09	10.39	2.00	87.9
PAN	182.51	87.92	0.00	0.0

HYDROMETER BACK SIEVING

SIEVE	CUM. MASS RETAINED (g)	% RETAINED	PARTICLE SIZE(mm)	% PASSING	
850µm	3.07	5.39	0.85	82.5	
425µm	4.98	3.35	0.43	79.2	
250µm	6.13	2.02	0.25	77.2	
150µm	7.23	1.93	0.15	75.2	
106µm	7.73	0.88	0.11	74.4	
75µm	8.54	1.42	0.08	72.9	

HYDROMETER

Started: Finished:	DATE (MM\DD\YYYY 4/6/2017 4/6/2017	TIME) (HH:MM:SS) Null Null					
Elapsed Time (min)	HYDROMETER READING	R DEFLOCCULANT CORRECTION	WATER TEMP (°C)	CORRECTED HYDROMETER READING	PARTICLE SIZE (mm)	% PASSING	PLOT
1.00 2.00 5.00 15.00 30.00 60.00 250.00 1440.00	45.00 43.00 39.50 33.50 30.50 27.00 21.50 15.50	4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0	22.6 22.6 22.6 22.6 22.6 22.6 22.6 22.6	41.00 39.00 35.50 29.50 26.50 23.00 17.50 11.50	0.0430 0.0308 0.0200 0.0120 0.0086 0.0062 0.0031 0.0013	71.3 67.8 61.7 51.3 46.1 40.0 30.4 20.0	True True True True True True True
Project Numb Project Task Sample Numb Checked By	1	668031 000 H-103-1		Depth Units Testing Date Tested By LabID		Metric 4/6/2017 11:41:02 Al Sieve - KP, Hydrome 17-154	A CONTRACTOR OF A CONTRACTOR O

Golder Associates



APPENDIX E

Photographs of Instrument Installations and Monitoring





APPENDIX E SITE PHOTOGRAPHS - NKW1



Photograph 1: Accelerometers delivered to Golder office.



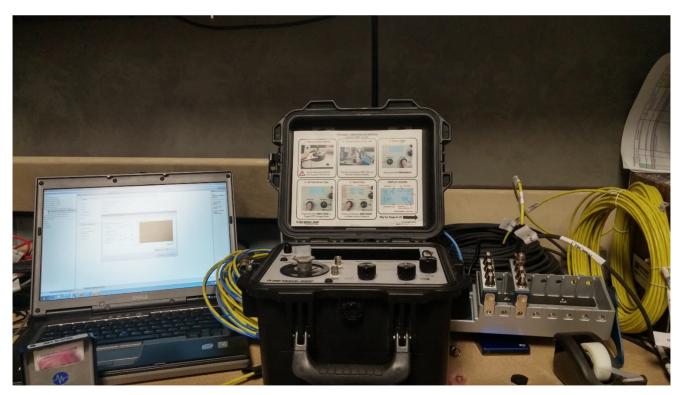
Photograph 2: Seismic uniaxial accelerometer.







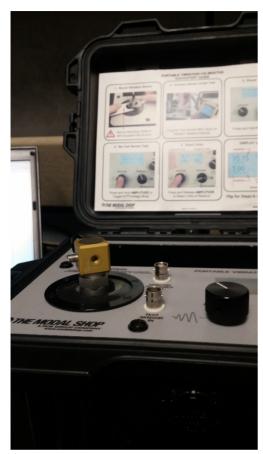
Photograph 3: Triaxial accelerometer.



Photograph 4: Bench-scale controlled-source vibration measurement system for checking sensors.







Photograph 5: Triaxial accelerometer on controlled-source vibration measurement system.



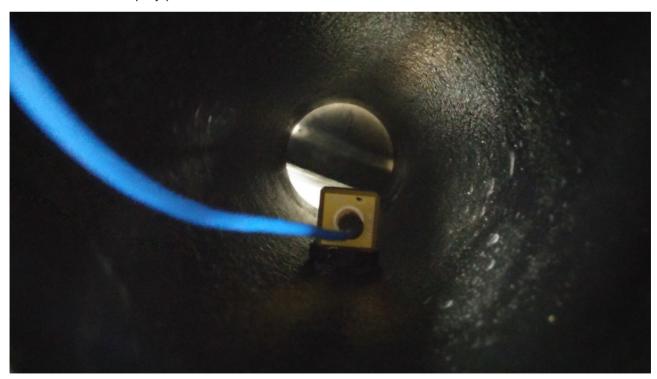
Photograph 6: Seismic accelerometer mounted on steel base of protective housing.





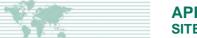


Photograph 7: Seismic accelerometer mounted on steel base of protective housing with connections coated with spray-plastic.



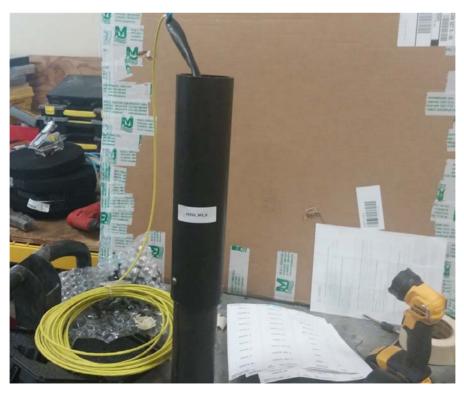
Photograph 8: Triaxial accelerometer mounted inside protective housing and with connection coated with sprayplastic prior to mounting housing to steel base.







Photograph 9: Top of protective housing, showing infilling with sand and clear plastic top cover (later sealed against water intrusion.



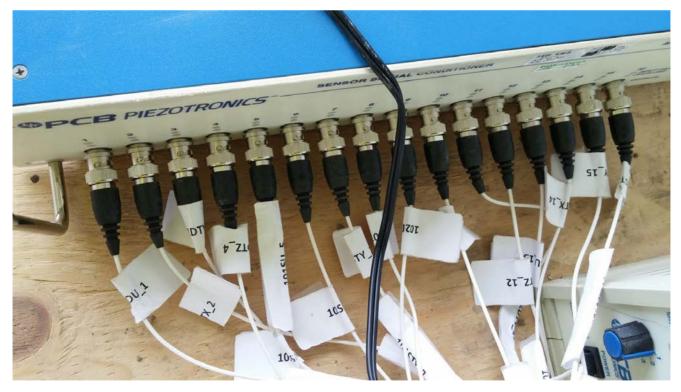
Photograph 10: Completed set of coupled sensors and housing with cables prior to field deployment.







Photograph 11: Field connection of signal conditioner/power source.



Photograph 12: Field connection of data logger.

APPENDIX E SITE PHOTOGRAPHS - NKW1



Photograph 13: Drilling for borehole installation of accelerometers at T42 site.



Photograph 14: View of boreholes at T42 site looking toward Claymore Line along timber mat roadway.







Photograph 15: Coupling of accelerometers in protective housing to drill rig, as vibration source, along with independent accelerometer field checked with controlled-source vibration measurement system (at T42 site).



Photograph 16: Close-up view of coupled accelerometers in protective housing, drill rig and independent accelerometer (at T42 site).







Photograph 17: Installation of accelerometers in borehole at T42 site.



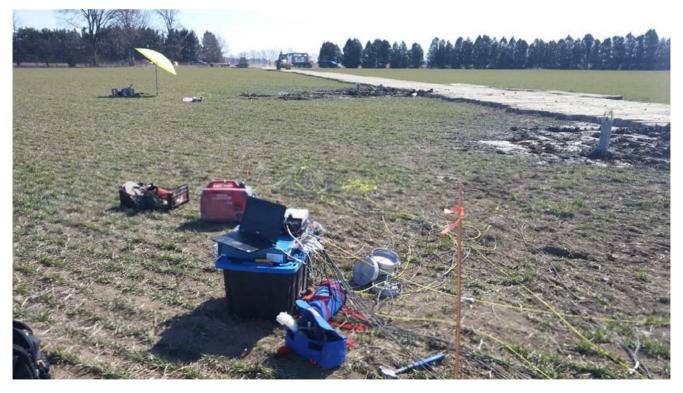
Photograph 18: Bucket immersed in grout for measuring down-hole grout quantities for coupling accelerometer housing to soil or rock in bottom of boreholes at T42 site.



APPENDIX E SITE PHOTOGRAPHS - NKW1



Photograph 19: Drill rig tracking toward Claymore Line on timber mats at T42 site.



Photograph 20: Field set up of data loggers and signal conditioner systems for testing of installed accelerometer systems (looking toward Claymore Line) at T42 site.







Photograph 21: Field set up of data loggers and signal conditioner systems at T42 site for testing of installed accelerometer systems with Instantel Minimate data logger positioned temporarily on borehole casing and geophone placed nearby (looking toward test pile location).



Photograph 22: Vacuum truck operating at T42 site to remove cuttings and fluids after completion of installation and during baseline instrument readings.







Photograph 23: Seismic accelerometer coupled to ground stake near borehole BH101/101A at T42 site after removing topsoil.



Photograph 24: Pile toe (tip) showing welded plate to close end of pile (driving shoe).







Photograph 25: Pile driving on March 29, 2017 showing first pile section driven, second pile section on ground awaiting lifting and welding.



Photograph 26: Test pile site T5, May 3, 2017 looking south, showing test pile on ground.









Photograph 27: Test pile T5 tip, May 3, 2017.







Photograph 28: Layout of surface monitoring points at T5 test pile site looking west from near test pile.







Photograph 29: Controlled-source vibration measurement system at Well #1 location.



Photograph 30: Seismic accelerometer coupled to Well #1 casing.







Photograph 31: Seismic accelerometer coupled to Well #1 casing showing data logger and Golder van.



Photograph 32: Seismic accelerometer coupled to Well #2 casing.







Photograph 33: Well #3 casing and well lid conditions (note band/hose clamp and wire leading into well through gasket).



Photograph 34: Well #3 with accelerometers mounted to casing.





APPENDIX E SITE PHOTOGRAPHS - NKW1



Photograph 35: Well #4 pump house.



Photograph 36: Well #4 well casing with accelerometers attached. Note corroded casing top and fittings.

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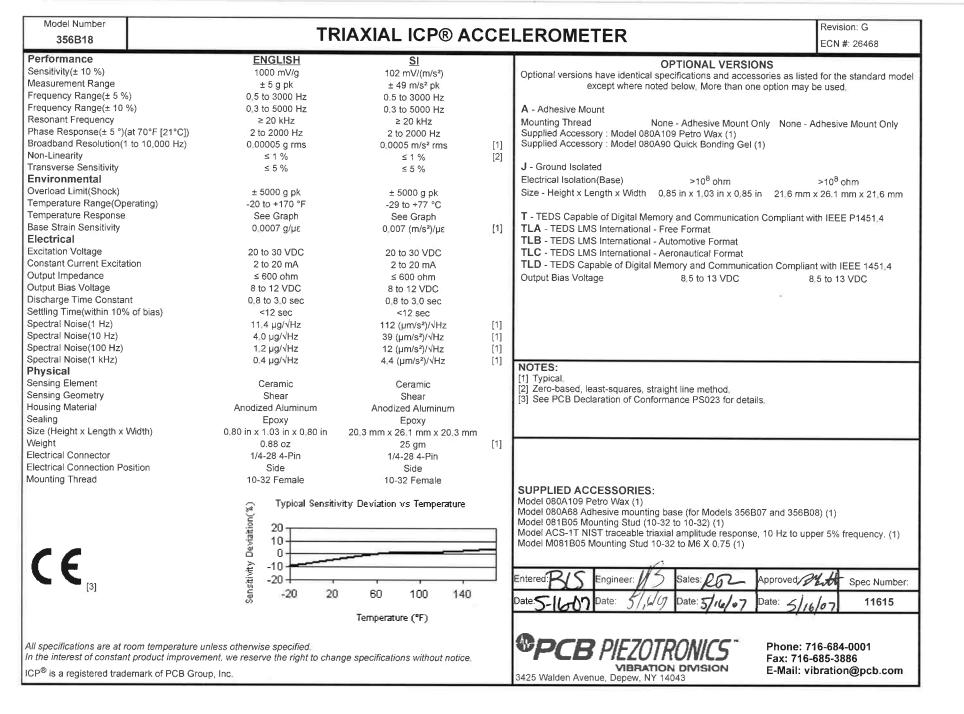


APPENDIX F

Instrument Specifications and Manufacturer's Calibration Records



Model Number				EDONETED	Revision: H
393A03	SEISN	IC ICP® AC	CEL	EROMETER	ECN #: 29751
Performance	ENGLISH	SI		OPTIONAL VERSIONS	
Sensitivity(± 5 %)	1000 mV/g	102 mV/(m/s ²)		Optional versions have identical specifications and accessories as list	ed for the standard model
Measurement Range	±5gpk	± 49 m/s² pk		except where noted below. More than one option may	
Frequency Range(± 5 %)	0.5 to 2000 Hz	0.5 to 2000 Hz			
Frequency Range(± 10 %)	0.3 to 4000 Hz	0.3 to 4000 Hz		T - TEDS Capable of Digital Memory and Communication Compliant	with IEEE P1451.4
Frequency Range(± 3 dB)	0.2 to 6000 Hz	0.2 to 6000 Hz		Output Bias Voltage 8.5 to 12.5 VDC	8.5 to 12.5 VDC
Resonant Frequency	≥ 10 kHz	≥ 10 kHz		· · ·	80
Broadband Resolution(1 to 10,000 Hz)	0.00001 g rms	0.0001 m/s ² rms	[1]		
Non-Linearity	≤ 1 %	≤1%	[2]		
Transverse Sensitivity	≤ 7 %	≤ 7 %			
Environmental					
Overload Limit(Shock)	± 5000 g pk	± 49,050 m/s² pk			
Temperature Range	-65 to +250 °F	-54 to +121 °C			
Temperature Response	See Graph	See Graph			
Base Strain Sensitivity	≤ 0.0005 g/με	≤ 0.005 (m/s²)/με	[1]	NOTES:	
Electrical				[1] Typical.[2] Zero-based, least-squares, straight line method.	
Excitation Voltage	18 to 30 VDC	18 to 30 VDC		[3] See PCB Declaration of Conformance PS023 for details.	
Constant Current Excitation	2 to 20 mA	2 to 20 mA			
Output Impedance	<250 ohm	<250 ohm			
Output Bias Voltage	8 to 12 VDC	8 to 12 VDC			
Discharge Time Constant	1 to 3 sec	1 to 3 sec			
Settling Time	<15 sec	<15 sec	[4]		
Spectral Noise(1 Hz)	2 µg/√Hz	20 (µm/sec ²)/√Hz	[1]		
Spectral Noise(10 Hz)	0.5 µg/√Hz	5 (µm/sec ²)/√Hz	[1]		
Spectral Noise(100 Hz)	0.2 µg/√Hz	2 (µm/sec ²)/√Hz	[1]		
Spectral Noise(1 kHz)	0.1 µg/√Hz	1 (µm/sec ²)/√Hz	[1]		
Electrical Isolation(Case)	≥ 10 ⁸ ohm	≥ 10 ⁸ ohm			
Physical					1
Sensing Element	Ceramic	Ceramic			
Sensing Geometry	Shear	Shear			
Housing Material	Stainless Steel Hermetic	Stainless Steel Hermetic			
Sealing Size (Hex x Height)	1 3/16 in x 2 3/16 in	30.2 mm x 55.6 mm			
Weight	7,4 oz	210 gm	[1]		
Electrical Connector	2-Pin MIL-C-5015	2-Pin MIL-C-5015	191		
Electrical Connection Position					
Mounting Thread	1/4-28 Female	1/4-28 Female			
Mounting Torque	2 to 5 ft-lb	3 to 7 N-m		SUPPLIED ACCESSORIES:	
inounting ronquo				Model 081B20 Mounting Stud, with shoulder (1/4-28 to 1/4-28) (1)	
	 Typical Sensitivity Dev 	viation vs Temperature		Model 085A31 Protective Thermal Jacket (1)	
		<i>.</i> 0.		Model ACS-1 NIST traceable frequency response (10 Hz to upper 5% Model ACS-4 Single axis, low frequency phase and amplitude respon	point). (1)
	÷≣ 20 ,			(1)	se cal ironi 0.5 to 10 Hz
	······································			Model M081B20 Mounting Stud 1/4-28 to M6 X 0.75 (1)	
	ä 0+			, , , , , , , , , , , , , , , , , , ,	
	.10		_		
				Entered: 14 Engineer: 34 Sales: WDC Approved: {	Spec Number:
[3]	រគ្គ -20 1 ទ្លូ -70 -30 10 50) 90 130 170 21	0 250	Date: 12-2-08 Date: 11-24-08 Date: 11-24-08 Date: 12-	393-1030-80
		nperature (°F)			
All specifications are at room temperature	unless otherwise specified				716-684-0001
In the interest of constant product improve	ement, we reserve the right to change spe	cifications without notice.		Fax: 71	6-685-3886
ICP [®] is a registered trademark of PCB G				VIBRATION DIVISION E-Mail: 3425 Walden Avenue, Depew, NY 14043	vibration@pcb.com
Tor to a registered trademark of r ob G	oup, no.			3420 Walden Avenue, Depew, NT 14040	



Blastmate III[™] Minimate Plus [™] Minimate Blaster[™]

Sensors Cont'd



Standard Geophone

Measures ground motion in three orthogonal directions. The sensor includes three ground spikes for soft surfaces or can be bolted to hard surfaces.

PART NUMBER	714A9701
RESPONSE STANDARD	ISEE SPECIFICATION 2000 EDITION
FREQUENCY RANGE	2 TO 250 Hz
VELOCITY RANGE	UP TO 10 in/s (254 mm/s)
RESOLUTION	0.000625in/s (0.0159mm/s)
SENSOR DENSITY	133 lbs/ft ³ (2.13 g/cc)
CABLE LENGTH	6 ft (2 m)
MAXIMUM CABLE LENGTH	250 ft (75 m)
REQUIRED SOFTWARE	BLASTWARE COMPLIANCE

DIN Geophone

Measures ground motion in three orthogonal directions. The sensor includes three ground spikes for soft surfaces or can be bolted to hard surfaces.

PART NUMBER	718A3301	
RESPONSE STANDARD	DIN 45669-1 CLASS 1	
FREQUENCY RANGE	1 TO 315 Hz	
VELOCITY RANGE	UP TO 10 in/s (254 mm/s)	
RESOLUTION	0.000625in/s (0.0159mm/s)	
SENSOR DENSITY	133 lbs/ft ³ (2.13 g/cc)	
CABLE LENGTH	6 ft (2 m)	
MAXIMUM CABLE LENGTH	3250 ft (1000 m)	
REQUIRED SOFTWARE	BLASTWARE COMPLIANCE	







Triaxial High Frequency Geophone

Measure high frequency, high amplitude vibrations in three orthogonal directions. Designed for near field monitoring of blasting activities.

Not available for Minimate Blaster.

PART NUMBER	714A9101	
FREQUENCY RANGE	30 TO 1000 Hz	
VELOCITY RANGE	UP TO 100in/s (2540mm/s)	
RESOLUTION	0.05in/s (1.27mm/s)	
SENSOR DENSITY	145 lbs/ft ³ (2.33 g/cc)	
CABLE LENGTH	100 ft (30 m)	
MAXIMUM CABLE LENGTH	3250 ft (1000 m)	
REQUIRED SOFTWARE	BLASTWARE ADVANCED	

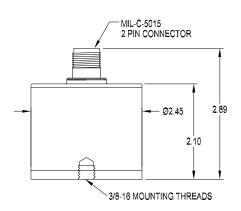




Features

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- Ultra high sensitivity
- Ultra low-noise electronics for clear signals at sub micro-g levels
- Low frequency capable
- Low pass filtered to eliminate high frequencies
- Reverse wiring protection



Model 731A Ultra-quiet, ultra low frequency, seismic accelerometer

Dynamic

Sensitivity, ±10%, 25°C	10 V/a
Acceleration range	. 10 V/y
Acceleration range	. 0.5 g peak
Amplitude nonlinearity	. 1%
Frequency response:	0.40 000.11
±10%	
±3 dB	
Resonance frequency	. 750 Hz
Transverse sensitivity, max	. 1% of axial
Temperature response:	
–10°C	12%
+65°C	
Electrical	
Power requirement: voltage source	18 - 30 VDC
i owei requirement. Vottage source	2 10 - 30 VDC
current regulating diode	. Z - TU MA
Electrical noise, equiv. g:	0.5
Broadband 2.5 Hz to 25 kHz	
Spectral 2 Hz	. 0.03 µg/V Hz
10 Hz	
100 Hz	
Output impedance, max	. 100Ω
Bias output voltage	. 9 VDC
Grounding	. case isolated

Environmental

–10 to 65°C
10 g peak
fragile
20 µg/gauss hermetic
hermetic
0.0001 g/µstrain

Physical

Sensing element design	PZT ceramic / flexure
Weight	775 grams
Case material	316L stainless steel
Mounting Output connector	2 pin, MIL-C-5015 style
Mating connector	R6 type
Mating connector Recommended cabling	J9 / J9T2A

Connector pin	Function
Shell	ground
A	power/ signal
B	common

Note: Special handling required due to sensitivity, wooden protective case included Accessories supplied: SF7 mounting stud; calibration data (level 3) Options: Power unit/amplifier P31

> Wilcoxon Research Inc 20511 Seneca Meadows Parkway Germantown, MD 20876 USA

Tel: 301 330 8811 Fax: 301 330 8873 Email: wilcoxon@meggitt.com

www.meggitt.com



Due to continued research and product development, Wilcoxon Research reserves the right to amend this specification without notice.

98078 Rev.C.6 03/10

Wilcoxon Research

Calibration Data Card SHEAR ACCELEROMETER MODEL #: 356B18 SERIAL #: W218952 (z axis)	Calibration Data Card SHEAR ACCELEROMETER MODEL #: 356B18 SERIAL #:LW218952 (y axis)	Calibration Data Calibration Data Calibration Data Calibration Data Calibration Data Calibration Construction
SENSITIVITY: <u>996</u> mV/g (101.6 mV/m/s ²)	SENSITIVITY: <u>1021</u> mV/g (104.1 mV/m/s ²)	SENSITIVITY: <u>1037</u> mV/ (105.7 mV/m/s
BIAS LEVEL: <u>11.0</u> VDC Date: <u>2/15/17</u> By: <u>C. Kinyon</u> For further information, please refer to the accompanying calibration certificate. CPCB PIEZOTRONICS 3425 WALDEN AVE · DEPEW, NY 14043 888-684-0013	BIAS LEVEL: <u>11.0</u> VDC Date: <u>2/15/17</u> By: <u>C. Kinyon</u> For further Information, please refer to the accompanying calibration certificate. C. Kinyon Sacompanying calibration certificate. Sacompanying calibration certificate. Sacompanying calibration certificate. Sacompanying calibration certificate.	BIAS LEVEL: VD Date: 2/15/17 By: C. Kinyon For further information, please refer to Hi accompanying calibration certificate. PCB PLEZOTRONLCS 3425 WALDEN AVE - DEPEW, NY 14043 888-684-0013
Calibration Data Card SHEAR ACCELEROMETER MODEL #:356B18 SERIAL #:LW218950 (z axis) SENSITIVITY: 987 mV/g	Calibration Data Card SHEAR ACCELEROMETER MODEL #: 356B18 SERIAL #:LW218950 (y axis) SENSITIVITY: 984 mV/g	Calibration Data Calibration Data Calibration Data Calibration Data Calibration Data Calibration Constraints (Constraints)
(100.7 mV/m/s ²)	(100.4 mV/m/s ²) BIAS LEVEL:11.0 VDC	BIAS LEVEL: 10.9 VD
BIAS LEVEL: <u>11.0</u> VDC Date: <u>2/15/17</u> By: <u>C. Kinyon</u> For further information, please refer to the accompanying calibration certificate. CPCE PIEZOTRONICS 3425 WALDEN AVE •: DEFEW, N14043 888-884-0013	Date: 2/15/17 By: C. Kinyon For further Information, please refer to the accompanying calibration certificate. CPCB PIEZOTRONICS 3425 WALDEN AVE - DEPEW, NY 14043 888-684-0013	Date: <u>2/15/17</u> By: <u>C. Kinyon</u> For further information, please refer to the accompanying calibration certificate. C. B. PIEZOTRONICS 3425 WALDEN AVE - DEPEW, NY 14043 888-684-0013
Calibration Data Card SHEAR ACCELEROMETER	Calibration Data Card SHEAR ACCELEROMETER MODEL #: 356B18	Calibration Data Caro SHEAR ACCELEROMETER MODEL #: 356B18 SERIAL #:LW218953 (x axis)

₽

SENSITIVITY: 967 mV/g (98.6 mV/m/s²)

SENSITIVITY: 1049

Date: 2/15/17 By: ____

BIAS LEVEL: 11.0 VDC

For further information, please refer to the accompanying calibration certificate.

mV/g

(107.0 mV/m/s²)

C. Kinyon

BIAS LEVEL: <u>11.0</u> VDC Date: <u>2/15/17</u> By: <u>C. Kinyon</u> (101.2 mV/m/s²)

BIAS LEVEL: 10.9 VDC

For further information, please refer to the accompanying calibration certificate.

Date: 2/15/17 By: C. Kinyon

For further information, please refer to the accompanying calibration certificate.

3425 WALDEN AVE - DEPEW, NY 14043 888-684-0013

Calibration Data Card	Calibration Data Card
SHEAR ACCELEROMETER	SHEAR ACCELEROMETER
MCDEL #: 356B18	MODEL #: 356B18
SERIAL #:LW218951 (z axis)	SERIAL #:LW218951 (x axis)
SENSITIVITY: 1031 mV/g	SENSITIVITY: <u>1050</u> mV/g
(105.1 mV/m/s ²)	(107.1 mV/m/s ²)
BIAS LEVEL: <u>11.0</u> VDC Date: <u>2/15/17</u> By: <u>C. Kinyon</u> For further information, please refer to the accompanying calibration cortificate. C. Kinyon For further information, please refer to the accompanying calibration cortificate. BIAS LEVEL: <u>C. Kinyon</u> For further information, please refer to the accompanying calibration cortificate.	BIAS LEVEL: <u>11.0</u> VDC Date: <u>2/15/17</u> By: <u>C. Kinyon</u> For further information, please refer to the accompanying calibration certificate. CPCB PIEZOTRONICS 3425 WALDEN AVE · OEPEW, NY 14043 888-684-0013

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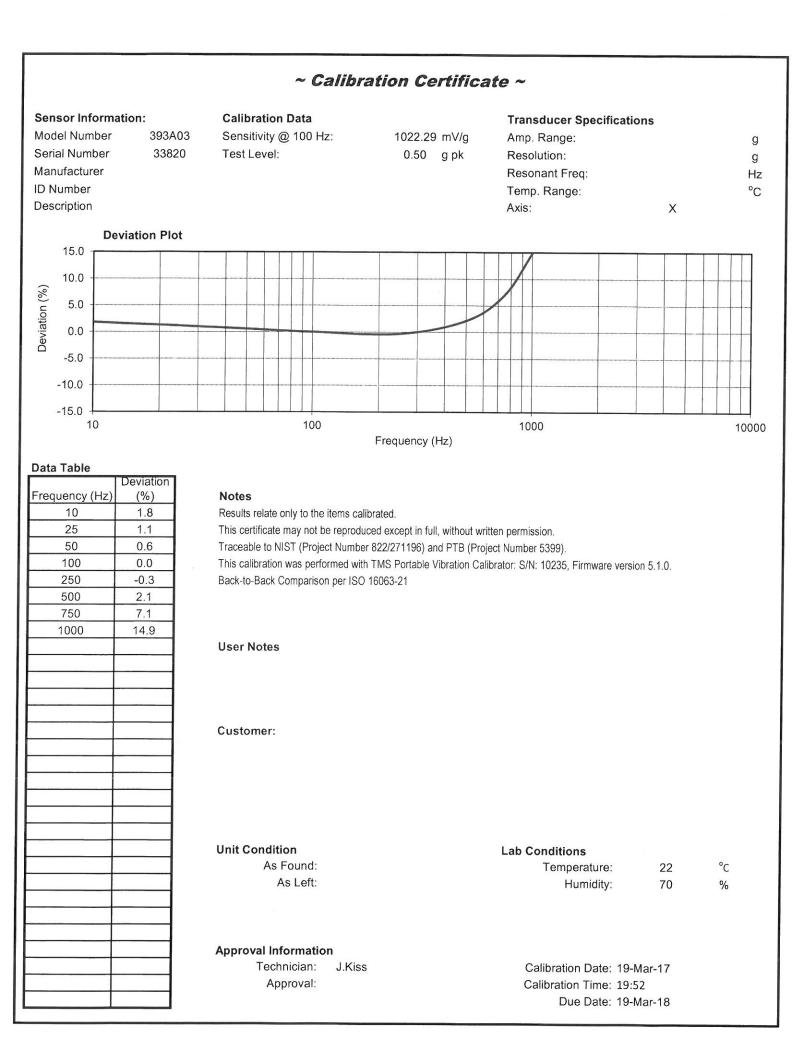
MODEL #: SERIAL #:		
SENSITIVITY		mV/g
	(104.0 r	nV/m/s²)
BIAS LEVEL:	11.0	VDC
D . 0/4 E /47 -	By: C.	Kinvon

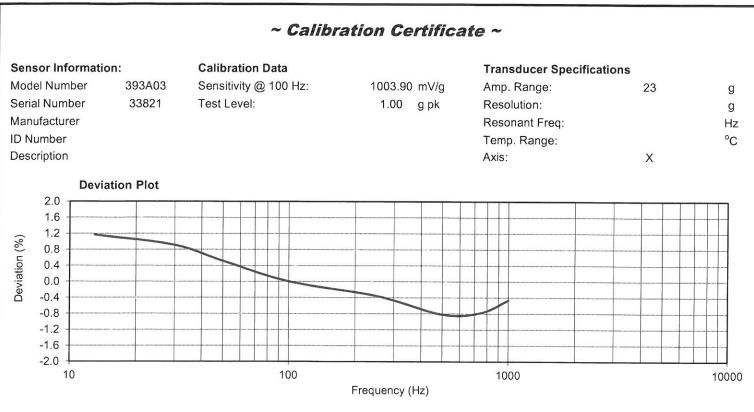
0

Calibration Data Card SHEAR ACCELEROMETER MCDEL #: 356B18 SERIAL #:LW219095 (z axis)	Calibration Data Card SHEAR ACCELEROMETER MCDEL,#: 356B18 SERIAL #:LW219095 (y axis)	MC
SENSITIVITY: 1065 mV/g (108.6 mV/m/s ²)	SENSITIVITY: 1013 mV/g (103.3 mV/m/s ²)	SE SE
BIAS LEVEL: <u>11.2</u> VDC Date: <u>2/15/17</u> By: <u>C. Kinyon</u> For further information, please refer to the accompanying calibration certificate. CPCB PIEZOTRONICS 3425 WALDEN AVE DEPEW, NY 14043 888-684-0013	BIAS LEVEL: <u>11.2</u> VDC Date: <u>2/15/17</u> By: <u>C. Kinyon</u> For further Information, please refer to the accompanying calibration certificate. CPCB PIEDORFONICS 3425 WALDEN AVE - DEFEW, NY 14043 888-684-0013	BIA Date

Calibration		
IODEL #:	356B	18
SERIAL #: LW2		x axis)
ENSITIVITY:		mV/g nV/m/s²)
AS LEVEL: _	11.2	VDC
ate: <u>2/15/17</u> By		Kinyon

For further information, please refer to the accompanying calibration certificate. 3425 WALDEN AVE · DEPEW, NY 14043 888-684-0013 *P





Data Table

	Deviation
Frequency (Hz)	(%)
13	1.2
30	0.9
50	0.5
100	0.0
250	-0.4
500	-0.8
750	-0.8
1000	-0.5

Notes

Results relate only to the items calibrated. This certificate may not be reproduced except in full, without written permission. Traceable to NIST (Project Number 822/271196) and PTB (Project Number 5399). This calibration was performed with TMS Portable Vibration Calibrator: S/N: 10235, Firmware version 5.1.0. Back-to-Back Comparison per ISO 16063-21

User Notes

Customer:

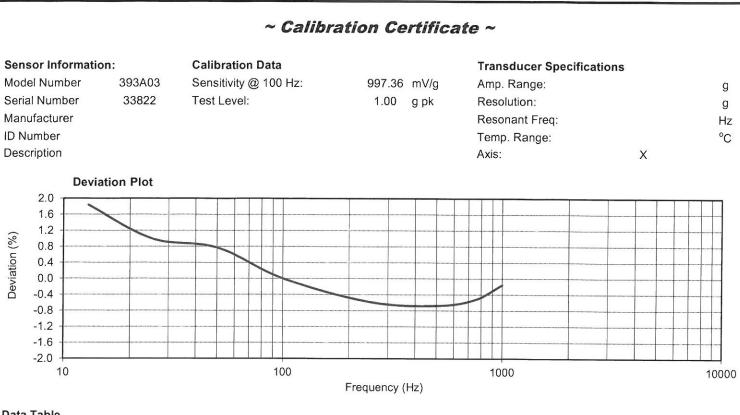
Unit Condition As Found: As Left:

Lab Conditions	
Temperature:	22
Humidity:	70

°C %

Approval Information

Technician: J.Kiss Approval: Calibration Date: 15-Mar-17 Calibration Time: 7:34 Due Date: 15-Mar-18



Data Table

Deviation

3	Deviation
Frequency (Hz)	(%)
13	1.8
25	1.0
50	0.8
100	0.0
250	-0.6
500	-0.7
750	-0.5
1000	-0.2

Notes

Results relate only to the items calibrated. This certificate may not be reproduced except in full, without written permission. Traceable to NIST (Project Number 822/271196) and PTB (Project Number 5399). This calibration was performed with TMS Portable Vibration Calibrator: S/N: 10235, Firmware version 5.1.0. Back-to-Back Comparison per ISO 16063-21

User Notes

Customer:

Unit Condition As Found: As Left:

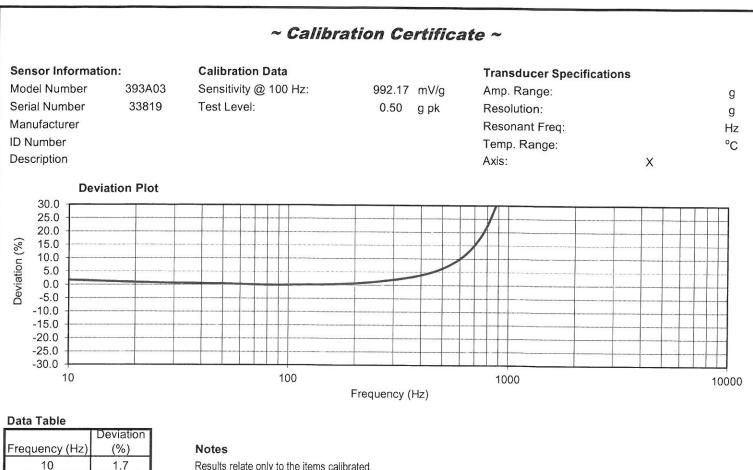
Lab	Conditions
	Temperature:

mperature:	20	°C
Humidity:	70	%

Approval Information

Technician: **J.Kiss** Approval:

Calibration Date: 15-Mar-17 Calibration Time: 7:40 Due Date: 15-Mar-18



Results relate only to the items calibrated. This certificate may not be reproduced except in full, without written permission. Traceable to NIST (Project Number 822/271196) and PTB (Project Number 5399). This calibration was performed with TMS Portable Vibration Calibrator: S/N: 10235, Firmware version 5.1.0. Back-to-Back Comparison per ISO 16063-21

User Notes

25

50

100

250

500

750

1000

0.7

0.4

0.0

1.3

6.3

18.4

42.4

Customer:

Unit Condition	Lab Conditions		
As Found:	Temperature:	22	°C
As Left:	Humidity:	70	%

Approval Information

Technician: J.Kiss Approval:

Calibration Date: 19-Mar-17 Calibration Time: 19:47 Due Date: 17-Mar-18

Calibration Data

Low Frequency Accelerometer

Model 731A

Serial Number 10460

Sensitivity 10.5 V/g

Bias Voltage 9.7 Vdc

Resonance 850 Hz

Maximum Amplitude Range 0.5 g peak

Fr	equency R	espon	se	
±1dB	0.10 Hz	to	330 Hz	
±3dB	0.05 Hz	to	490 Hz	

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W.

Calibrated by: S.HONGMANIVAN Date: 03/08/2017

This calibration is traceable to the National Institute of Standards and Technology, Gaithersburg, MD 20899. Frequency Response is traceable 5 Hz to 10 kHz. Low end frequency response and amplitude range are nominal values.

Sensitivity measured at 10 Hz, 0.5g, 25°C.

Meggitt (Maryland), Inc. is an ISO 9001 Registered Company.

Calibration Data

Low Frequency Accelerometer

Model 731A

Serial Number 10461

Sensitivity 10.4 V/g

Bias Voltage 9.5 Vdc

Resonance 850 Hz

Maximum Amplitude Range 0.5 g peak

Fre	equency R	espon	se
±1dB	0.10 Hz	to	330 Hz
±3dB	0.05 Hz	to	490 Hz

Calibrated by: S.HONGMANIVAN Date: 03/08/2017

This calibration is traceable to the National Institute of Standards and Technology, Gaithersburg, MD 20899. Frequency Response is traceable 5 Hz to 10 kHz. Low end frequency response and amplitude range are nominal values. Sensitivity measured at 10 Hz, 0.5g, 25°C.

Meggitt (Maryland), Inc. is an ISO 9001 Registered Company.

Meggitt Sensing Systems 20511 Seneca Meadows Parkway, Germantown MD 20876, USA

Calibration Data

Low Frequency Accelerometer

Model 731A

Serial Number 10463

Sensitivity 10.6 V/g

Bias Voltage 9.3 Vdc

Resonance 850 Hz

Maximum Amplitude Range 0.5 g peak

Fre	equency R	espon	se
±1dB	0.10 Hz	to	330 Hz
±3dB	0.05 Hz	to	500 Hz

Calibrated by: S.HONGMANIVAN Date: 03/08/2017

This calibration is traceable to the National Institute of Standards and Technology, Gaithersburg, MD 20899. Frequency Response is traceable 5 Hz to 10 kHz. Low end frequency response and amplitude range are nominal values. Sensitivity measured at 10 Hz, 0.5g, 25°C.

Meggitt (Maryland), Inc. is an ISO 9001 Registered Company.

MEGGíTT

Calibration Data

Low Frequency Accelerometer

Model 731A

Serial Number 10464

Sensitivity 10.5 V/g

Bias Voltage 9.4 Vdc

Resonance 810 Hz

Maximum Amplitude Range 0.5 g peak

Fre	equency R	espon	se
±1dB	0.10 Hz	to	330 Hz
±3dB	0.05 Hz	to	490 Hz

Calibrated by: S.HONGMANIVAN Date: 02/08/2017

This calibration is traceable to the National Institute of Standards and Technology, Gaithersburg, MD 20899. Frequency Response is traceable 5 Hz to 10 kHz. Low end frequency response and amplitude range are nominal values. Sensitivity measured at 10 Hz, 0.5g, 25°C.

Meggitt (Maryland), Inc. is an ISO 9001 Registered Company.

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Calibration Data

Low Frequency Accelerometer

Model 731A

Serial Number 10465

Sensitivity 10.3 V/g

Bias Voltage 9.6 Vdc

Resonance 850 Hz

Maximum Amplitude Range 0.5 g peak

Fr	equency R	espon	se	
±1dB	0.10 Hz	to	330 Hz	
±3dB	0.05 Hz	to	490 Hz	

Calibrated by: S.HONGMANIVAN Date: 03/08/2017

This calibration is traceable to the National Institute of Standards and Technology, Gaithersburg, MD 20899. Frequency Response is traceable 5 Hz to 10 kHz. Low end frequency response and amplitude range are nominal values.

Sensitivity measured at 10 Hz, 0.5g, 25°C.

Meggitt (Maryland), Inc. is an ISO 9001 Registered Company.

Meggitt Sensing Systems 20511 Seneca Meadows Parkway, Germantown MD 20876, USA

Tel: +1 (301) 330 8811 Tel: 1 800 WILCOXON Fax: +1 (301) 330 8873 www.meggittsensingsystems.com www.meggitt.com

Meggitt (Maryland), Inc d/b/a Meggitt Sensing Systems

Calibration Certificate

Part Number: 714A9801 Description: LINEAR MICROPHONE 2-250HZ Serial Number: BH9341 Calibration Date: February 22, 2017 Calibration Equipment: 714J7401

Instantel certifies that the above product was calibrated in accordance with the applicable Instantel procedures. These procedures are part of a quality system that is designed to assure that the product listed above meets or exceeds Instantel specifications

Instantel further certifies that the measurement instruments used during the calibration of this product are traceable to the National Institute of Standards and Technology; or National Research Council of Canada. Evidence of traceability is on file at Instantel and is available upon request.

The environment in which this product was calibrated is maintained within the operating specifications of the instrument.

Please note that the sensor check function is intended to check that the sensors are connected to the unit, installed in the proper orientation and sufficiently level to operate properly This function should not be confused with a formal calibration, which requires the sensors be checked against a reference that is traceable to a known standard Instantel recommends that products be returned to Instantel or an authorized service and calibration facility for annual calibration.

Calibrated By: __________Li Pan

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Calibration Certificate

Part Number: 714A9701 Description: TRIAXIAL GEOPHONE (ISEE) Serial Number: BG17714 Calibration Date: February 22, 2017 Calibration Equipment: 714J7401

Instantel certifies that the above product was calibrated in accordance with the applicable Instantel procedures. These procedures are part of a quality system that is designed to assure that the product listed above meets or exceeds Instantel specifications

Instantel further certifies that the measurement instruments used during the calibration of this product are traceable to the National Institute of Standards and Technology; or National Research Council of Canada. Evidence of traceability is on file at Instantel and is available upon request.

The environment in which this product was calibrated is maintained within the operating specifications of the instrument

Please note that the sensor check function is intended to check that the sensors are connected to the unit, installed in the proper orientation and sufficiently level to operate properly. This function should not be confused with a formal calibration, which requires the sensors be checked against a reference that is traceable to a known standard Instantel recommends that products be returned to Instantel or an authorized service and calibration facility for annual calibration.

Calibrated By: ____

Andrew Stockwell

instantel

Part Number: 716A0403 Description: MINIMATE PLUS W/EXT. GEO Serial Number: BE18695 Calibration Date: February 22, 2017 Calibration Equipment: 718A1501

Instantel certifies that the above product was calibrated in accordance with the applicable Instantel procedures. These procedures are part of a quality system that is designed to assure that the product listed above meets or exceeds Instantel specifications

Instantel further certifies that the measurement instruments used during the calibration of this product are traceable to the National Institute of Standards and Technology; or National Research Council of Canada. Evidence of traceability is on file at Instantel and is available upon request.

The environment in which this product was calibrated is maintained within the operating specifications of the instrument

Please note that the sensor check function is intended to check that the sensors are connected to the unit, installed in the proper orientation and sufficiently level to operate properly. This function should not be confused with a formal calibration, which requires the sensors be checked against a reference that is traceable to a known standard instantel recommends that products be returned to Instantel or an authorized service and calibration facility for annual calibration.

Calibrated By:

Andrew Stockwell



Part Number: 714A9801 Description: LINEAR MICROPHONE 2-250HZ Serial Number: BH11358 Calibration Date: February 22, 2017 Calibration Equipment: 714J7401

Instantel certifies that the above product was calibrated in accordance with the applicable Instantel procedures. These procedures are part of a quality system that is designed to assure that the product listed above meets or exceeds Instantel specifications

Instantel further certifies that the measurement instruments used during the calibration of this product are traceable to the National Institute of Standards and Technology; or National Research Council of Canada. Evidence of traceability is on file at Instantel and is available upon request.

The environment in which this product was calibrated is maintained within the operating specifications of the instrument.

Please note that the sensor check function is intended to check that the sensors are connected to the unit, installed in the proper orientation and sufficiently level to operate properly. This function should not be confused with a formal calibration, which requires the sensors be checked against a reference that is traceable to a known standard. Instantel recommends that products be returned to Instantel or an authorized service and calibration facility for annual calibration.

Calibrated By:

Andrew Stockwell

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Part Number: 716A0403 Description: MINIMATE PLUS W/EXT. GEO Serial Number: BE15696 Calibration Date: February 22, 2017 Calibration Equipment: 718A1501

Instantel certifies that the above product was calibrated in accordance with the applicable Instantel procedures. These procedures are part of a quality system that is designed to assure that the product listed above meets or exceeds Instantel specifications

Instantel further certifies that the measurement instruments used during the calibration of this product are traceable to the National Institute of Standards and Technology; or National Research Council of Canada. Evidence of traceability is on file at Instantel and is available upon request.

The environment in which this product was calibrated is maintained within the operating specifications of the instrument.

Please note that the sensor check function is intended to check that the sensors are connected to the unit, installed in the proper orientation and sufficiently level to operate properly. This function should not be confused with a formal calibration, which requires the sensors be checked against a reference that is traceable to a known standard Instantel recommends that products be returned to Instantel or an authorized service and calibration facility for annual calibration.

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Part Number: 714A9701 Description: TRIAXIAL GEOPHONE (ISEE) Serial Number: BG9326 Calibration Date: February 22, 2017 Calibration Equipment: 714J7401

Instantel certifies that the above product was calibrated in accordance with the applicable Instantel procedures. These procedures are part of a quality system that is designed to assure that the product listed above meets or exceeds Instantel specifications

Instantel further certifies that the measurement instruments used during the calibration of this product are traceable to the National Institute of Standards and Technology; or National Research Council of Canada. Evidence of traceability is on file at Instantel and is available upon request.

The environment in which this product was calibrated is maintained within the operating specifications of the instrument

Please note that the sensor check function is intended to check that the sensors are connected to the unit, installed in the proper orientation and sufficiently level to operate properly This function should not be confused with a formal calibration, which requires the sensors be checked against a reference that is traceable to a known standard Instantel recommends that products be returned to Instantel or an authorized service and culibration facility for annual calibration.

Calibrated By: _

and Instante

Li Pan

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Part Number: 716A0403 Description: MINIMATE PLUS W/EXT. GEO Serial Number: BE8719 Calibration Date: March 13, 2017 Calibration Equipment: 718A1501

Instantel certifies that the above product was calibrated in accordance with the applicable Instantel procedures. These procedures are part of a quality system that is designed to assure that the product listed above meets or exceeds Instantel specifications

Instantel further certifies that the measurement instruments used during the calibration of this product are traceable to the National Institute of Standards and Technology; or National Research Council of Canada. Evidence of traceability is on file at Instantel and is available upon request.

The environment in which this product was calibrated is maintained within the operating specifications of the instrument.

Please note that the sensor check function is intended to check that the sensors are connected to the unit, installed in the proper orientation and sufficiently level to operate properly. This function should not be confused with a formal calibration, which requires the sensors be checked against a reference that is traceable to a known standard. Instantel recommends that products be returned to Instantel or an authorized service and calibration facility for annual calibration.

Calibrated By:

and Instante

Ctor.

Tuyen Bui

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Part Number: 714A9701 Description: TRIAXIAL GEOPHONE (ISEE) Serial Number: BG16603 Calibration Date: March 13, 2017 Calibration Equipment: 714J7401

Instantel certifies that the above product was calibrated in accordance with the applicable Instantel procedures. These procedures are part of a quality system that is designed to assure that the product listed above meets or exceeds Instantel specifications

Instantel further certifies that the measurement instruments used during the calibration of this product are traceable to the National Institute of Standards and Technology; or National Research Council of Canada. Evidence of traceability is on file at Instantel and is available upon request.

The environment in which this product was calibrated is maintained within the operating specifications of the instrument.

Please note that the sensor check function is intended to check that the sensors are connected to the unit, installed in the proper orientation and sufficiently level to operate properly. This function should not be confused with a formal calibration, which requires the sensors be checked against a reference that is traceable to a known standard. Instantel recommends that products be returned to Instantel or an authorized service and calibration facility for annual calibration.

Calibrated By:

Tuyen Bui



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Part Number: 716A0403 Description: MINIMATE PLUS W/EXT. GEO Serial Number: BE8720 Calibration Date: March 13, 2017 Calibration Equipment: 718A1501

Instantel certifies that the above product was calibrated in accordance with the applicable Instantel procedures. These procedures are part of a quality system that is designed to assure that the product listed above meets or exceeds Instantel specifications

Instantel further certifies that the measurement instruments used during the calibration of this product are traceable to the National Institute of Standards and Technology; or National Research Council of Canada. Evidence of traceability is on file at Instantel and is available upon request.

The environment in which this product was calibrated is maintained within the operating specifications of the instrument.

Please note that the sensor check function is intended to check that the sensors are connected to the unit, installed in the proper orientation and sufficiently level to operate properly. This function should not be confused with a formal calibration, which requires the sensors be checked against a reference that is traceable to a known standard. Instantel recommends that products be returned to Instantel or an authorized service and calibration facility for annual calibration.

Calibrated By:

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Tuyen Bui

Instantel

ney Works or

Part Number: 714A9701 Description: TRIAXIAL GEOPHONE (ISEE) Serial Number: BG18657 Calibration Date: March 13, 2017 Calibration Equipment: 714J7401

Instantel certifies that the above product was calibrated in accordance with the applicable Instantel procedures. These procedures are part of a quality system that is designed to assure that the product listed above meets or exceeds Instantel specifications

Instantel further certifies that the measurement instruments used during the calibration of this product are traceable to the National Institute of Standards and Technology; or National Research Council of Canada. Evidence of traceability is on file at Instantel and is available upon request.

The environment in which this product was calibrated is maintained within the operating specifications of the instrument.

Please note that the sensor check function is intended to check that the sensors are connected to the unit, installed in the proper orientation and sufficiently level to operate properly. This function should not be confused with a formal calibration, which requires the sensors be checked against a reference that is traceable to a known standard. Instantel recommends that products be returned to Instantel or an authorized service and calibration facility for annual calibration.

Calibrated By:

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~Certificate of Calibration~

The Modal Shop		
9100D		
737		
Portable Vibration Calibrator		
PRD-P278		
BTH Barris		
TMS Rental		

12-INO	/-16
9.9 °F	1
	15-Nov 9.9_°F

Humidity: 21.1 °C 29.7 %

As found: In Tolerance As left: In Tolerance Internal Reference 10.18 mV/g Sensitivity @ 100 Hz: 1.04 mV/m/s² (Measured at Monitor Reference Out BNC)

Reference Equipment:

Manufacturer	Description	Model Number	Serial Number	Due Date
PCB	Standard Sensor	353B02	126967	8/24/2017
PCB	Signal Cond.	442A102	299	8/23/2017
HP	DMM	34401A	US36061937	3/1/2017

	Standar	d Sensor	Unit Ur	nder Test	% difference
Frequency Hz	Measured Acc g pk	eleration Level m/s ²	Displayed Acc g pk	celeration Level m/s ²	Displayed / Measured
7	0.40	3.92	0.40	3.92	-0.01%
10	0.81	7.97	0.81	7.94	-0.36%
30	1.00	9.82	1.00	9.81	-0.18%
50	1.00	9.83	1.00	9.81	-0.21%
80	1.00	9.81	1.00	9.81	-0.08%
100	1.00	9.81	1.00	9.81	-0.04%
160	1.00	9.81	1.00	9.81	0.01%
300	1.00	9.78	1.00	9.81	0.23%
500	1.00	9.80	1.00	9.81	0.07%
1000	1.00	9.81	1.00	9.81	-0.05%
2000	1.00	9.80	1.00	9.81	0.11%
3000	1.00	9.82	1.00	9.81	-0.14%
4000	1.00	9.83	1.00	9.81	-0.26%
5000	1.00	9.83	1.00	9.81	-0.20%
6000	1.00	9.83	1.00	9.81	-0.26%
8000	1.00	9.79	1.00	9.81	0.19%
9000	1.01	9.86	1.00	9.81	-0.54%
10000	1.00	9.86	1.00	9.81	-0.49%

Notes:

1. This document certifies that the above meets published specifications.

2. The equipment referenced above has been calibrated using standards traceable to NIST (Project Number 822/271196) and PTB (Project Number 5399). Evidence of traceability is on file at The Modal Shop.

3. The results documented in this certificate relate only to the items tested or calibrated.

4. This certificate may not be reproduced, except in full, without the written consent of The Modal Shop, Inc.

 Measurement uncertainty (95% confidence level with coverage factor 2) for frequency ranges tested during calibration are as follows: 7<10 Hz; ±4.0%, 10<30 Hz; ± 3.0%, 30<100 Hz; ± 1.5%, 100 Hz; ± 1.5%, 100<2000 Hz; ± 1.5%, 2000-10,000 Hz; ± 4.0%.



Calibration Lab Certificate Number 2649-01 PRD-F256 revH 7/29/2016



The Modal Shop Inc. 3149 East Kemper Road Cincinnati, Ohio 45241 +1-513-351-9919 www.modalshop.com

Calibration ID: 11/15/16 13:55

page 1 of 1



57377

ICP® Accelerometer

Sensor Information

Model Number: 393A03

Serial Number: 45222

Manufacturer: PCB

~Calibration Certificate~

Calibration Data

Sensitivity @ 10 Hz:	1,008	mV/g
Phase @ 10 Hz:	0.03	deg.
Test Level:	1.00	g
Output Bias Level:	11.1	VDC

3149 East Kemper Rd. Cincinnati, OH 45241 Ph : 513-351-9919 Fax: 513-458-2172 www.modalshop.com

Transducer Specifications

Amp. Range:	± 5	g
Resolution:	0.00001	g
Resonant Freq:	≥ 10000	Hz
Temp. Range:	-54 to 121	°C
	-65 to 250	۴
Axis:	Uni-Axial	

Data Table

ID Number:

Description:

Freq. (Hz)	Deviation (%)	Phase (deg)
0.5	-1.1135	12.8169
1	0.6316	6.2437
2	0.7043	2.9073
3	0.5200	1.6752
4	0.4246	1.0607
5	0.2988	0.7223
6.3	0.2181	0.4402
7	0.1874	0.3299
8	0.1677	0.1897
10	0.0000	0.0290
30	-0.8181	-0.6411
50	-1.2135	-0.6711
100	-1.7880	-0.7841
300	-2.7910	-0.7894
500	-3.0108	-0.6941
1000	-2.8024	-0.5980
2000	-0.5789	-0.3170

Customer

TMS Rental 3149 E. Kemper Rd Cincinnati, OH 45241 User Notes

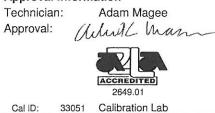
Lab Conditions

Temperature:	74 (23)	⁰F (℃)
Humidity:	44	%

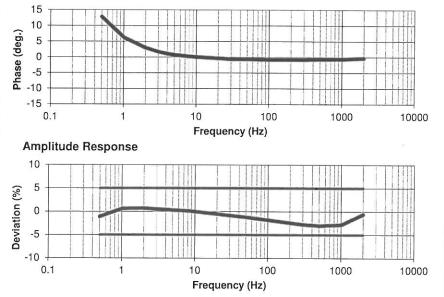
44 %

Cal Date: 1-May-17 Due Date:

Approval Information



Phase Response



Notes

Results relate only to the items calibrated.

This certificate may not be reproduced except in full, without written permision.

Method: Back-to-Back Comparison Calibration per ISO 16063 Part 21

This calibration was performed with TMS 9155 Calibration Workstation 1 version 6.0.0 Calibration traceable to NIST (project number 822/271196).

Back-to-Back Comparison Calibration per ISO 16063-21

Procedures Used: PRD-P220, PRD-P214

Measurement uncertainty (95% confidence level with coverage factor 2) for frequency ranges tested during calibration are as follows: 0.5-1 Hz; 1.10%; >1-10 Hz; \pm 0.80%, 11-99 Hz; \pm 1.20%, 100 Hz; \pm 0.75%, 101-920 Hz; \pm 1.00%, 921-5000 Hz; \pm 1.40%, 5001-10,000 Hz; \pm 1.90%, 10,001-15,000 Hz; \pm 2.20%, 15,001-20,000 Hz; \pm 2.8%.

Unit Condition

As Found: In Tolerance

As Left: In Tolerance

Description	Manufacturer	Model	Serial	Due Date
Data Aquisition Card	NI	PCI-4461	19A1EE8	8/18/2017
Ref Std Conditioner	NI	PCI-6251	136F2A3	10/25/2017
Reference Std	PCB	080A200	110553	2/13/2018
Air Bearing Shaker	PCB	396C11	603	n/a
Ref Std Conditioner	PCB	442A102	305	2/13/2018
SUT Signal Conditioner	PCB	443B101	373	11/7/2017
Power Amplifier	TMS	2100E21-C	50002	n/a
Reference Std	TMS	2129E025	111	10/25/2017
Long Stroke Shaker	TMS	2129E025-779	111	n/a
				Pac



57379

ICP® Accelerometer

Sensor Information

Model Number: 393A03

Serial Number: 45215

Manufacturer: PCB

~Calibration Certificate~

Calibration Data

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С

3149 East Kemper Rd. Cincinnati, OH 45241 Ph: 513-351-9919 Fax: 513-458-2172 www.modalshop.com

Transducer Specifications

Amp. Range:	± 5	g
Resolution:	0.00001	g
Resonant Freq:	≥ 10000	Hz
Temp. Range:	-54 to 121	°C
	-65 to 250	۴
Axis:	Uni-Axial	

Data Table

ID Number:

Description:

Freq. (Hz)	Deviation (%)	Phase (deg)
0.5	-0.6098	11.4590
1	0.6453	5.4998
2	0.5800	2.5505
3	0.3932	1.4523
4	0.3126	0.9052
5	0.1979	0.5990
6.3	0.1280	0.3524
7	0.1046	0.2527
8	0.0931	0.1335
10	0.0000	-0.0883
30	-0.8297	-0.6485
50	-1.1549	-0.7093
100	-1.7910	-0.7903
300	-2.7340	-0.8262
500	-3.0301	-0.7639
1000	-3.1325	-0.7363
2000	-1.8563	-0.5469



TMS Rental 3149 E. Kemper Rd Cincinnati, OH 45241 **User Notes**

Lab Conditions

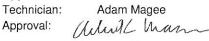
Temperature:	73 (23)	°F (℃)
Humidity:	45	%

%

1-May-17

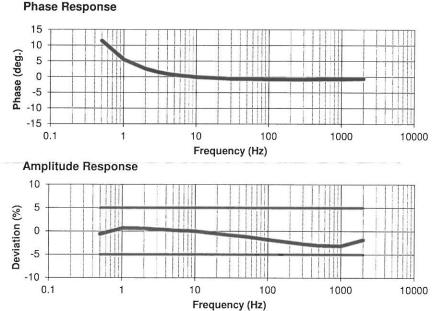
Cal Date: Due Date:

Approval Information





Cal ID: 33048 Calibration Lab



Notes

Results relate only to the items calibrated.

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Method: Back-to-Back Comparison Calibration per ISO 16063 Part 21

This calibration was performed with TMS 9155 Calibration Workstation 1 version 6.0.0 Calibration traceable to NIST (project number 822/271196).

Back-to-Back Comparison Calibration per ISO 16063-21

Procedures Used: PRD-P220, PRD-P214

Measurement uncertainty (95% confidence level with coverage factor 2) for frequency ranges tested during calibration are as follows: 0.5-1 Hz; 1.10%; >1-10 Hz; ± 0.80%, 11-99 Hz; ±1.20%, 100 Hz; ± 0.75%, 101-920 Hz; ± 1.00%, 921-5000 Hz; ± 1.40%, 5001-10,000 Hz; ± 1.90%, 10,001-15,000 Hz; ± 2.20%, 15,001-20,000 Hz; ± 2.8%.

Unit Condition

As Found: In Tolerance

As Left: In Tolerance

Description	Manufacturer	Model	Serial	Due Date
Data Aquisition Card	NI	PCI-4461	19A1EE8	8/18/2017
Ref Std Conditioner	NI	PCI-6251	136F2A3	10/25/2017
Reference Std	PCB	080A200	110553	2/13/2018
Air Bearing Shaker	PCB	396C11	603	n/a
Ref Std Conditioner	PCB	442A102	305	2/13/2018
SUT Signal Conditioner	PCB	443B101	373	11/7/2017
Power Amplifier	TMS	2100E21-C	50002	n/a
Reference Std	TMS	2129E025	111	10/25/2017
Long Stroke Shaker	TMS	2129E025-779	111	n/a
				Page 1 of



Sensor Information

Model Number: 393A03

Serial Number: 44845

Manufacturer: PCB

~Calibration Certificate~

Calibration Data 4.4. 0 1011 Se

Sensitivity @ 10 Hz:	1,020	mv/g
Phase @ 10 Hz:	0.10	deg.
Test Level:	1.00	g
Output Bias Level:	10.9	VDC

3149 East Kemper Rd. Cincinnati, OH 45241 Ph: 513-351-9919 Fax: 513-458-2172 www.modalshop.com

Transducer Specifications

Amp. Range:	± 5	g
Resolution:	0.00001	g
Resonant Freq:	≥ 10000	Hz
Temp. Range:	-54 to 121	°C
	-65 to 250	۴
Axis:	Uni-Axial	

Data Table

ID Number:

Description:

Freq. (Hz)	Deviation (%)	Phase (deg)
0.5	-1.6633	14.0828
1	0.3320	6.9124
2	0.5019	3.2741
3	0.3457	1.9269
4	0.2598	1.2567
5	0.1370	0.8809
6.3	0.0631	0.5768
7	0.0373	0.4531
8	0.0206	0.2999
10	0.0000	0.0988
30	-0.8043	-0.5776
50	-1.1777	-0.6396
100	-1.7727	-0.7828
300	-2.7257	-0.8487
500	-3.1080	-0.7200
1000	-3.1394	-0.6401
2000	-1.8636	-0.4417

ICP® Accelerometer

Customer

TMS Rental 3149 E. Kemper Rd Cincinnati, OH 45241 **User Notes**

Lab Conditions

Temperature:	73 (23)	⁰F (°C)
Humidity:	45	%

5 %

Cal Date: Due Date:

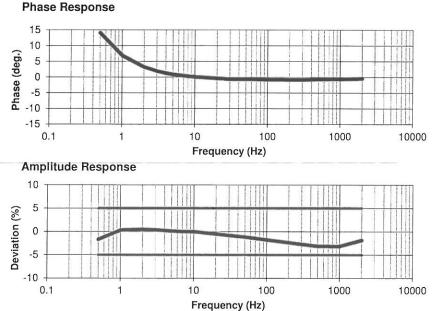
Approval Information

Technician: Adam Magee Approval: man

1-May-17



Calibration Lab Cal ID: 33049



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Notes

Results relate only to the items calibrated.

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Method: Back-to-Back Comparison Calibration per ISO 16063 Part 21

This calibration was performed with TMS 9155 Calibration Workstation 1 version 6.0.0 Calibration traceable to NIST (project number 822/271196).

Back-to-Back Comparison Calibration per ISO 16063-21

Procedures Used: PRD-P220, PRD-P214

Measurement uncertainty (95% confidence level with coverage factor 2) for frequency ranges tested during calibration are as follows: 0.5-1 Hz; 1.10%; >1-10 Hz; ± 0.80%, 11-99 Hz; ±1.20%, 100 Hz; ± 0.75%, 101-920 Hz; ± 1.00%, 921-5000 Hz; ± 1.40%, 5001-10,000 Hz; ± 1.90%, 10,001-15,000 Hz; ± 2.20%, 15,001-20,000 Hz; ± 2.8%.

Unit Condition

As Found: In Tolerance

As Left: In Tolerance

Description	Manufacturer	Model	Serial	Due Date
Data Aquisition Card	NI	PCI-4461	19A1EE8	8/18/2017
Ref Std Conditioner	NI	PCI-6251	136F2A3	10/25/2017
Reference Std	PCB	080A200	110553	2/13/2018
Air Bearing Shaker	PCB	396C11	603	n/a
Ref Std Conditioner	PCB	442A102	305	2/13/2018
SUT Signal Conditioner	PCB	443B101	373	11/7/2017
Power Amplifier	TMS	2100E21-C	50002	n/a
Reference Std	TMS	2129E025	111	10/25/2017
Long Stroke Shaker	TMS	2129E025-779	111	n/a
				Page 1 of



Sensor Information

Model Number: 393A03

Serial Number: 44846

Manufacturer: PCB

~Calibration Certificate~

Calibration Data

Sensitivity @ 10 Hz:	1,017	mV/g
Phase @ 10 Hz:	-0.10	deg.
Test Level:	1.00	g

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Transducer Specifications

Amp. Range:	± 5	g
Resolution:	0.00001	g
Resonant Freq:	≥ 10000	Hz
Temp. Range:	-54 to 121	°C
	-65 to 250	۴
Axis:	Uni-Axial	

Data Table

ID Number: Description:

Freq. (Hz)	Deviation (%)	Phase (deg)
0.5	-0.5587	12.0484
1	0.8912	5.7700
2	0.8196	2.6535
3	0.6069	1.4987
4	0.5040	0.9244
5	0.3717	0.6057
6.3	0.2843	0.3512
7	0.2564	0.2460
8	0.2369	0.1216
10	0.0000	-0.1049
30	-0.8271	-0.6569
50	-1.2656	-0.6832
100	-1.8599	-0.8089
300	-2.8676	-0.8467
500	-3.2330	-0.7538
1000	-3.2950	-0.6500
2000	-2.0466	-0.3827

ICP® Accelerometer

Customer

TMS Rental 3149 E. Kemper Rd Cincinnati, OH 45241 **User Notes**

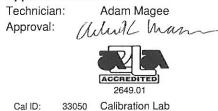
Lab Conditions

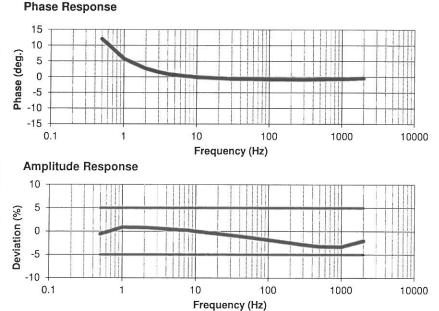
Temperature:	74 (23)	°F (℃)
Humidity:	23	%

3 %

Cal Date: 1-May-17 Due Date:

Approval Information





Notes

Results relate only to the items calibrated.

This certificate may not be reproduced except in full, without written permision.

Method: Back-to-Back Comparison Calibration per ISO 16063 Part 21

This calibration was performed with TMS 9155 Calibration Workstation 1 version 6.0.0 Calibration traceable to NIST (project number 822/271196).

Back-to-Back Comparison Calibration per ISO 16063-21

Procedures Used: PRD-P220, PRD-P214

Measurement uncertainty (95% confidence level with coverage factor 2) for frequency ranges tested during calibration are as follows: 0.5-1 Hz; 1.10%; >1-10 Hz; ± 0.80%, 11-99 Hz; ±1.20%, 100 Hz; ± 0.75%, 101-920 Hz; ± 1.00%, 921-5000 Hz; ± 1.40%, 5001-10,000 Hz; ± 1.90%, 10,001-15,000 Hz; ± 2.20%, 15,001-20,000 Hz; ± 2.8%.

Unit Condition

As Found: In Tolerance

As Left: In Tolerance

Description	Manufacturer	Model	Serial	Due Date
Data Aquisition Card	NI	PCI-4461	19A1EE8	8/18/2017
Ref Std Conditioner	NI	PCI-6251	136F2A3	10/25/2017
Reference Std	PCB	080A200	110553	2/13/2018
Air Bearing Shaker	PCB	396C11	603	n/a
Ref Std Conditioner	PCB	442A102	305	2/13/2018
SUT Signal Conditioner	PCB	443B101	373	11/7/2017
Power Amplifier	TMS	2100E21-C	50002	n/a
Reference Std	TMS	2129E025	111	10/25/2017
Long Stroke Shaker	TMS	2129E025-779	111	n/a
			1994 T	Page 1 c

The Modal Shop, Inc. (513) 351-9919

Manufacturer: PCB Model: 393A03 Serial: 44846 ID:

Sens: 1016.9601 mV/g @ 10Hz Cal Date: 5/1/2017 Due Date: The Modal Shop, Inc. (513) 351-9919

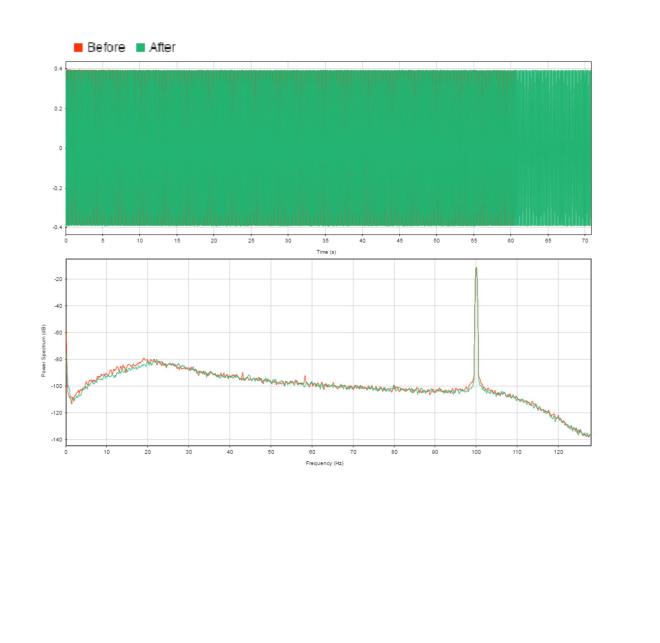
Manufacturer: PCB Model: 393A03 Serial: 45222 ID: 57377 Sens: 1008.3763 mV/g @ 10Hz Cal Date: 5/1/2017 Due Date:

The Modal Shop, Inc. (513) 351-9919 Manufacturer: PCB Model: 393A03 Serial: 44845 ID:

Sens: 1020.3987 mV/g @ 10Hz Cal Date: 5/1/2017 Due Date: The Modal Shop, Inc. (513) 351-9919

Manufacturer: PCB Model: 333A03 Serial: 45215 ID: 57375

Sens: 998.4807 mV/g 0 10Hz Cal Date: 5/1/2017 Due Date:

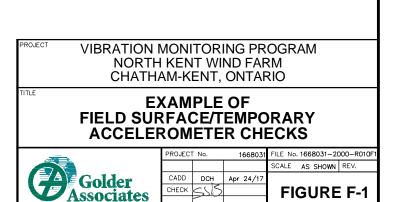


RED - BEFORE MONITORING

GREEN - AFTER MONITORING

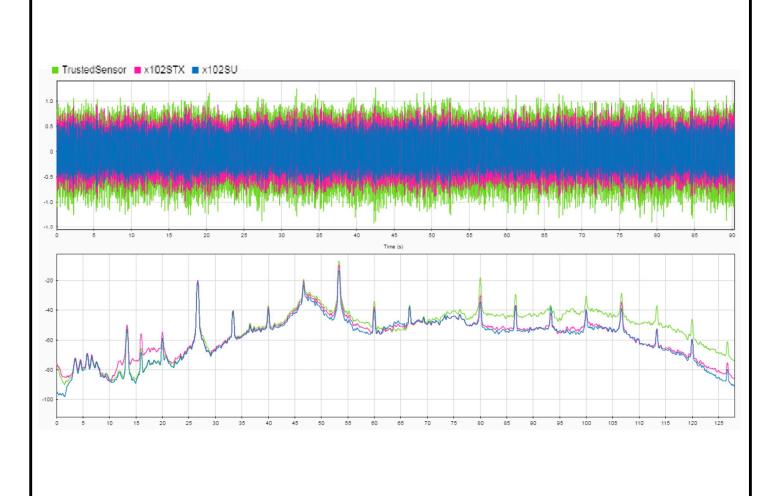
NOTES

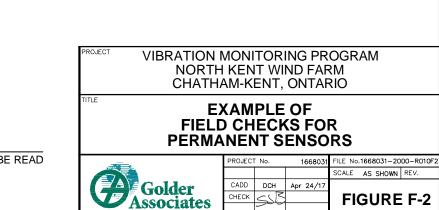
THIS DRAWING IS SCHEMATIC ONLY AND IS TO BE READ IN CONJUNCTION WITH ACCOMPANYING TEXT.



Drawing file: 1668031-2000-R010F1.dwg Apr 24,

2017 - 4: 32pm





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FIGURE F-2

NOTES

THIS DRAWING IS SCHEMATIC ONLY AND IS TO BE READ IN CONJUNCTION WITH ACCOMPANYING TEXT.



APPENDIX G

Temporary Well Monitoring System Evaluation





TEMPORARY MOUNTING OF ACCELEROMETERS

Typically, the type of accelerometers used for this project are mounted to fixed structures or machinery using one of three general methods:

- 1) Threaded Stud: Usually, for monitoring structures with a flat, smooth surface within which a threaded hole can be made, mounting accelerometers using a threaded stud provides the most secure sensor coupling. For this project, a flat and smooth surface was not available for the domestic well casings, other than the lid, and drilling and tapping a threaded hole in the casing was not acceptable since this would permanently damage the well casing. The accelerometers were not mounted to the lids since the well lid to well casing connection character was not known. Mounting stud orientation would require the accelerometers in this case to be oriented horizontally unless a right-angle bracket was utilized with a secondary mounting stud and two studs were used to secure the bracket to the well casing.
- 2) Adhesive: When drilling and tapping a threaded hole in the structure is not possible, yet a flat, clean and smooth surface is available, accelerometers can be mounted with the application of a thin layer of hot glue or wax for temporary mounting or epoxy can be used for permanent monitoring. A flat, clean and smooth surface was not available for the domestic well casings and the casings had been exposed to weathering that could readily influence the bonding of such adhesives. Further, adhering the sensor directly to the casing would have required mating the flat bottom of the accelerometer to the rounded side of the casing, positioning the accelerometer in the horizontal direction, or use of a right-angle bracket and additional mounting hardware (with a stud on the bracket for the accelerometer base). Accelerometers mounted with adhesives are known to suffer some reduction in high-frequency response. Use of hot glue to adhere the accelerometer to the aluminum well cover was not selected because the details regarding tightness of fit between the cover and steel were not known at the time and introduced another uncertainty. Wax was not considered suitable because of the temperature and weather conditions at the time.
- 3) Magnetic Mounting: Rare earth or ceramic magnets that are flat or dual rail magnets can sometimes be used to couple accelerometers to ferric structures or machinery. When magnetic mounts are used, the surfaces to which the magnets will contact often have to be cleaned and properly prepared since poor installations can result in a significant drop in accelerometer sensitivity at high frequencies. Dual rail magnet mounts also must be selected so that the rail sizes are compatible with the radius of curvature of any curved mounting surfaces. Prefabricated magnets, and particularly dual rail magnets, are most commonly manufactured to orient the accelerometer perpendicular to the plane of the surface to which the magnet will be attached. Thus, in this case, a secondary angle bracket or mount would have been required to achieve vertical accelerometer orientation. Magnetic mounting is also known to have the potential to generate very high acceleration levels (shock) during mounting and special precautions or shock-protected accelerometers can be required. Magnetic mounting was used for the horizontal accelerometers during well monitoring for the T5 test pile site (Wells #3 and #4). A concern with vertical mounting of accelerometers on a magnetic mount with an additional bracket or block mount was the potential for amplifying or damping vertical vibrations through the accelerometer inertia and horizontal moment arm generated by the magnet and mounting bracket dimensions.





Given the conditions and mounting concerns identified above, for field deployment the accelerometers were securely coupled to the domestic well casings using steel hose clamps. The steel hose clamps permitted control of the accelerometer orientation and coupling to a vertical cylindrical well casing(s) of various diameters that could be field-fit. During field monitoring of one well, the thin synthetic cover was left on the accelerometer for Wells #1 and #2 to both protect it from the hose clamp and, when subject to the high compressive stresses induced by the hose clamp, provide additional frictional resistance to any differential movement.

To address possible concerns about the potential for this coupling mechanism to adversely affect data, the hoseclamp coupling and field deployment of the sensors was mimicked on a controlled vibration induction system. This system included:

- APS 400 ELECTRO-SEIS® Long Stroke Shaker with Linear Ball Bearings, manufactured by APS Dynamics, Inc. is a subsidiary of SPEKTRA GmbH Dresden, Germany (see specification sheet, this Appendix);
- APS 0412 Reaction Mass Assembly (see specification sheet, this Appendix); and
- a piece of steel pipe, similar in diameter and wall thickness to the well casings;

The steel pipe section was fixed to the moving arm of the APS400 shaker unit with an angle bracket and bolt interior to the pipe, steel hose clamps and, for added security to avoid any tilting as compared to the arm, large plastic zip ties were used. Accelerometers (PCB393A03) were attached to the system, all with a vertical orientation, as follows:

- 1) accelerometer with synthetic cover attached to pipe with hose clamp;
- 2) accelerometer without synthetic cover attached to pipe with hose clamp;
- 3) accelerometer without synthetic cover attached to pipe with two ceramic magnets and angle bracket;
- 4) accelerometer attached to moving arm of the ASP400 shaker unit with mounting stud.

Photos of the laboratory configuration are provided below. Data from the accelerometers was gathered using the Rion DA21 unit.

During testing, the pipe and accelerometer assembly were subjected to controlled vibrations for a duration of 1 to 2 minutes and repeated several times. Shaker unit settings for frequency and force are in the table below. We note that at an acceleration value of about 0.5 g, the magnetic mounting system was shaken off the casing.



Frequency (Hz)	g ¹ (approximate)	PPV (mm)/s
0.3	0.0047	24.5
1	0.4400	687.0
3	0.0300	15.6
10	0.0300	4.7
30	0.0300	1.6

Table 1: Summary of Shaker Unit Vibration Settings

Output from this experiment is illustrated by Figure G-1. The output illustrates that at an induced vibration frequency of 10 Hz, as controlled from the shaker unit, the output from the group of accelerometers mounted to the prototype well casing piece begins to depart from the output from the accelerometer mounted directly on the shaker unit arm in the frequency range above approximately 12 Hz. Similar departure of the results is illustrated at the induced vibration frequency of 30 Hz. However, in both these instances, all the accelerometers mounted on the prototype casing using different methods respond in a virtually identical manner (difference of less than about 10 per cent). Based on these results, Golder concluded that the differences in accelerometers were measuring vibration response characteristics associated with the mounting of the prototype well casing to the shaker unit. The controlled-vibration system demonstrated that the hose clamp mounting system was also reviewed with the accelerometer supplier who expressed no concern with the field system for the range of frequencies and measured for this project (see attached correspondence).

¹ The approximate value for gravitational acceleration shown in the table above is based on the voltage-controlled settings on the shaker unit and measurements made during the experiment with the fixed accelerometer based on voltage output readings.





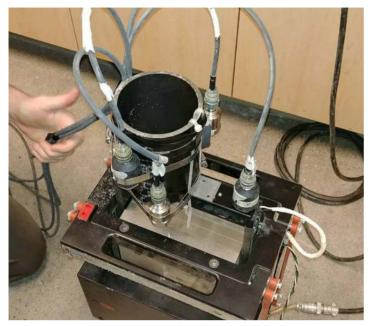


Figure G-1: Physical arrangement of shaker unit, pipe and four accelerometers prior to exposure to controlled vibrations.

n:\active\2016\3 proj\1668031 pattern_north kent vib monit_chatham-kent\ph 2000-vib monit field work\2-correspondence\5-rpts\5-revised final\appendix g\appendix g text.docx





APS 400 ELECTRO-SEIS® Long Stroke Shaker with Linear Ball Bearings

Page 1 of 5



The **APS 400 ELECTRO-SEIS**[®] shaker is a longstroke, electrodynamic force generator specifically designed to be used alone or in arrays for studying dynamic response characteristics of various structures. It finds use in modal excitation of complex structures, particularly when low frequencies are required. Furthermore it can be used for low frequency vibration testing of components and assemblies.

Applications

- Determination of natural mode frequencies, shapes, damping ratios, and stress distributions
- Excitation of manufactured equipment in the factory or installed in the field to demonstrate compliance with seismic specification criteria
- · Seismic simulation for components
- Test and calibration for seismic instruments
- Geological Services, Science, Physics and Seismic

Features

- Can be used to generate sine wave, swept sine wave, random or impulse force waveforms, fully adjustable at source
- Test set-up flexibility operates fixed body, free body, free armature
- Optimized to deliver power to resonant load with minimum shaker weight and drive power
- Adjustable armature re-centering for horizontal and vertical operation or other external pre-loads
- Two-Man Portability 73 kg (160 lb) total weight



APS 400 ELECTRO-SEIS[®] Long Stroke Shaker with Linear Ball Bearings

Description and Characteristics

The APS 400 ELECTRO-SEIS[®] shaker has been optimized for driving structures at their natural resonance frequencies. It is an electrodynamic force generator, the output of which is directly proportional to the instantaneous value of the current applied to it, independent of frequency and load response. It can deliver random or transient as well as sinusoidal waveforms of force to the load. The armature has been designed for minimum mass loading of the drive point. The ample armature stroke allows driving antinodes of large structures at low frequencies and permits rated force at low frequencies when operating in a free body mode.

The unit employs permanent magnets and is configured such that the armature coil remains in a uniform magnetic field over the entire stroke range assuring force linearity. The enclosed, self-cooled construction provides safety and minimum maintenance. Attachment of the armature to the drive point is accomplished by a simple thrust rod like the APS 8610 - Modal Stinger.

An amplifier, such as the APS 145 - Power Amplifier, is required to provide armature drive power.



APS 400 with APS 0412 Reaction Mass Assembly

Modes of Operation

Free Armature Mode

In this mode, the armature provides the reaction mass for force delivered to the test structure via the shaker body. Auxiliary reaction mass may be added to the armature to decrease the low frequency limit for rated force operation.

The APS 400 shaker and APS 0412 - Reaction Mass may be used in a vertical or horizontal free armature mode with rated force down to less than 3 Hz. Feet and carrying handles are provided for ease in placement of the shaker on horizontal test surfaces.

Fixed Body Mode

By providing a rigid attachment between the body and ground, the full relative velocity and stroke capability is available for load motion. Maximum rated force can be delivered down to 0.01 Hz and 70 % maximum to 0 Hz.



APS 420 with APS 4222 - Trunnion and APS 8610 Modal Stinger

Page 2 of 5



APS 400 ELECTRO-SEIS[®] Long Stroke Shaker with Linear Ball Bearings

Page 3 of 5

Free Body Mode

In this mode, the body provides the reaction mass. Load and body motion are accommodated within the total relative velocity and stroke. Because of the high cross-axis stiffness provided by the armature linear guidance system, the shaker may be supported above ground level by means of suspension lines (APS 8612 - Steel Cable Kit) attached to the body. This provides a convenient mounting for introducing force parallel to a horizontal mounting surface. Examples of such surfaces include floors, roofs, platforms, cabinets, bridges and tanks.



APS 420 with APS 8610 - Modal Stinger and APS 8612 - Steel Cable Kit prepared for Free Body Mode operation

Shaker Table Mode

Auxiliary Table Kits are available which, when installed on the basic shaker, enable the shaker to provide long stroke excitation to components or model structures mounted on the table.

The APS 0452 Auxiliary Table Kit provides horizontal motion, the APS 0477 Auxiliary Table Kit provides vertical motion and the APS 0478 Auxiliary Table Kit provides either the vertical or horizontal motion configuration.



APS 400 with APS 0452 - Auxiliary Table Kit - Horizontal



APS 400 with APS 0477 - Auxiliary Table Kit - Vertical



APS 400 ELECTRO-SEIS[®] Long Stroke Shaker with Linear Ball Bearings

Performance

The primary purpose of the APS 400 ELECTRO-SEIS[®] shaker is to determine the dynamic characteristics of mechanical structures. At resonance, a large amount of energy is contained in the structure, and the shaker must accommodate the resulting motion. However, it need only supply the real mechanical power dissipated by damping mechanisms within the structure.

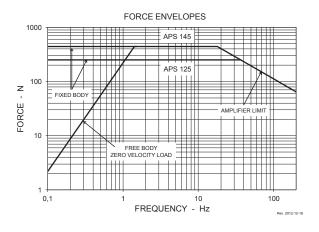
If a drive point on a structure in resonance is vibrating with a velocity of 1,000 mm/s (39 in/s) peak and a force of 445 N (100 lbf) peak is required to sustain the vibration level, then the shaker will be delivering approximately 220 W RMS to the structure. Such a load on the shaker is termed a matched resonant load, and it is purely resistive since the force is in a phase with the velocity.

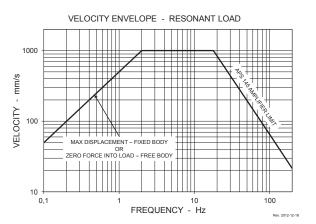
If the resonant load input is other than 445 N x 1,000 mm/s, the full 220 watts of mechanical power cannot be delivered to the structure, the system being either force or velocity limited. If the resulting maximum response level is not great enough, the user may have the option of moving the shaker to a drive point having an impedance closer to the matched value, or adding more shakers to the array driving the structure.

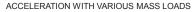
Within the limitations of maximum force and velocity, the actual power delivered to a structure is a function of the input mechanical impedance at the drive point. In typical modal testing, this input impedance varies widely in magnitude and phase angle. At different frequencies, the input impedance of the drive point may appear predominately springlike, mass-like, or resistive. Since the object of the tests is to establish resonant modes, at which the input mechanical impedance of all drive points are resistive, the shaker's maximum performance capability is most meaningful stated in terms of the force and velocity that can be obtained when driving a matched resistive load.

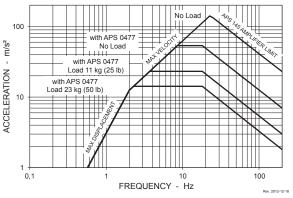
Therefore, performance is given in the form of graphs which present the envelopes of maximum force and velocity delivered to a resonant structure as functions of the resonance frequency of the structure.

Another application is the excitation for sensor calibration. Acceleration envelopes of the APS 400 ELECTRO-SEIS[®] shaker with various mass loads is shown in the lower graph for the 445 N rating.









Page 4 of 5



APS 400 ELECTRO-SEIS®

Long Stroke Shaker with Linear Ball Bearings

Page 5 of 5

Specifications

Shaker	APS 400
Force (Sine Peak)	445 N (100 lbf)
Velocity (Sine Peak)	1,000 mm/s (39 inch/s)
Stroke (Peak - Peak)	158 mm (6.25 inch)
Frequency Range	DC 200 Hz
Operation	horizontal or vertical
Armature Weight	2.8 kg (6.2 lb)
Max. Overhung Load at Armature Attachment Point	9.0 kg (20 lb)
DC Coil Resistance	1.6 Ω
Total Shaker Weight	73.0 kg (161 lb)
Shipping Weight	86.0 kg (190 lb)
Overall Dimension L x W x H	526 x 314 x 178 mm (20.7 x 12.4 x 7.0 inch)
Operating Temperature	5 40 degrees C
Storage Temperature	-25 55 degrees C

Accessories (optional)

Shaker	APS 400
Power Amplifier	APS 145
System Cable for Connecting Shaker to Amplifier	APS 0082-6E
Zero Position Controller for Vibration Exciters	APS 0109
Reaction Mass Assembly	APS 0412
Lifting Handles (Set of 4)	APS 0414
Carrying Handles and Tie-down Bars	APS 0421
Auxiliary Table Kit – Horizontal	APS 0452
Auxiliary Table Kit - Vertical	APS 0477
Auxiliary Table Kit - Horizontal and Vertical	APS 0478
Horizontal Reaction Mass System	APS 4001
Overtravel Switch	APS 8543
Modal Stinger Kit	APS 8610
Steel Cable Kit	APS 8612

Additional accessories available

Boone, Storer

From: Sent: To: Cc: Subject: Paul Gonsalves <pgonsalves@dalimar.ca> Tuesday, May 02, 2017 9:48 AM Boone, Storer Kiss, Jordan Mounting system opinion for Golder

Hi Storer,

You had asked for my opinion on a field method used to mount an accelerometer to a well casing for the purpose of monitoring ground vibrations.

The field mounting system in question consisted of:

- A steel hose clamp tightened to force the accelerometer into intimate contact with the steel well casing
- Leaving the thin black nylon protective cover on the accelerometer.

Background:

I understand that in this case that:

- It is not permissible to alter the steel well casing (e.g., drilling, tapping and treading a metal stud) or its aluminum lid
- Weather is cold, windy with occasional heavy rain, which rules out using wax or temporary glue to mount the accelerometer as they might fail
- Details of the well cover to well casing connection are unknown (e.g., looseness of fit, thread gauge, etc.)
- The range of frequencies of concern is 5 to 70 Hz and small fractions of gravitational acceleration (e.g., < 0.05 g) as might be associated with distant (e.g., >500 m) driving of foundation piles.

Conclusion:

While the hose-clamp mounting method is different than more typical approaches, I recognize that our instruments are adapted to address many challenging field conditions that require other mounting approaches.

If the range of frequency and amplitude concern were significantly greater than as noted above, I might have concern, but as it stands I'm comfortable with the set up proposed. The thin nylon cover is not readily compressible and, if it were to have any effect, might affect measurements at high frequencies (much greater than 100 Hz) and greater amplitudes.

As installed, I have no concern about the accelerometer readings being adversely affected by the mounting system and would fully expect the accelerometer movements to match the casing movements.

Best regards,

Paul Gonsalves Industrial Technical Sales C :647.226.3330

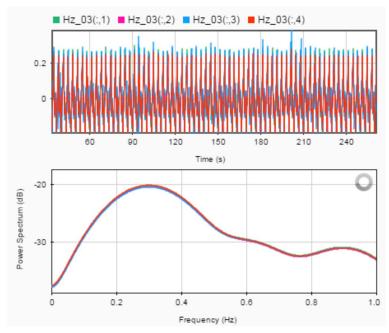


www.dalimar.ca - www.pcbpiezotronics.ca

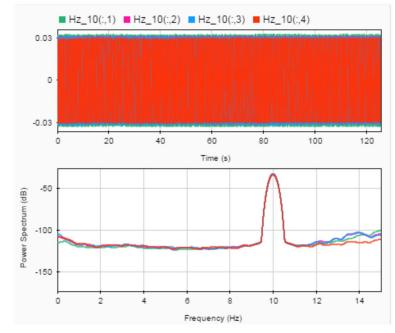


Best regards,

Paul Gonsalves



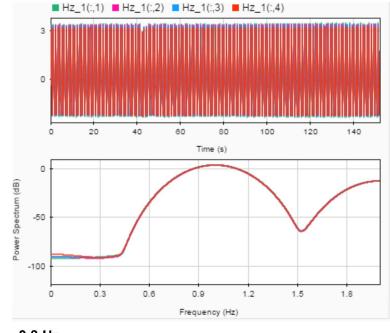




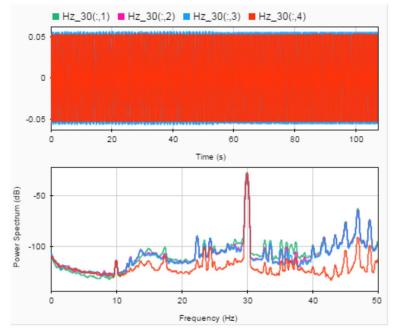


LEGEND

- GREEN AS MOUNTED TO WELL IN FIELD
- PINK AS MOUNTED WITHOUT RUBBER COVERING
- BLUE MAGNETIC COUPLING
- RED SENSOR FIXED TO SHAKER ARM



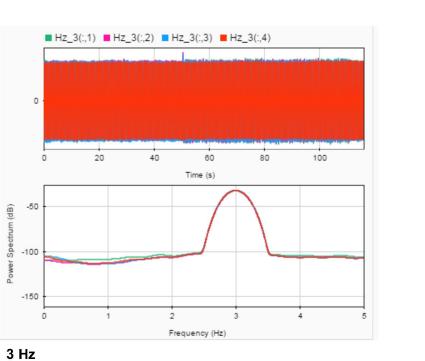
0.3 Hz



30 Hz

NOTES

THIS DRAWING IS SCHEMATIC ONLY AND IS TO BE READ IN CONJUNCTION WITH ACCOMPANYING TEXT.





ADD DCH Apr 28/17

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1668031

TILE No. 1668031-2000-R0100

FIGURE G-1

CALE AS SHOWN REV.

VIBRATION MONITORING PROGRAM NORTH KENT WIND FARM

ITLE

Golder

Associates

ROJECT

-50 m

-100



APPENDIX H

Instantel Minimate Data Reports



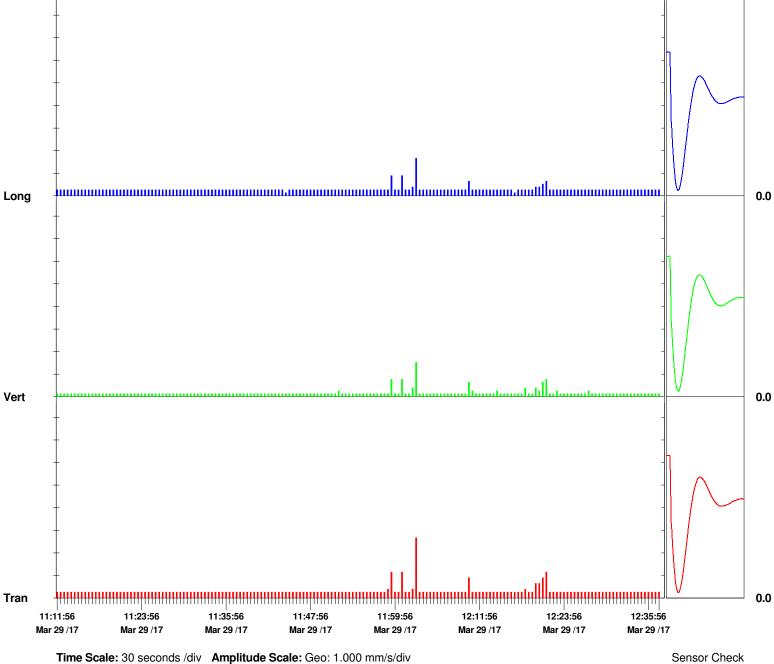


Histogram S Histogram F Number of Ir Range Sample Rate Notes	inish Time ntervals	11:11:26 March 29, 2017 12:37:21 March 29, 2017 2577.00 at 2 seconds Geo:254 mm/s 1024sps
Location: Client: User Name: General:	8100 Claym Patern Golder Asse North Kent	

BE15696 V 10.72-8.17 MiniMate Plus Serial Number **Battery Level** 6.2 Volts Unit Calibration February 22, 2017 by Instantel File Name Q696GTQF.320

	Tran	Vert	Long	
PPV	2.67	1.52	1.65	mm/s
ZC Freq	39	47	47	Hz
Date	Mar 29 /17	Mar 29 /17	Mar 29 /17	
Time	12:02:54	12:02:46	12:02:54	
Sensor Check	Passed	Passed	Passed	
Frequency	7.4	7.6	7.5	Hz
Overswing Ratio	4.0	3.8	4.2	

Peak Vector Sum 3.16 mm/s on March 29, 2017 at 12:02:54



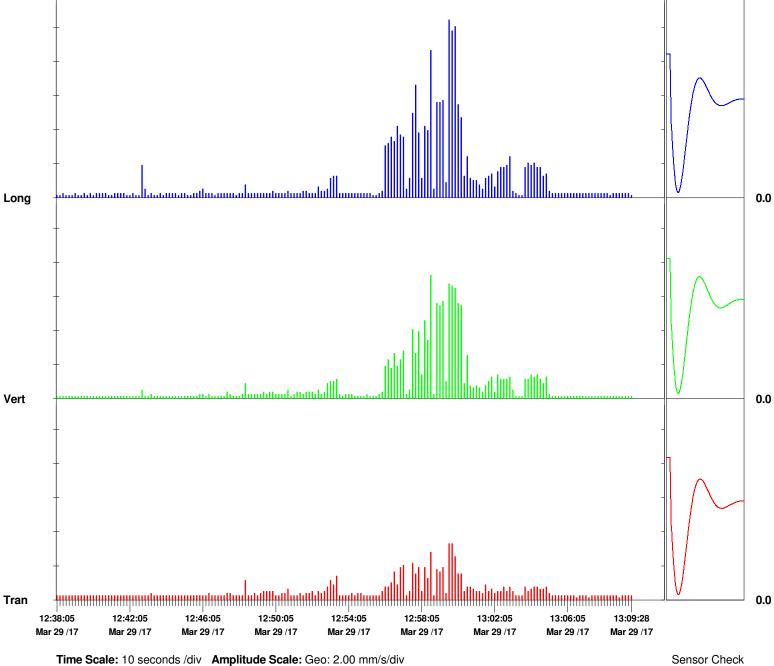


Histogram S Histogram F Number of Ir Range Sample Rate Notes	inish Time ntervals	12:37:55 March 29, 2017 13:09:28 March 29, 2017 946.00 at 2 seconds Geo:254 mm/s 1024sps
Location: Client: User Name: General:	8100 Clayn Patern Golder Ass North Kent	nore Rd ociates Ltd.

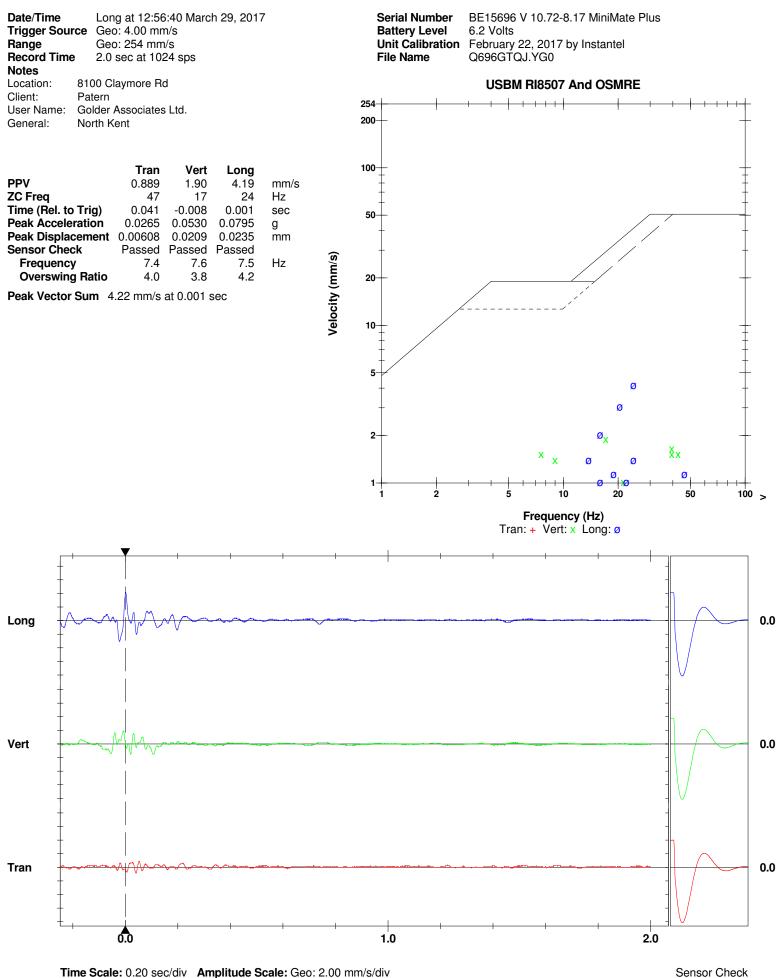
BE15696 V 10.72-8.17 MiniMate Plus Serial Number **Battery Level** 6.2 Volts Unit Calibration February 22, 2017 by Instantel File Name Q696GTQJ.370

	Tran	Vert	Long	
PPV	3.30	7.24	10.4	mm/s
ZC Freq	30	32	16	Hz
Date	Mar 29 /17	Mar 29 /17	Mar 29 /17	
Time	12:59:35	12:58:31	12:59:31	
Sensor Check	Passed	Passed	Passed	
Frequency	7.4	7.6	7.5	Hz
Overswing Ratio	4.0	3.8	4.2	

Peak Vector Sum 10.4 mm/s on March 29, 2017 at 12:59:33







Trigger = 🕨 Printed: April 24, 2017 (V 10.60 - 10.60) Sensor Check



FFT Report

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 Long at 12:56:40 March 29, 2017

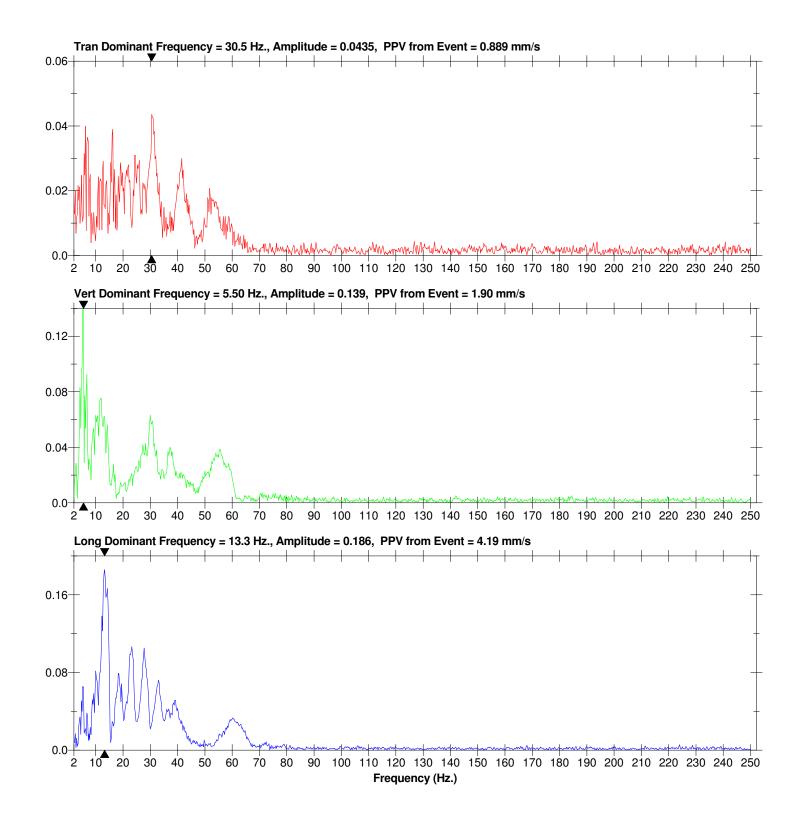
 Trigger Source
 Geo: 4.00 mm/s

 Range
 Geo: 254 mm/s

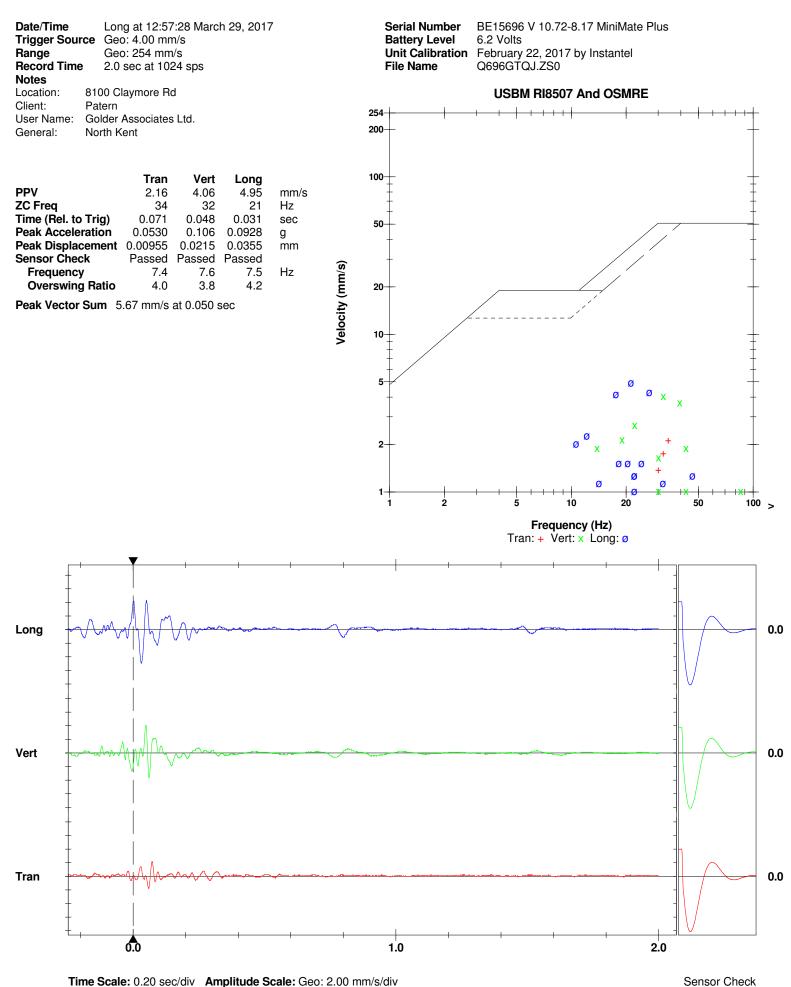
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQJ.YG0

NotesLocation:8100 Claymore RdClient:PaternUser Name:Golder Associates Ltd.General:North Kent









Notes

Location: Client:

General:

User Name:

FFT Report

 Date/Time
 Long at 12:57:28 March 29, 2017

 Trigger Source
 Geo: 4.00 mm/s

 Range
 Geo: 254 mm/s

 Record Time
 2.0 sec at 1024 sps

8100 Claymore Rd

Golder Associates Ltd.

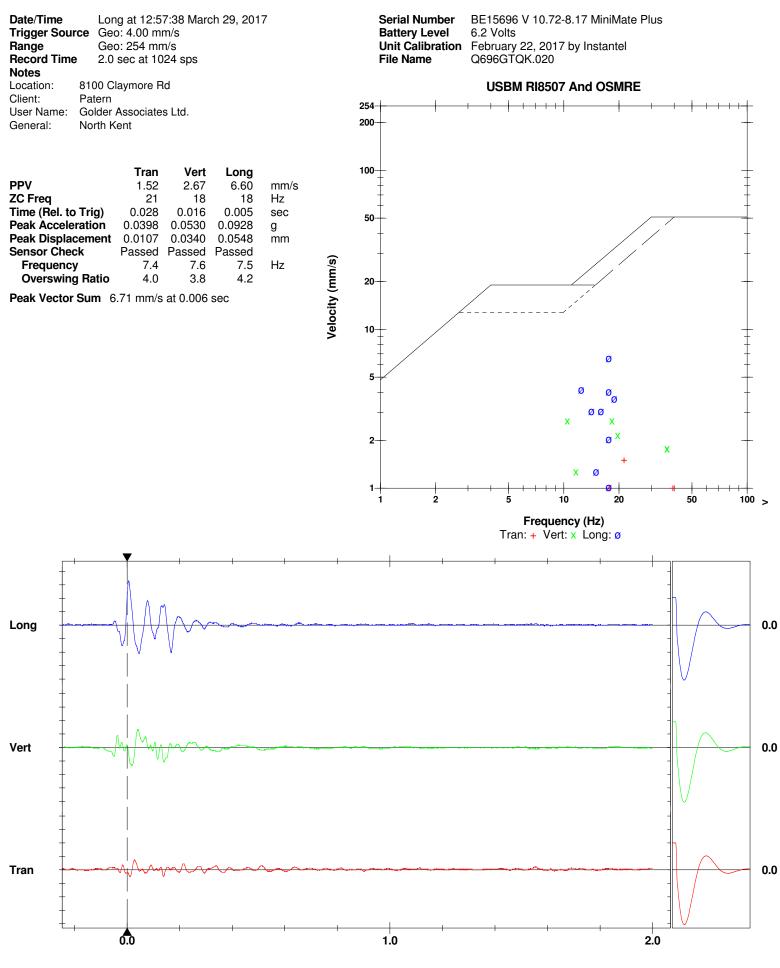
Patern

North Kent

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQJ.ZS0

Tran Dominant Frequency = 16.8 Hz., Amplitude = 0.0710, PPV from Event = 2.16 mm/s 0.08-0.06 0.04 0.02 0.0 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 2 10 20 30 40 50 60 70 80 Vert Dominant Frequency = 5.25 Hz., Amplitude = 0.211, PPV from Event = 4.06 mm/s 0.40+++ 0.35 0.30-0.25-0.20-0.15 0.10 0.05 0.0 2 20 30 40 50 60 70 80 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 10 Long Dominant Frequency = 14.3 Hz., Amplitude = 0.264, PPV from Event = 4.95 mm/s 0.40+ _ _ ____ _ 0.35-0.30-0.25 0.20 0.15 0.10 0.05 0.0 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 2 20 30 40 50 60 70 80 10 Frequency (Hz.)





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger =



 Date/Time
 Long at 12:57:38 March 29, 2017

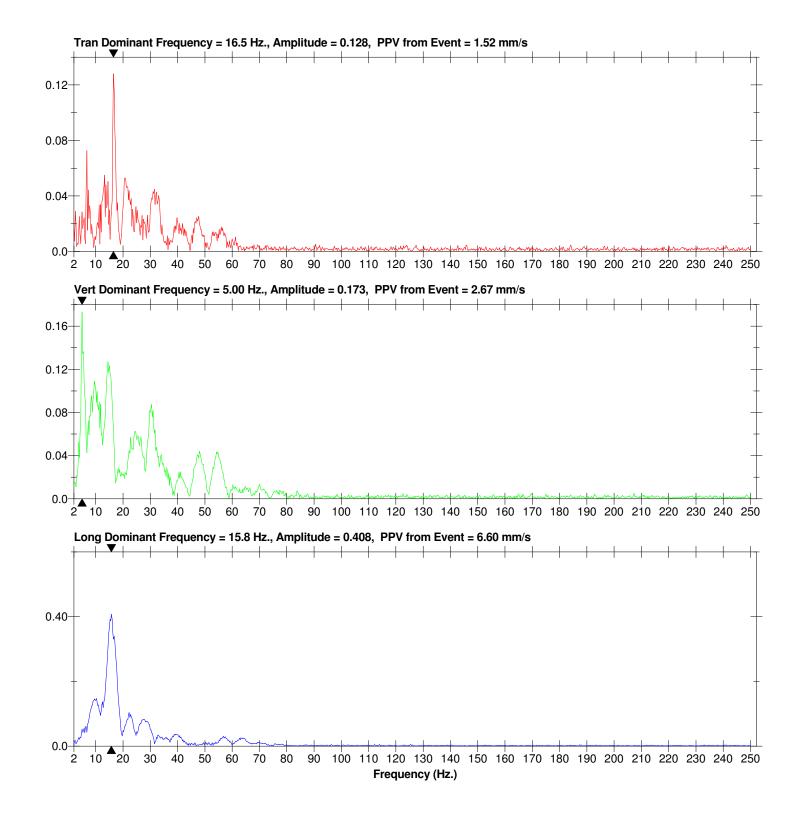
 Trigger Source
 Geo: 4.00 mm/s

 Range
 Geo: 254 mm/s

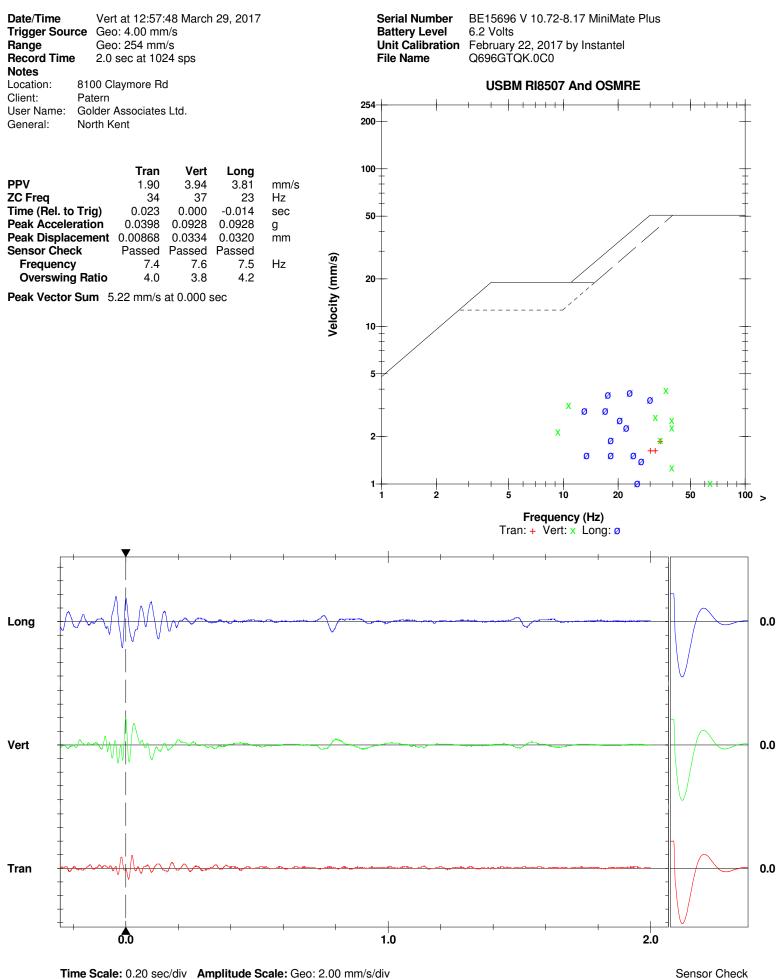
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Notes







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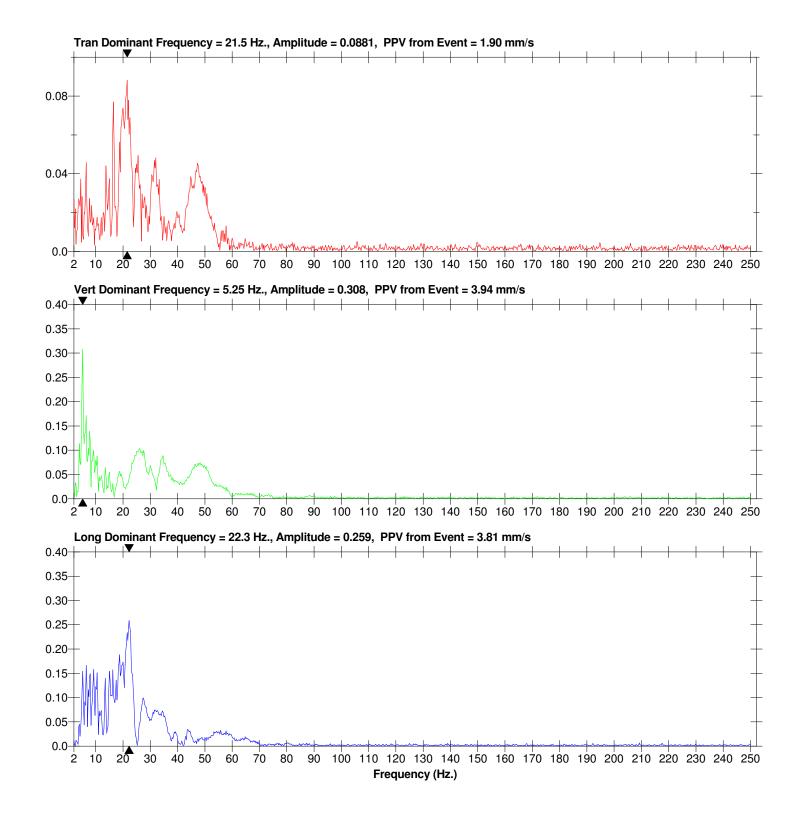
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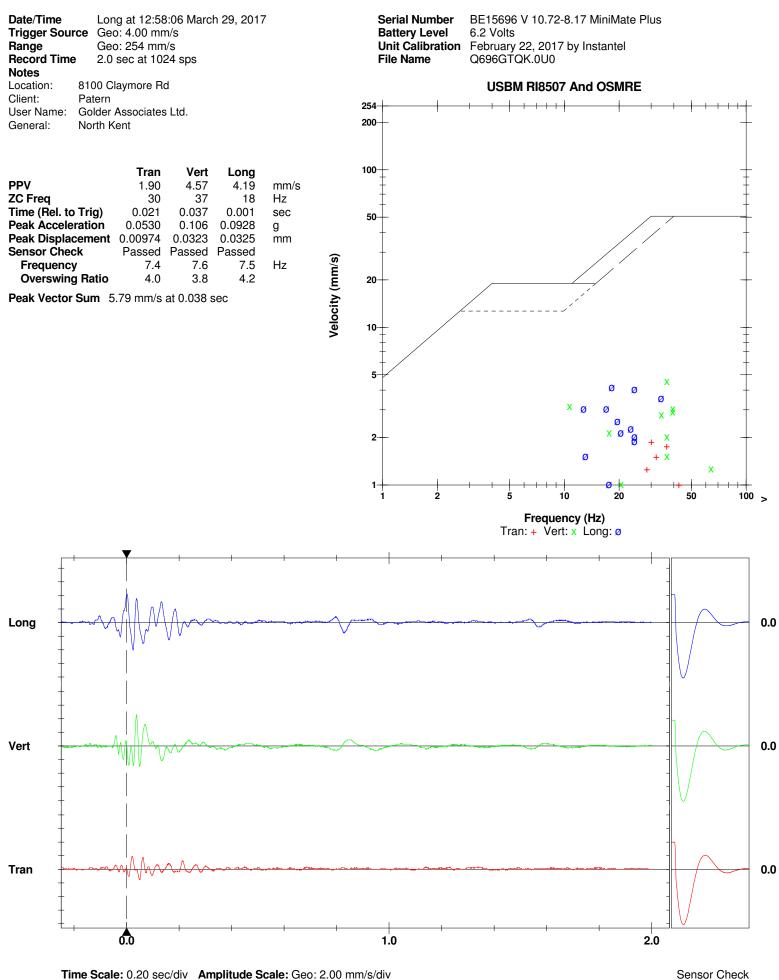
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Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQK.0C0

Notes







Trigger = 🕨 Printed: April 24, 2017 (V 10.60 - 10.60)

Format © 1995-2013 Xmark Corporation

Sensor Check



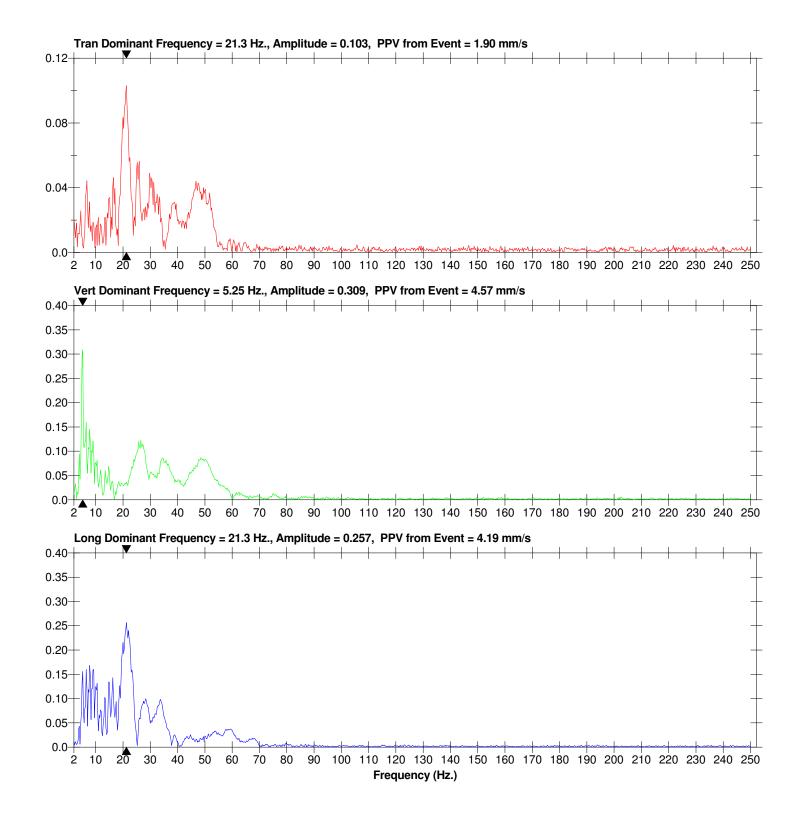
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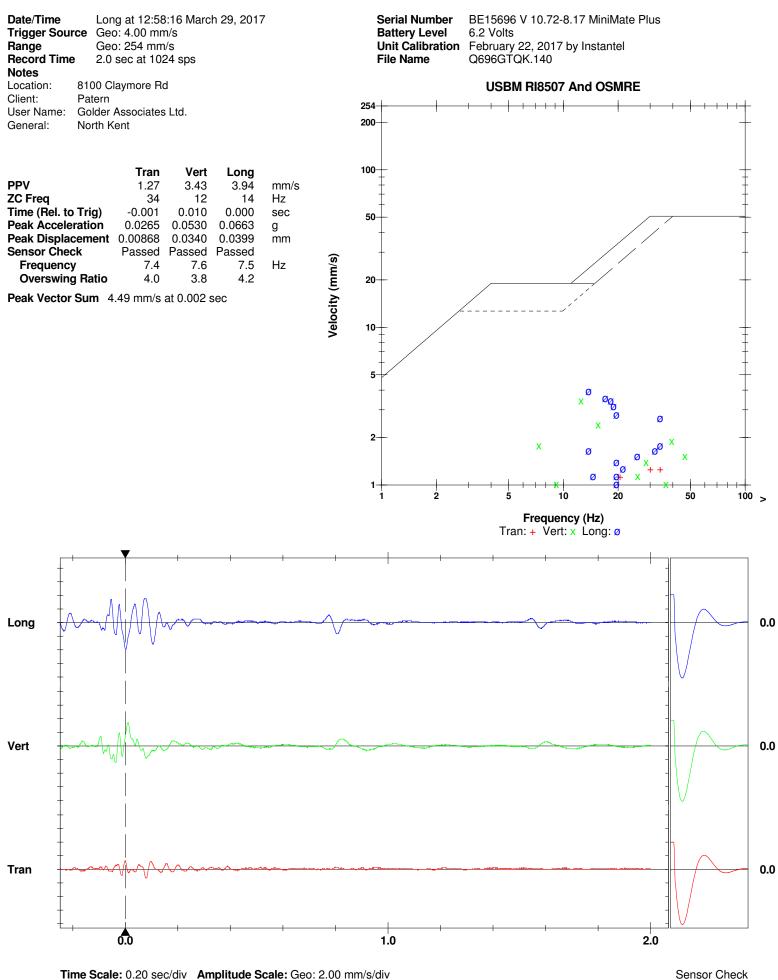
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Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQK.0U0







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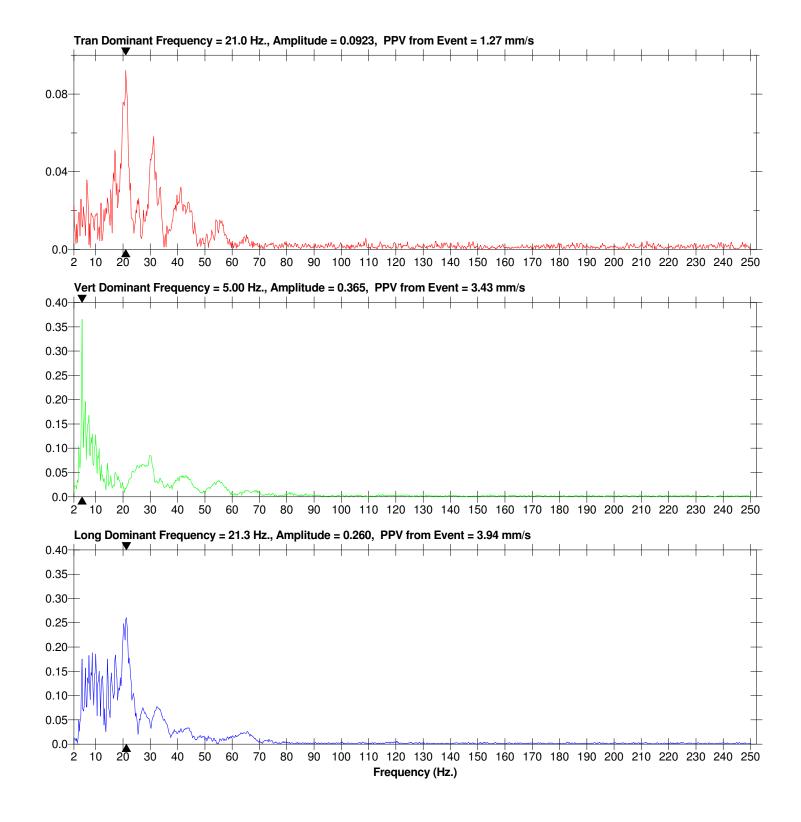
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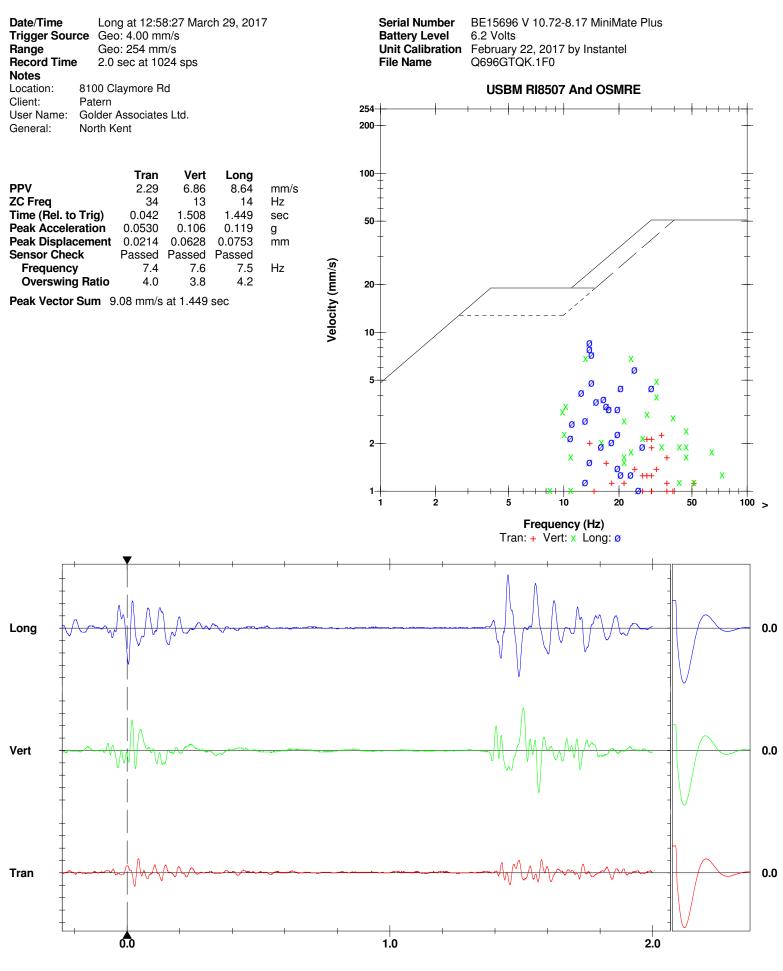
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Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQK.140











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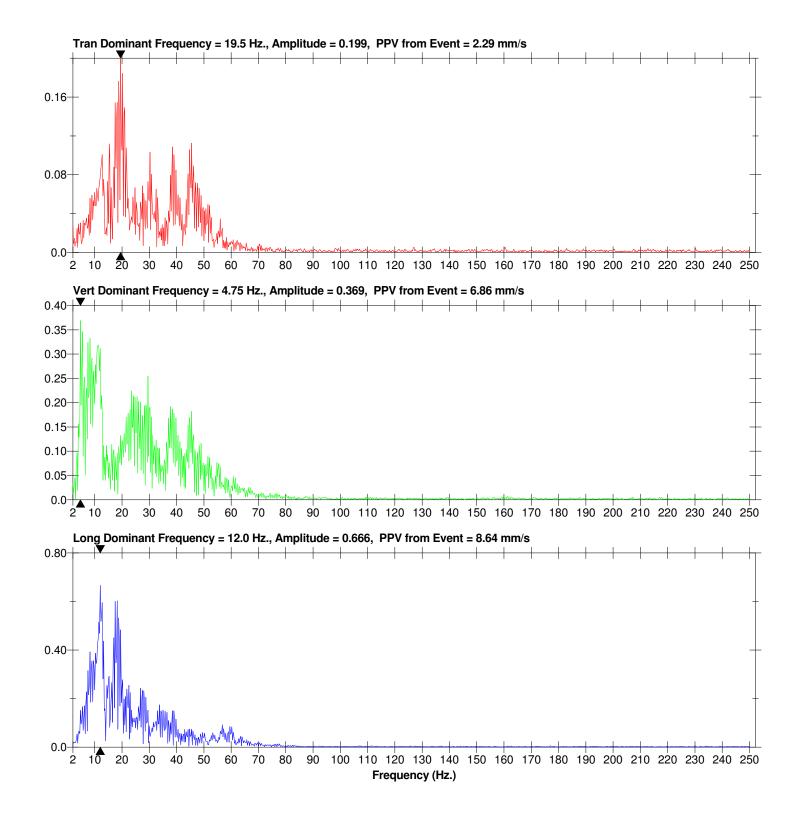
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 Range
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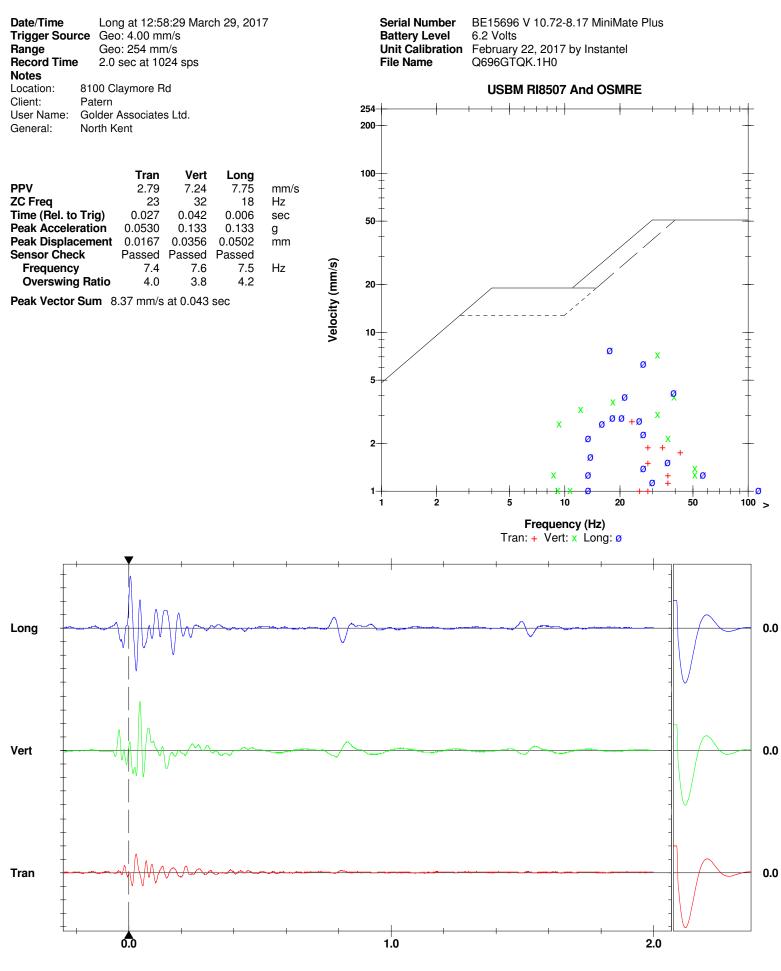
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Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQK.1F0

Notes







Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger =



 Date/Time
 Long at 12:58:29 March 29, 2017

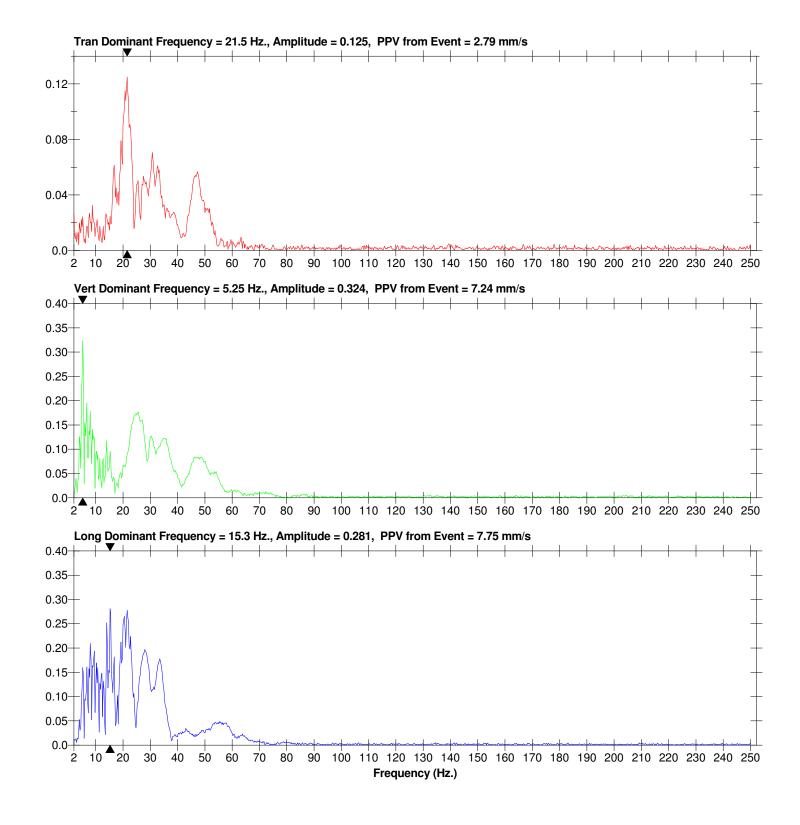
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 Range
 Geo: 254 mm/s

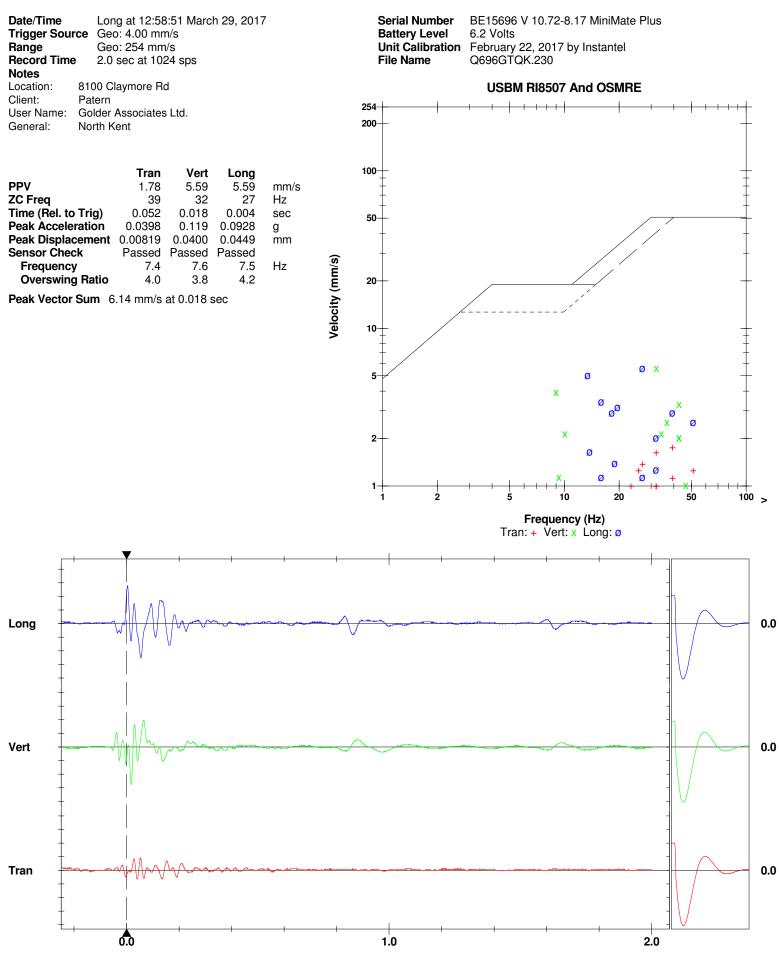
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Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQK.1H0









Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger = ▶ ____ ◀



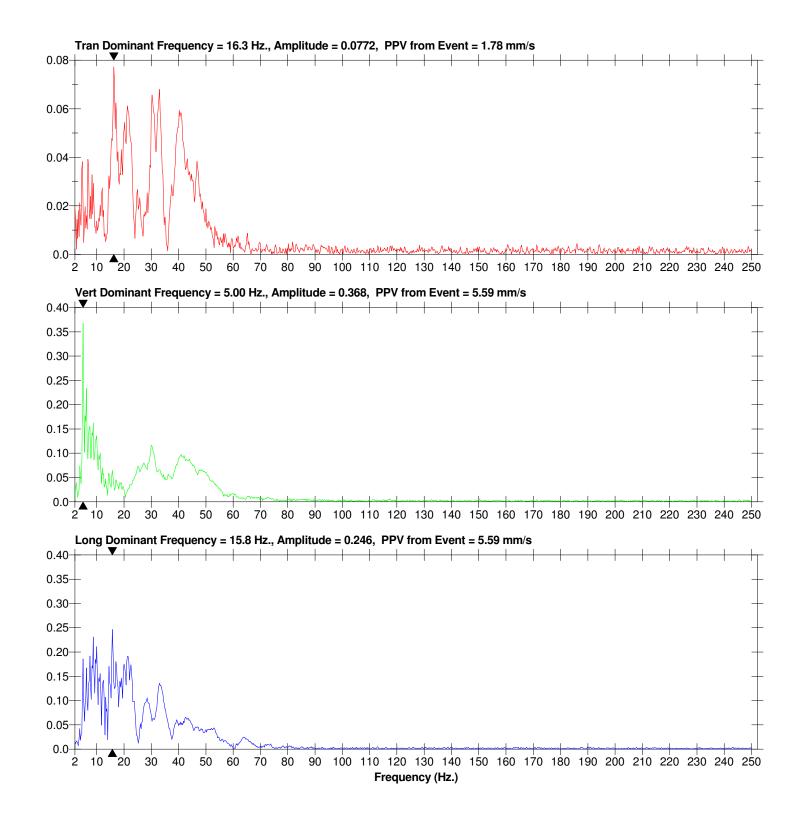
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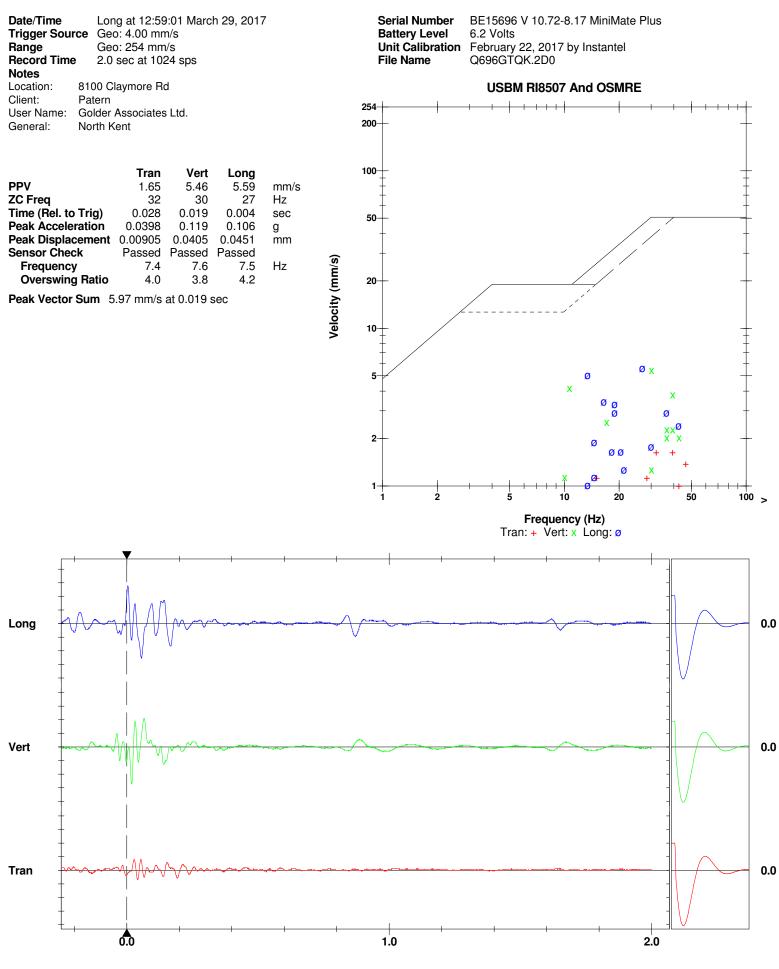
 Range
 Geo: 254 mm/s

 Record Time
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Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQK.230







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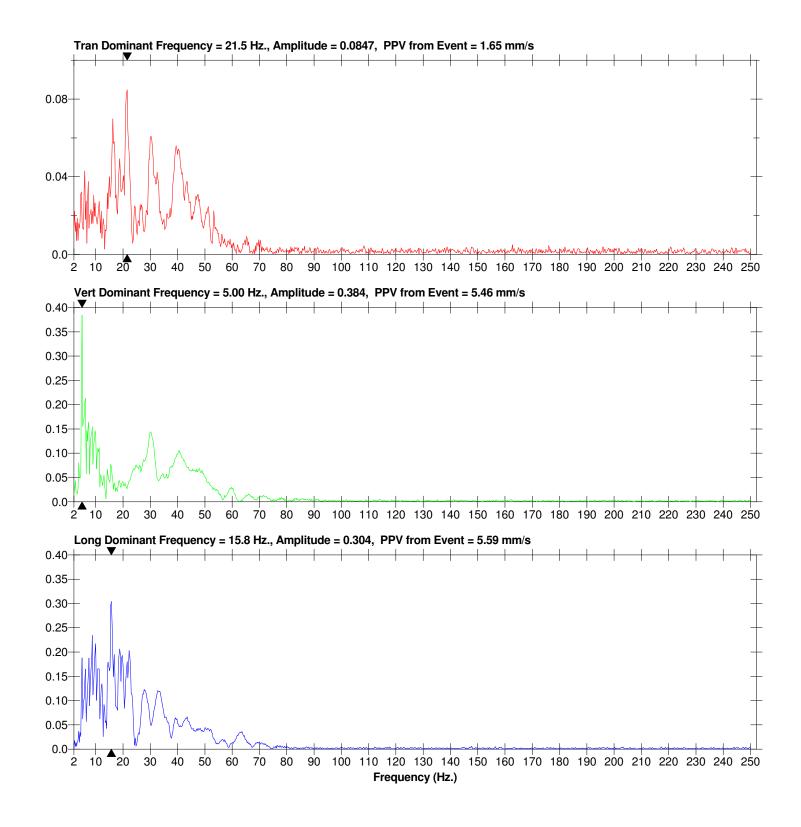
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 Range
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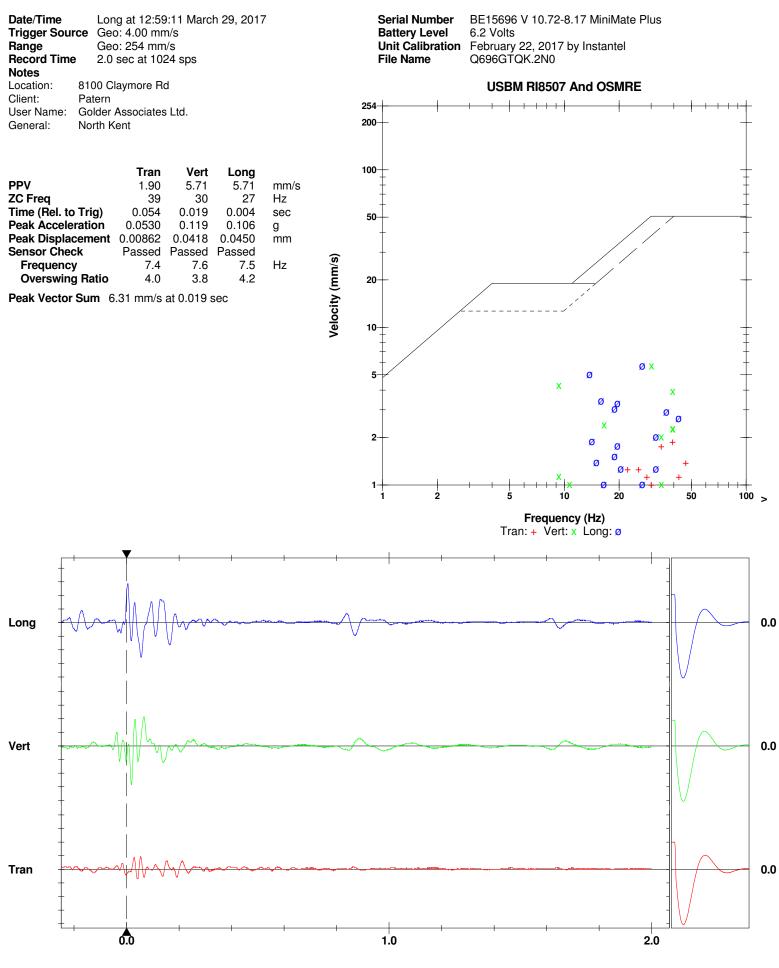
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Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQK.2D0

Notes







Trigger = ► _____ Printed: April 24, 2017 (V 10.60 - 10.60)

Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div

Format © 1995-2013 Xmark Corporation

Sensor Check



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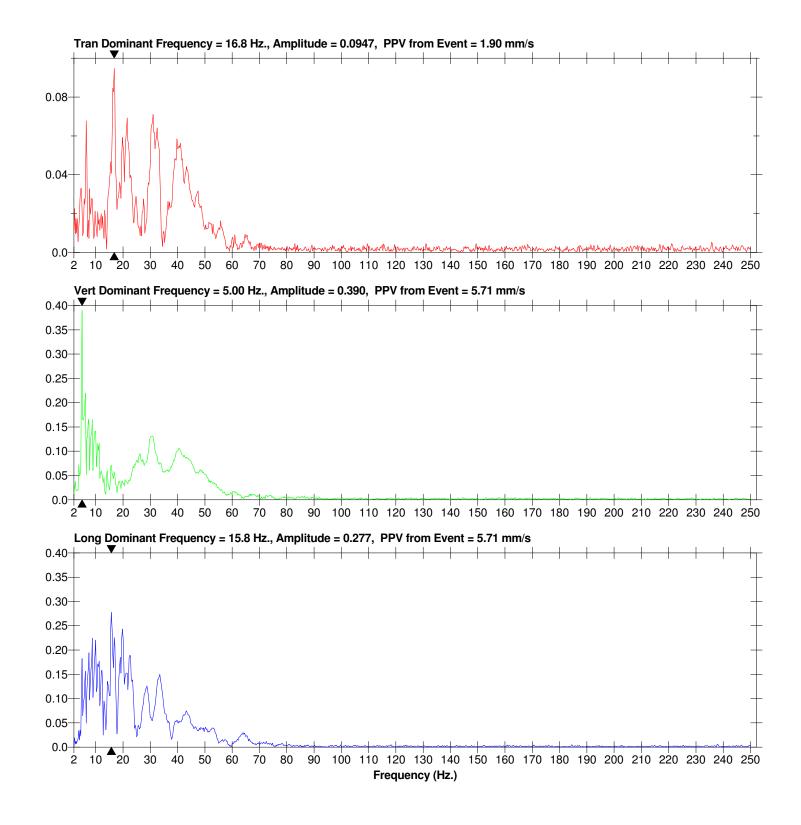
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 Geo: 4.00 mm/s

 Range
 Geo: 254 mm/s

 Record Time
 2.0 sec at 1024 sps

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQK.2N0

Notes



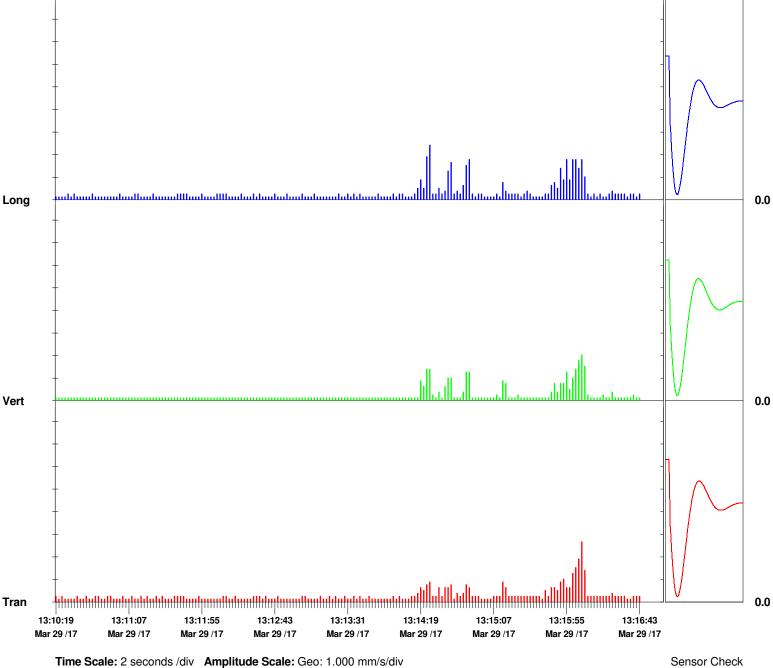


Histogram S Histogram F Number of Ir Range Sample Rate Notes	inish Time ntervals	13:10:17 March 29, 2017 13:16:43 March 29, 2017 193.00 at 2 seconds Geo:254 mm/s 1024sps
Location: Client: User Name: General:	8100 Claym Patern Golder Asse North Kent	

BE15696 V 10.72-8.17 MiniMate Plus Serial Number **Battery Level** 6.2 Volts Unit Calibration February 22, 2017 by Instantel File Name Q696GTQK.L50

	Tran	Vert	Long	
PPV	2.67	2.03	2.41	mm/s
ZC Freq	43	39	13	Hz
Date	Mar 29 /17	Mar 29 /17	Mar 29 /17	
Time	13:16:05	13:16:05	13:14:25	
Sensor Check	Passed	Passed	Passed	
Frequency	7.4	7.6	7.5	Hz
Overswing Ratio	4.0	3.8	4.2	

Peak Vector Sum 3.17 mm/s on March 29, 2017 at 13:16:05



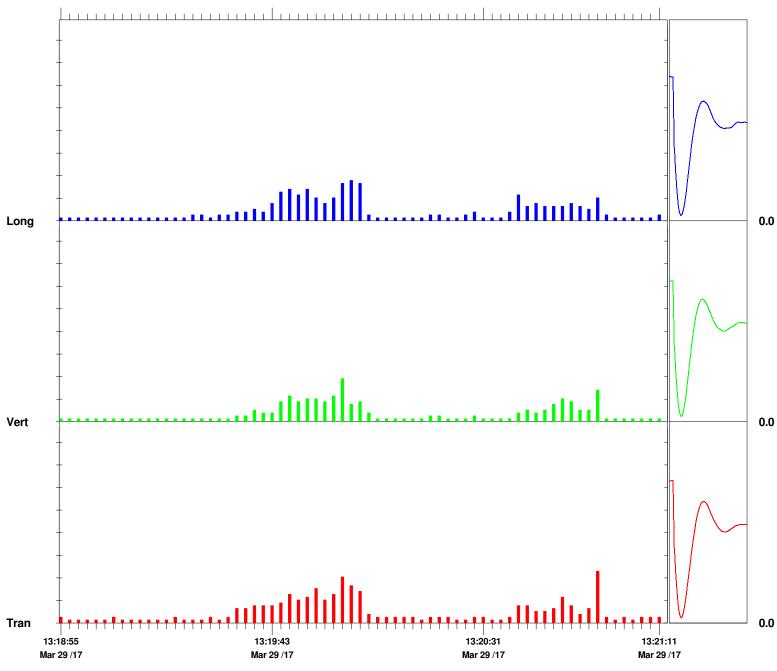


Histogram S Histogram F Number of Ir Range Sample Rate Notes	inish Time ntervals	13:18:53 March 29, 2017 13:21:12 March 29, 2017 69.00 at 2 seconds Geo:254 mm/s 1024sps
Location: Client: User Name: General:	8100 Claym Patern Golder Asso North Kent	

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQK.ZH0

	Iran	Vert	Long	
PPV	2.29	1.90	1.78	mm/s
ZC Freq	37	43	24	Hz
Date	Mar 29 /17	Mar 29 /17	Mar 29 /17	
Time	13:20:57	13:19:59	13:20:01	
Sensor Check	Passed	Passed	Passed	
Frequency	7.3	7.6	7.5	Hz
Overswing Ratio	3.8	3.7	4.2	

Peak Vector Sum 2.40 mm/s on March 29, 2017 at 13:19:59



Time Scale: 2 seconds /div Amplitude Scale: Geo: 1.000 mm/s/div

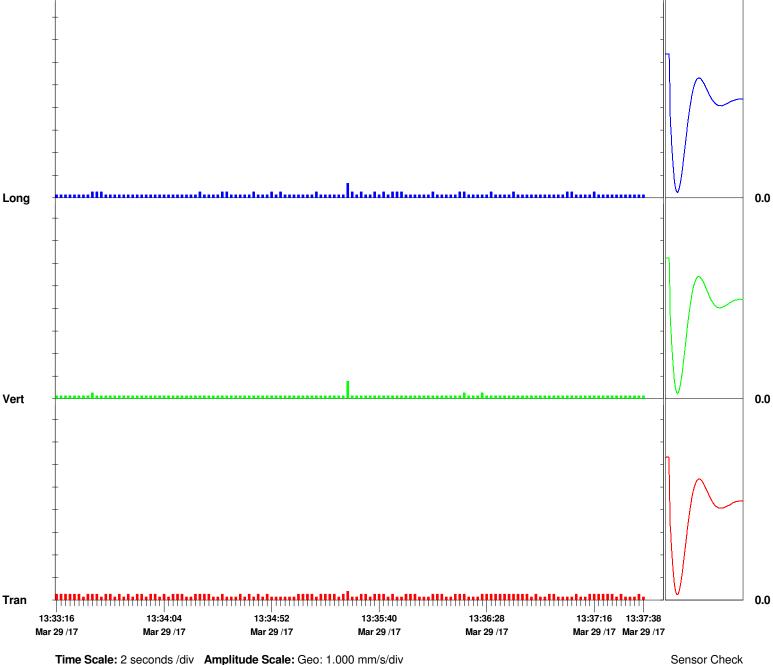


Histogram S Histogram F Number of Ir Range Sample Rate Notes	inish Time ntervals	13:33:14 March 29, 2017 13:37:40 March 29, 2017 132.00 at 2 seconds Geo:254 mm/s 1024sps
Location: Client: User Name: General:	8100 Claym Patern Golder Asse North Kent	

Serial Number BE15696 V 10.72-8.17 MiniMate Plus Battery Level Unit Calibration 6.2 Volts February 22, 2017 by Instantel Q696GTQL.NE0 File Name

	Tran	Vert	Long	
PPV	0.381	0.762	0.635	mm/s
ZC Freq	57	43	43	Hz
Date	Mar 29 /17	Mar 29 /17	Mar 29 /17	
Time	13:35:26	13:35:26	13:35:26	
Sensor Check	Passed	Passed	Passed	
Frequency	7.4	7.6	7.5	Hz
Overswing Ratio	4.0	3.8	4.2	

Peak Vector Sum 0.773 mm/s on March 29, 2017 at 13:35:26



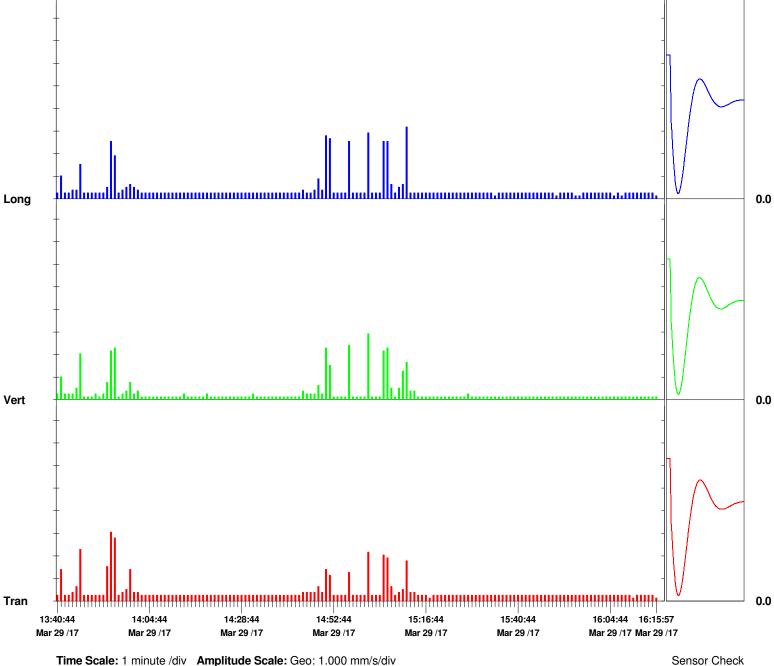


Histogram S Histogram F Number of Ir Range Sample Rate Notes	inish Time ntervals	13:39:44 March 29, 2017 16:15:57 March 29, 2017 4686.00 at 2 seconds Geo:254 mm/s 1024sps
Location: Client: User Name: General:	8100 Claym Patern Golder Asse North Kent	

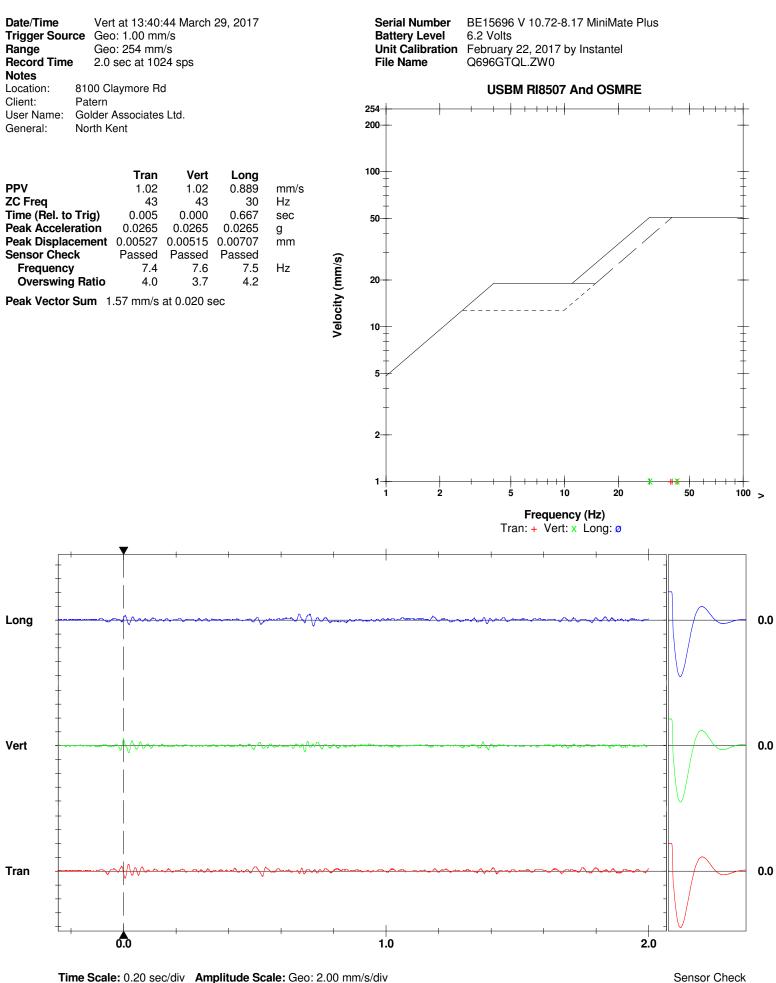
Serial Number BE15696 V 10.72-8.17 MiniMate Plus Battery Level Unit Calibration 6.2 Volts February 22, 2017 by Instantel File Name Q696GTQL.Y80

	Tran	Vert	Long	
PPV	3.05	2.92	3.17	mm/s
ZC Freq	47	27	47	Hz
Date	Mar 29 /17	Mar 29 /17	Mar 29 /17	
Time	13:54:42	15:01:18	15:11:38	
Sensor Check	Passed	Passed	Passed	
Frequency	7.4	7.6	7.5	Hz
Overswing Ratio	4.0	3.7	4.2	

Peak Vector Sum 3.63 mm/s on March 29, 2017 at 13:54:42







Trigger = 🕨 Printed: April 24, 2017 (V 10.60 - 10.60)

Format © 1995-2013 Xmark Corporation

Sensor Check



Notes

Location: Client:

General:

User Name:

FFT Report

 Date/Time
 Vert at 13:40:44 March 29, 2017

 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

 Record Time
 2.0 sec at 1024 sps

8100 Claymore Rd

Golder Associates Ltd.

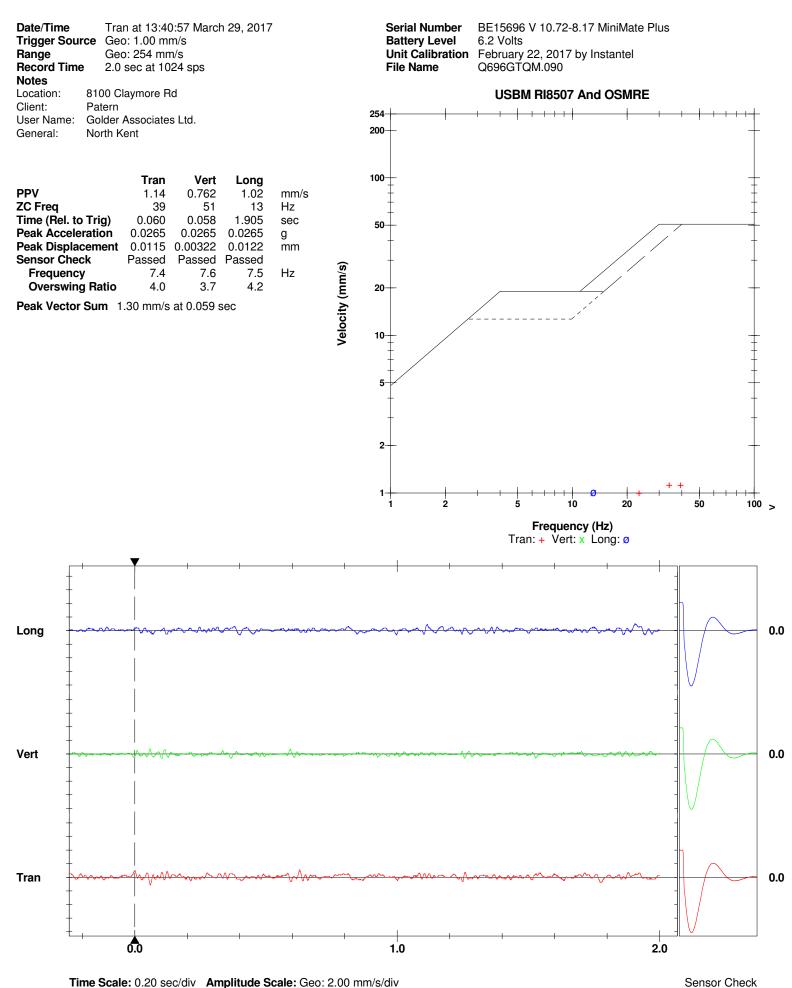
Patern

North Kent

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQL.ZW0

Tran Dominant Frequency = 39.5 Hz., Amplitude = 0.0754, PPV from Event = 1.02 mm/s 0.08 0.06 0.04 0.02 0.0 40 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 2 10 20 30 50 60 70 80 90 Vert Dominant Frequency = 33.8 Hz., Amplitude = 0.0528, PPV from Event = 1.02 mm/s 0.06-0.04 0.02 0.0 20 30 40 50 60 70 80 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 2 10 Long Dominant Frequency = 2.50 Hz., Amplitude = 0.0706, PPV from Event = 0.889 mm/s 0.08 +_ ____ ____ _ ____ _ ____ 0.06 0.04 0.02 0.0 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 20 30 40 50 70 80 10 60 Frequency (Hz.)







 Date/Time
 Tran at 13:40:57 March 29, 2017

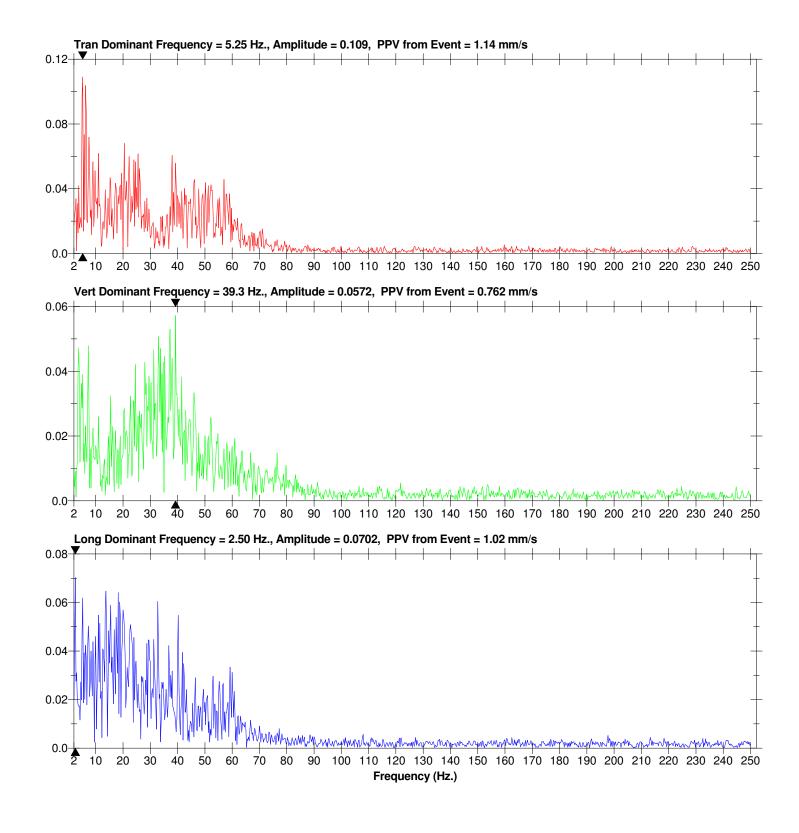
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

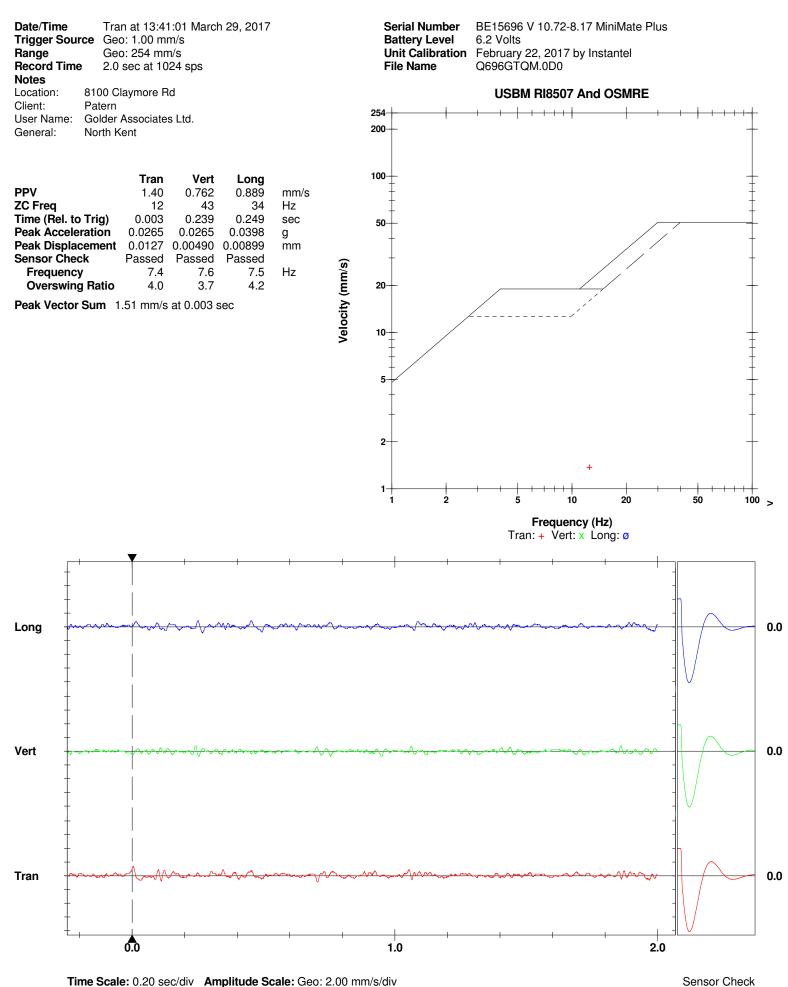
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQM.090

Notes









 Date/Time
 Tran at 13:41:01 March 29, 2017

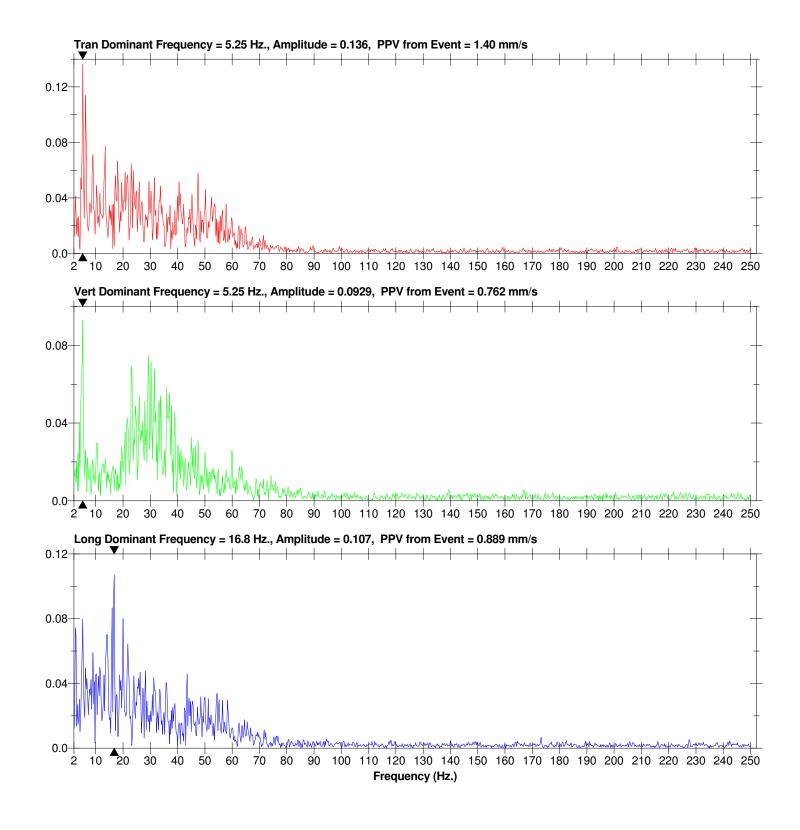
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

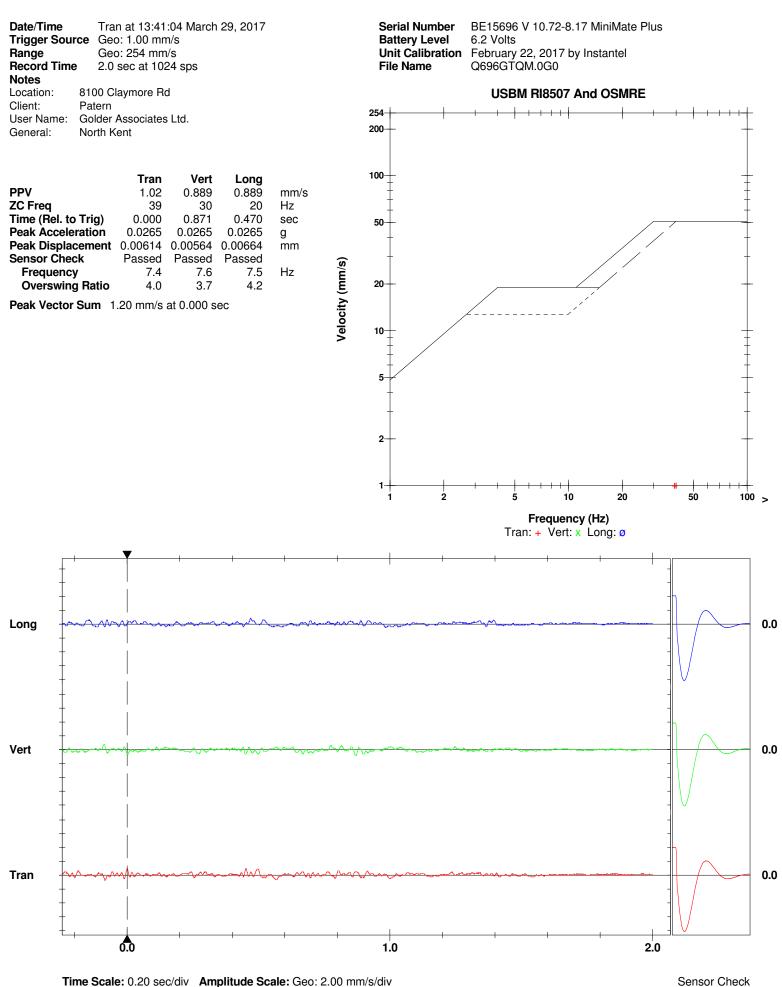
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQM.0D0

Notes







Trigger = ► _____ Printed: April 24, 2017 (V 10.60 - 10.60)



 Date/Time
 Tran at 13:41:04 March 29, 2017

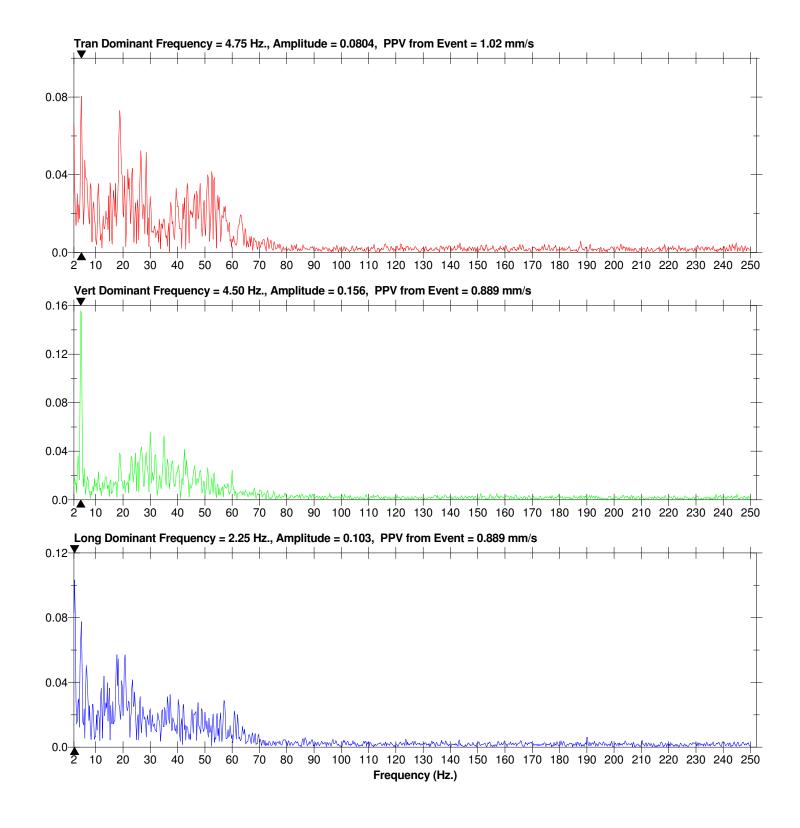
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

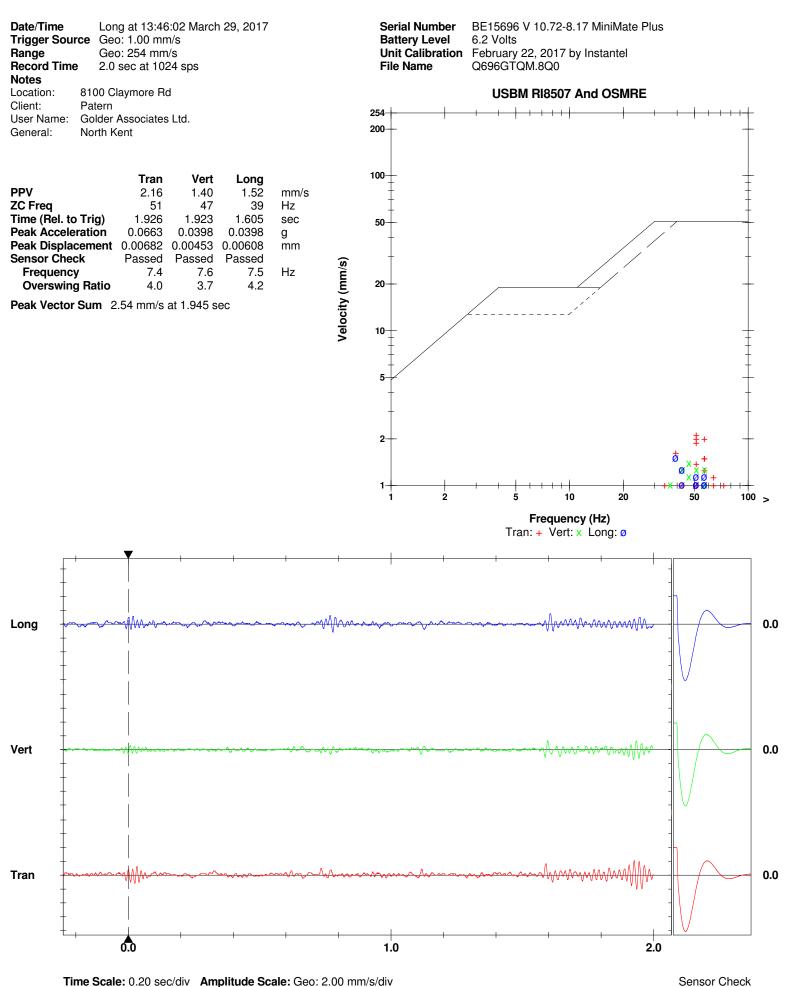
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQM.0G0

Notes







Trigger = ► _____ Printed: April 24, 2017 (V 10.60 - 10.60)

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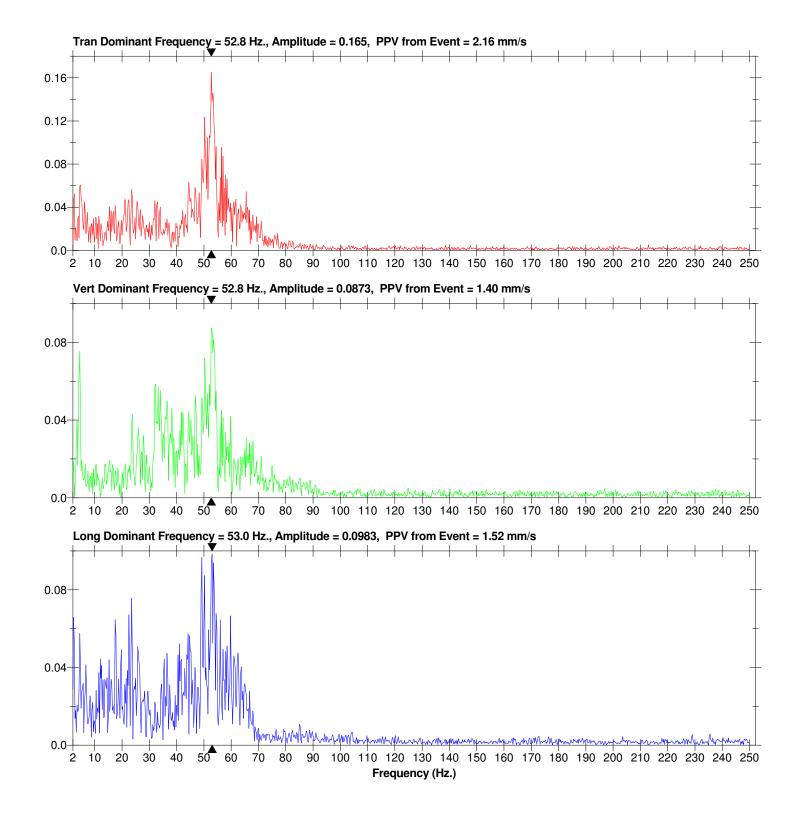
 Date/Time
 Long at 13:46:02 March 29, 2017

 Trigger Source
 Geo: 1.00 mm/s

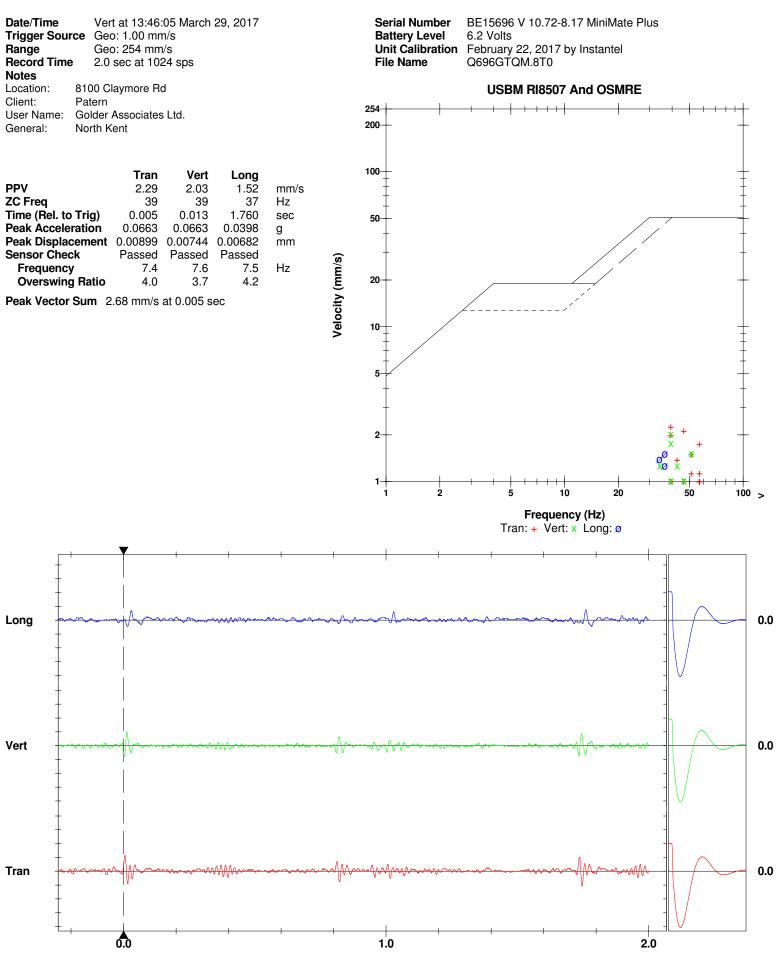
 Range
 Geo: 254 mm/s

 Record Time
 2.0 sec at 1024 sps

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQM.8Q0







Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger =



 Date/Time
 Vert at 13:46:05 March 29, 2017

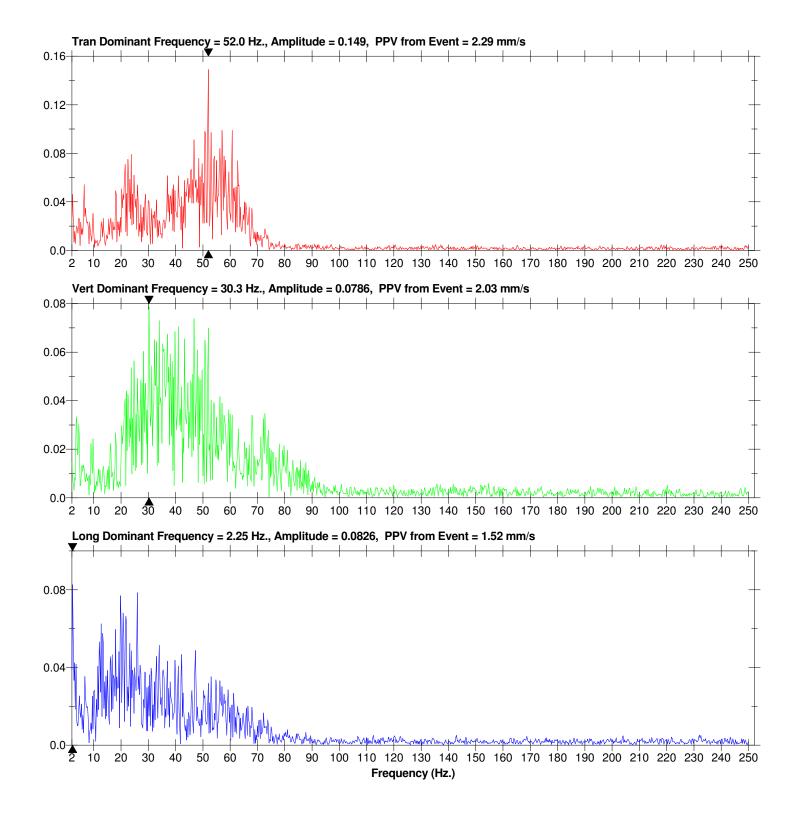
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

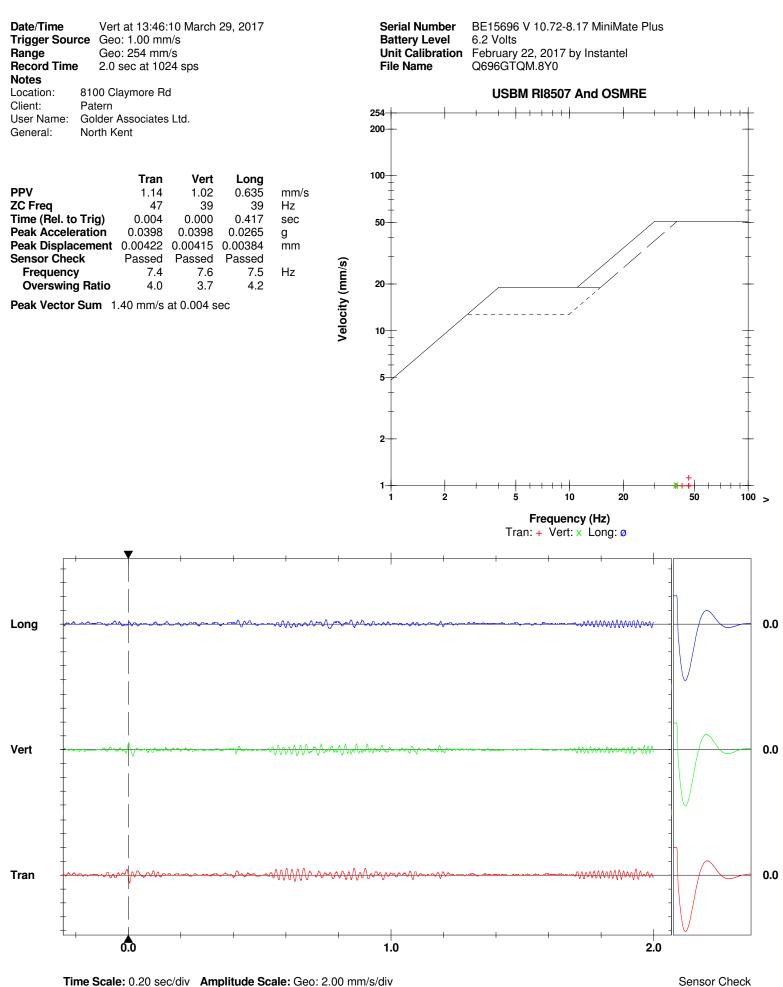
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQM.8T0

Notes









 Date/Time
 Vert at 13:46:10 March 29, 2017

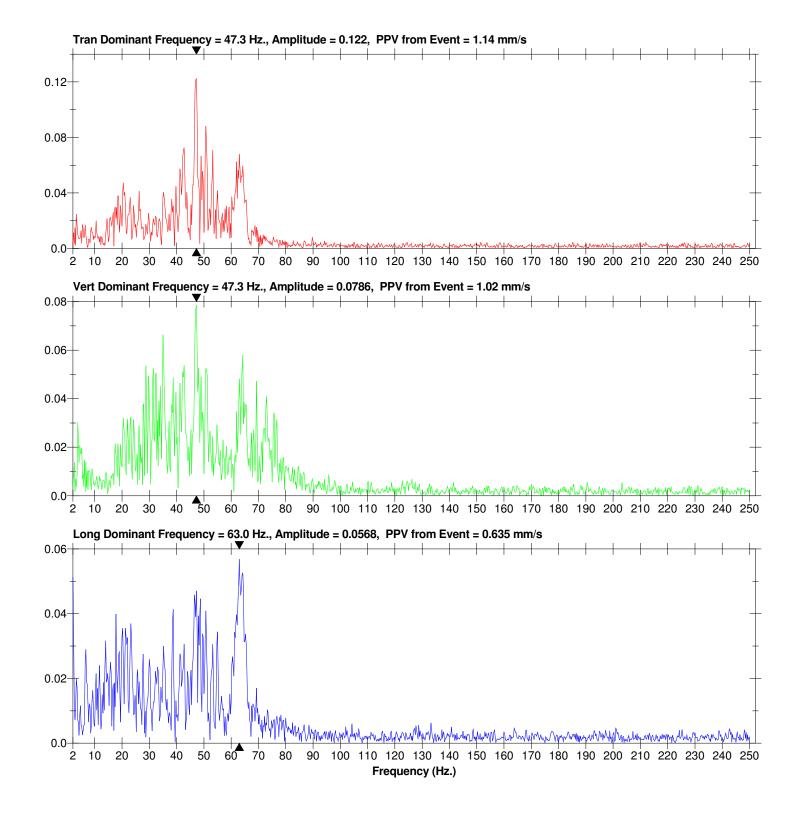
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

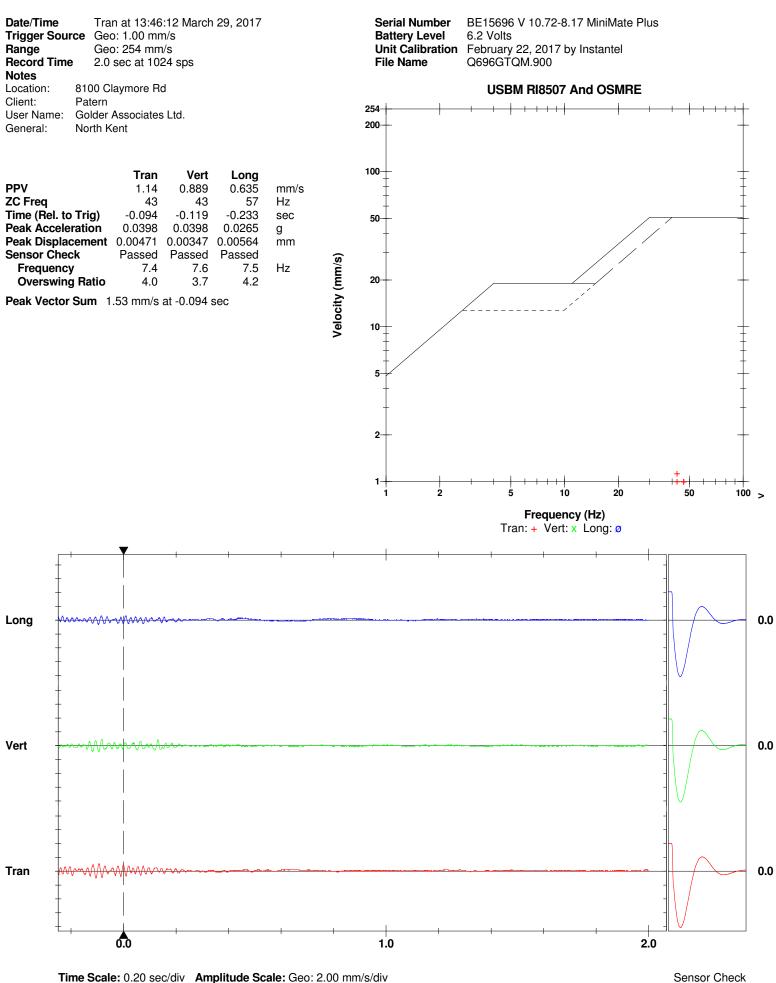
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQM.8Y0

Notes







Trigger = 🕨 Printed: April 24, 2017 (V 10.60 - 10.60) Sensor Check



 Date/Time
 Tran at 13:46:12 March 29, 2017

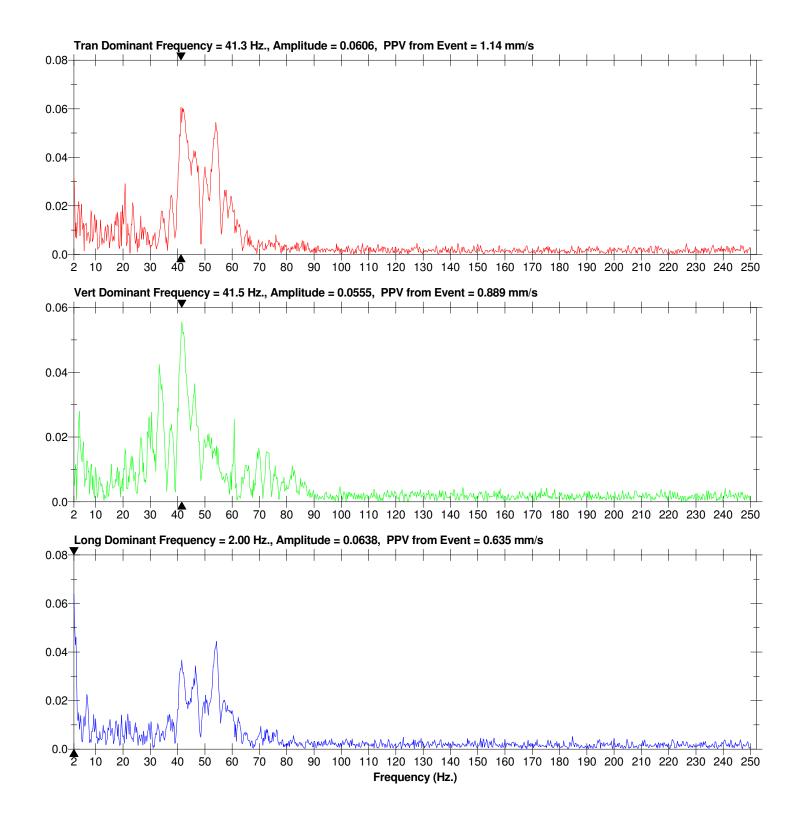
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

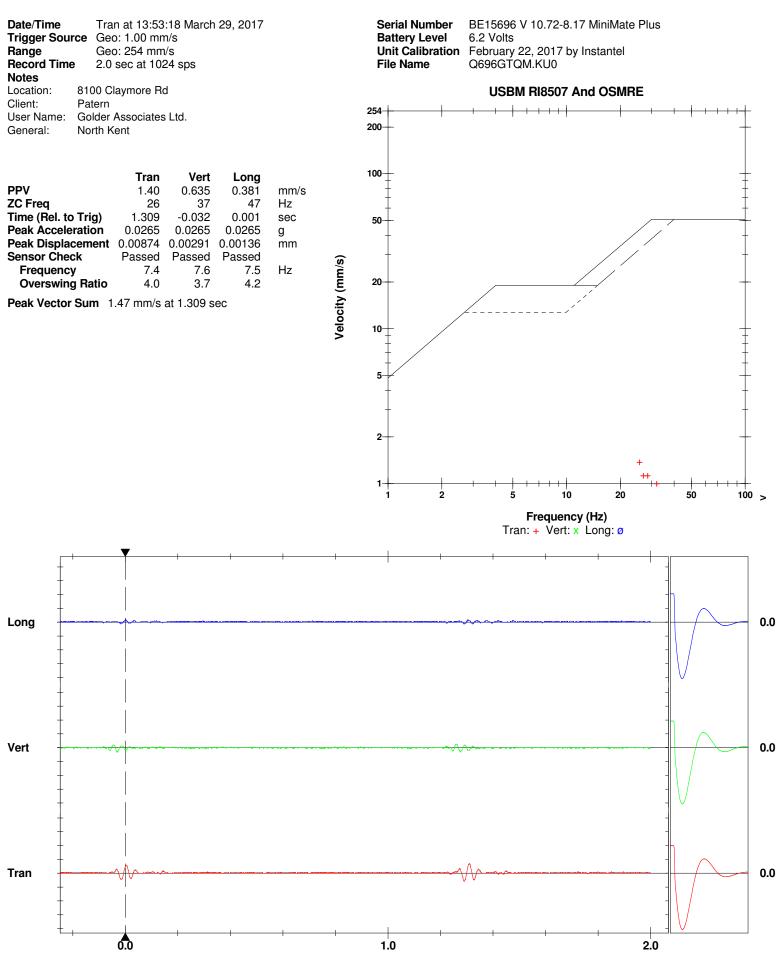
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQM.900









Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger = > _____



 Date/Time
 Tran at 13:53:18 March 29, 2017

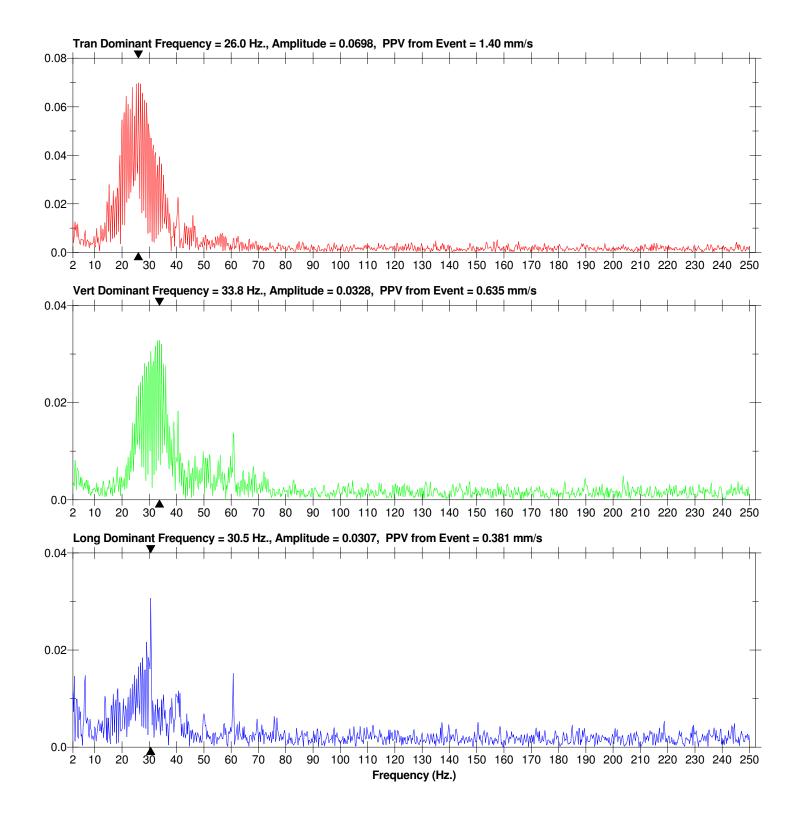
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

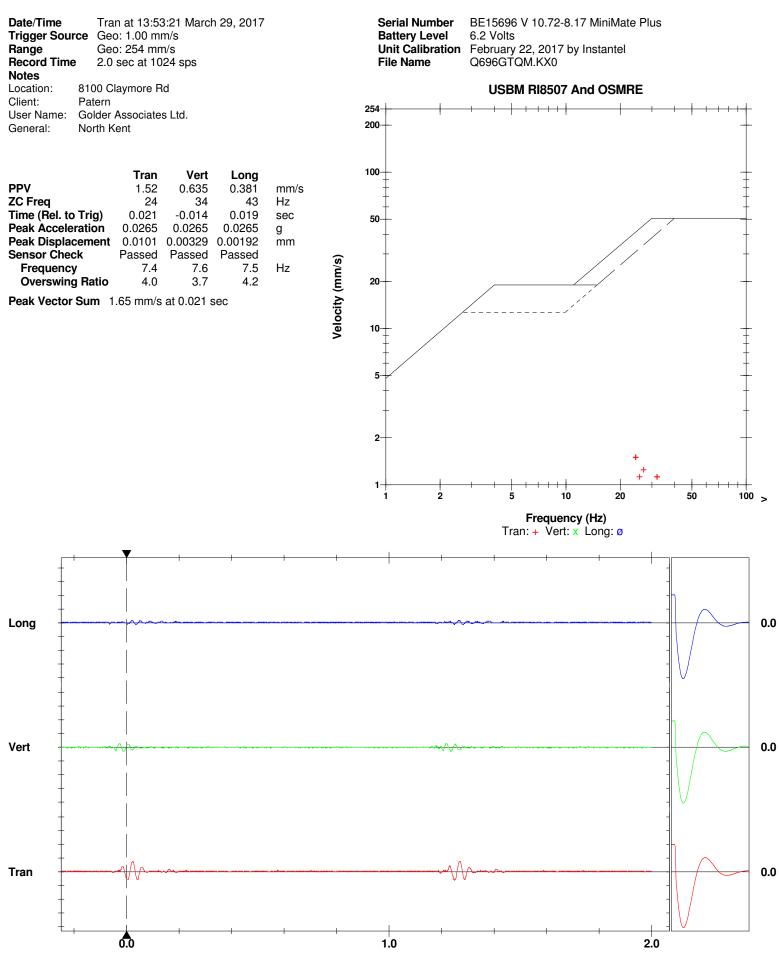
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQM.KU0

Notes







Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger =



 Date/Time
 Tran at 13:53:21 March 29, 2017

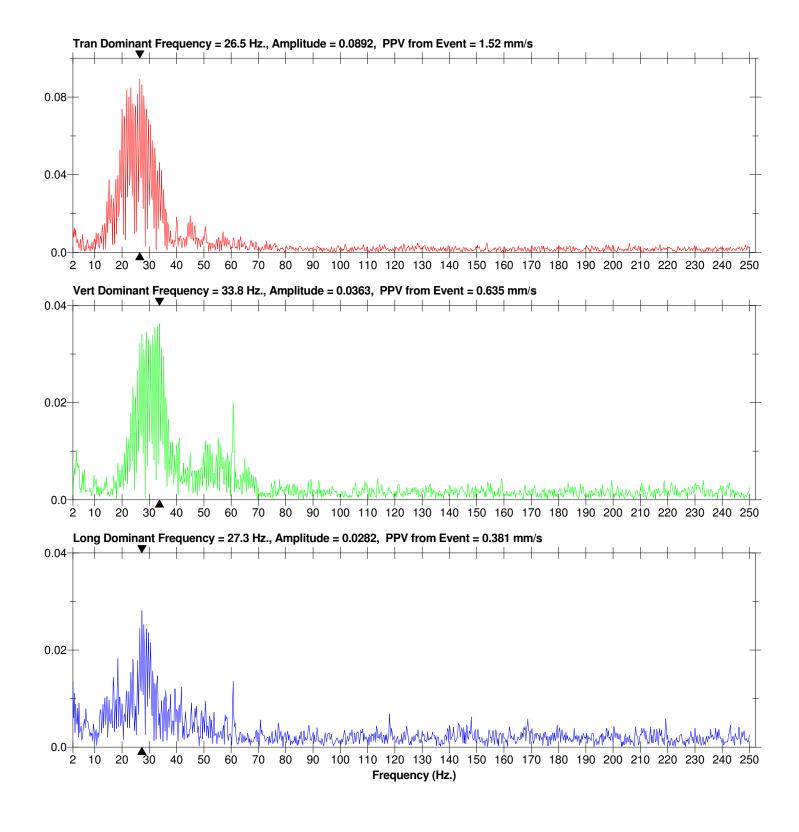
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

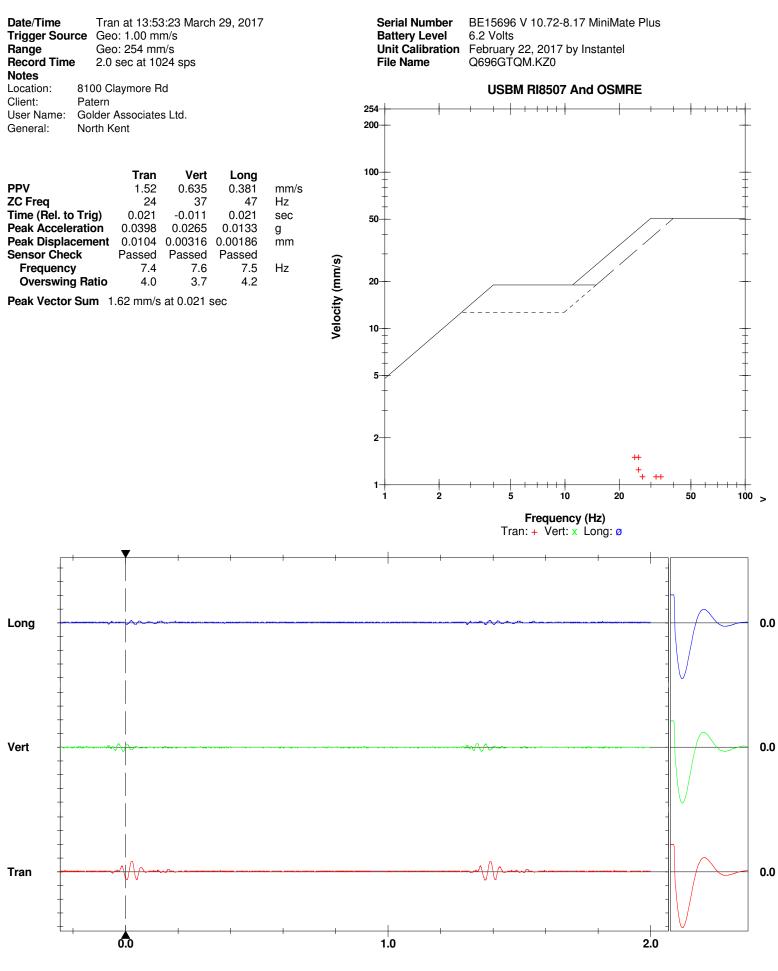
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQM.KX0

Notes







Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger =



 Date/Time
 Tran at 13:53:23 March 29, 2017

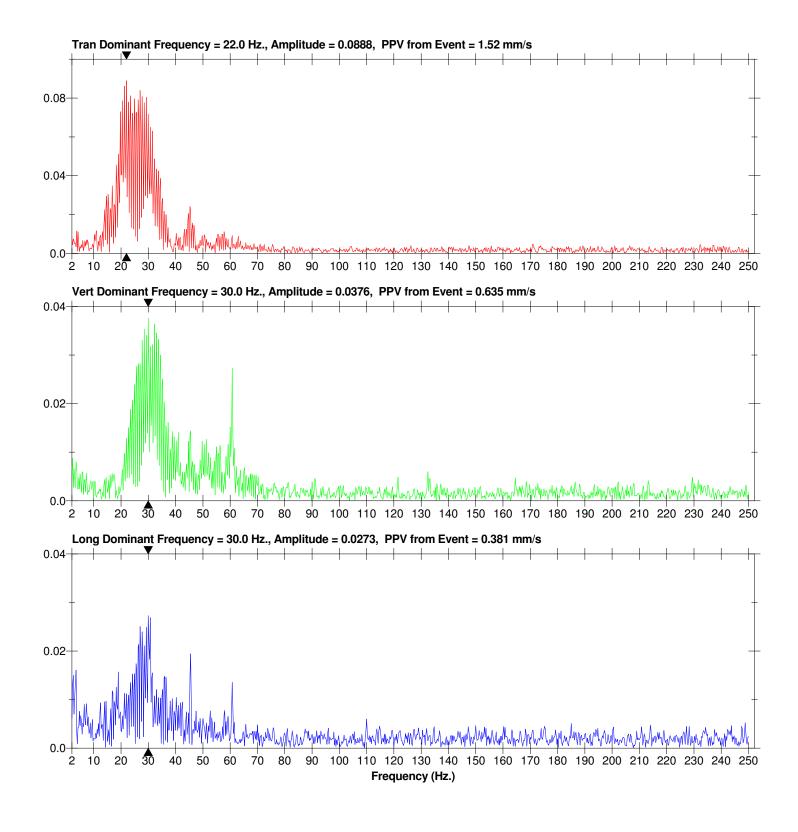
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

 Record Time
 2.0 sec at 1024 sps

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.2 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTQM.KZ0

Notes



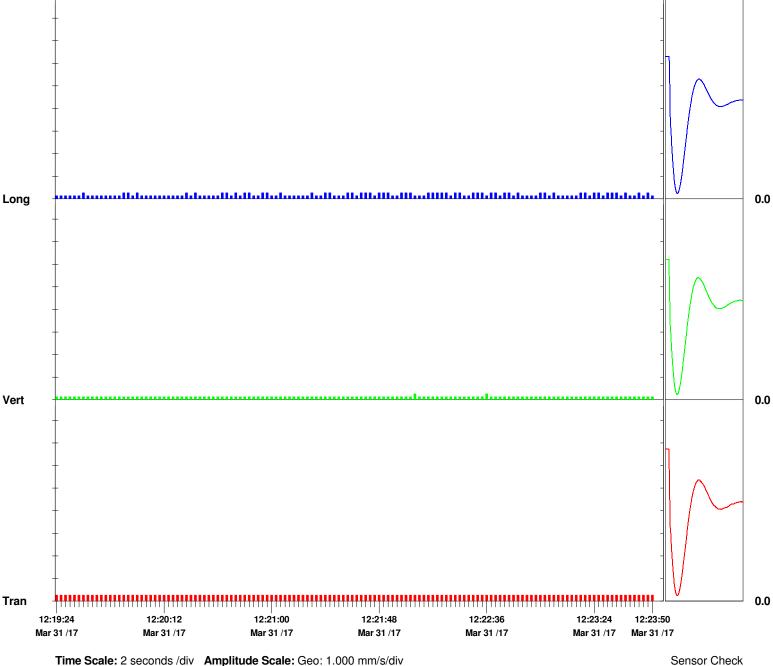


Histogram S Histogram F Number of Ir Range Sample Rate Notes	inish Time ntervals	12:19:22 March 31, 2017 12:23:51 March 31, 2017 134.00 at 2 seconds Geo:254 mm/s 1024sps
Location: Client: User Name: General:	8100 Claym Patern Golder Asso North Kent	

Serial Number BE15696 V 10.72-8.17 MiniMate Plus Battery Level Unit Calibration 6.1 Volts February 22, 2017 by Instantel File Name Q696GTU7.KA0

	Tran	Vert	Long	
PPV	0.254	0.254	0.254	mm/s
ZC Freq	>100	>100	>100	Hz
Date	Mar 31 /17	Mar 31 /17	Mar 31 /17	
Time	12:19:24	12:22:04	12:19:36	
Sensor Check	Passed	Passed	Passed	
Frequency	7.5	7.6	7.5	Hz
Overswing Ratio	3.9	3.8	4.2	

Peak Vector Sum 0.359 mm/s on March 31, 2017 at 12:20:48

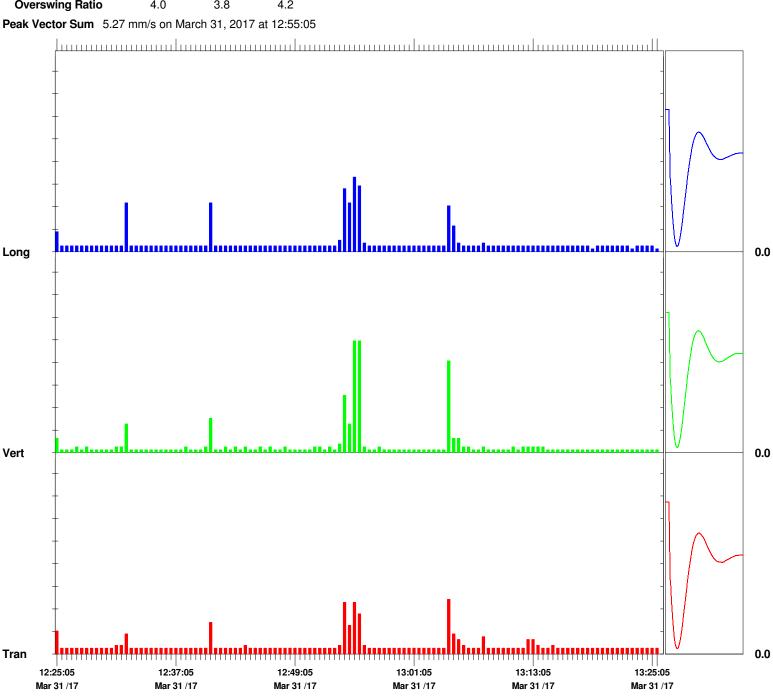




Histogram S Histogram F Number of Ir Range Sample Rate Notes	inish Time ntervals	12:24:35 March 31, 2017 13:25:09 March 31, 2017 1816.00 at 2 seconds Geo:254 mm/s 1024sps
Location: Client: User Name: General:	8100 Claym Patern Golder Asse North Kent	

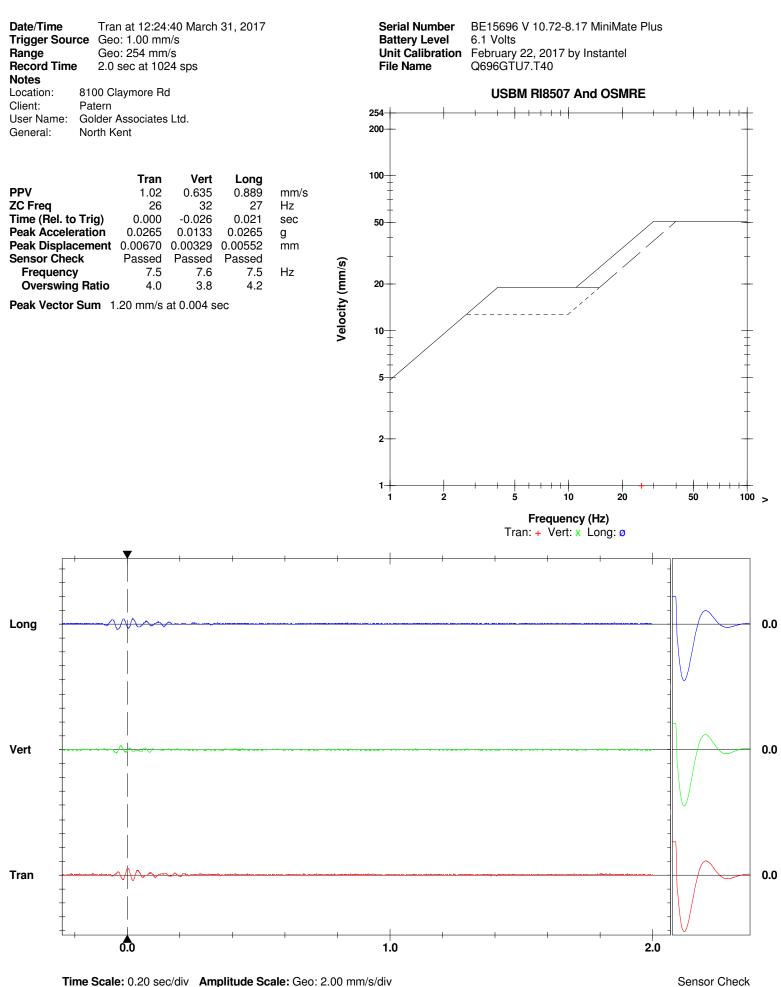
BE15696 V 10.72-8.17 MiniMate Plus **Serial Number** Battery Level Unit Calibration 6.1 Volts February 22, 2017 by Instantel Q696GTU7.SZ0 File Name

	Iran	Vert	Long	
PPV	2.41	4.95	3.30	mm/s
ZC Freq	23	28	27	Hz
Date	Mar 31 /17	Mar 31 /17	Mar 31 /17	
Time	13:04:35	12:55:05	12:55:01	
Sensor Check	Passed	Passed	Passed	
Frequency	7.5	7.6	7.5	Hz
Overswing Ratio	4.0	3.8	4.2	



Time Scale: 30 seconds /div Amplitude Scale: Geo: 1.000 mm/s/div





Trigger = ► _____ Printed: April 24, 2017 (V 10.60 - 10.60)



Notes

Location: Client:

FFT Report

 Date/Time
 Tran at 12:24:40 March 31, 2017

 Trigger Source
 Geo: 1.00 mm/s

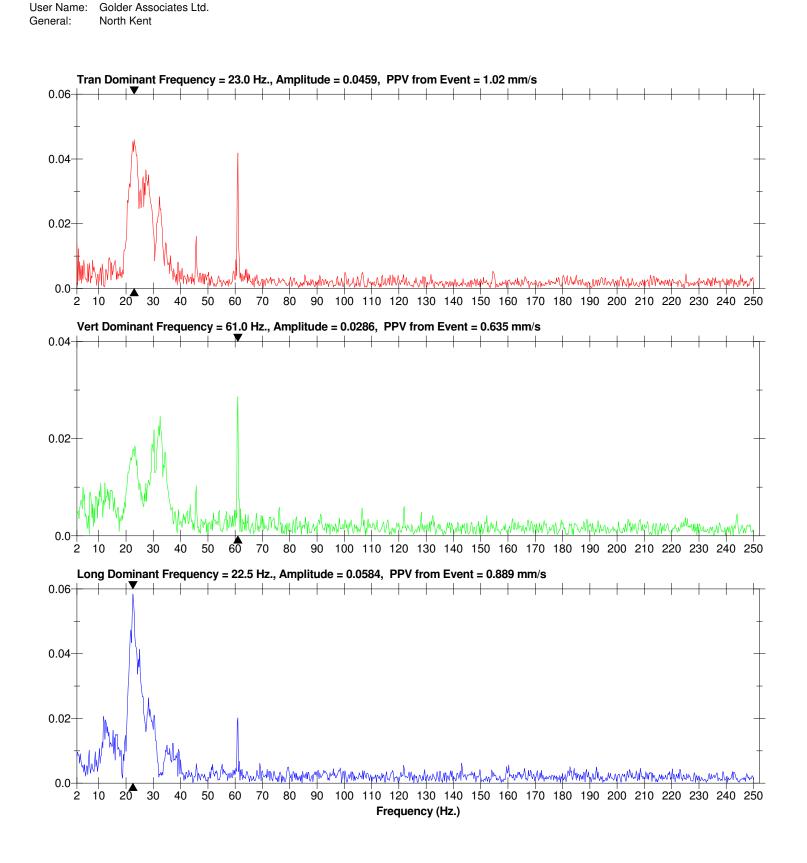
 Range
 Geo: 254 mm/s

 Record Time
 2.0 sec at 1024 sps

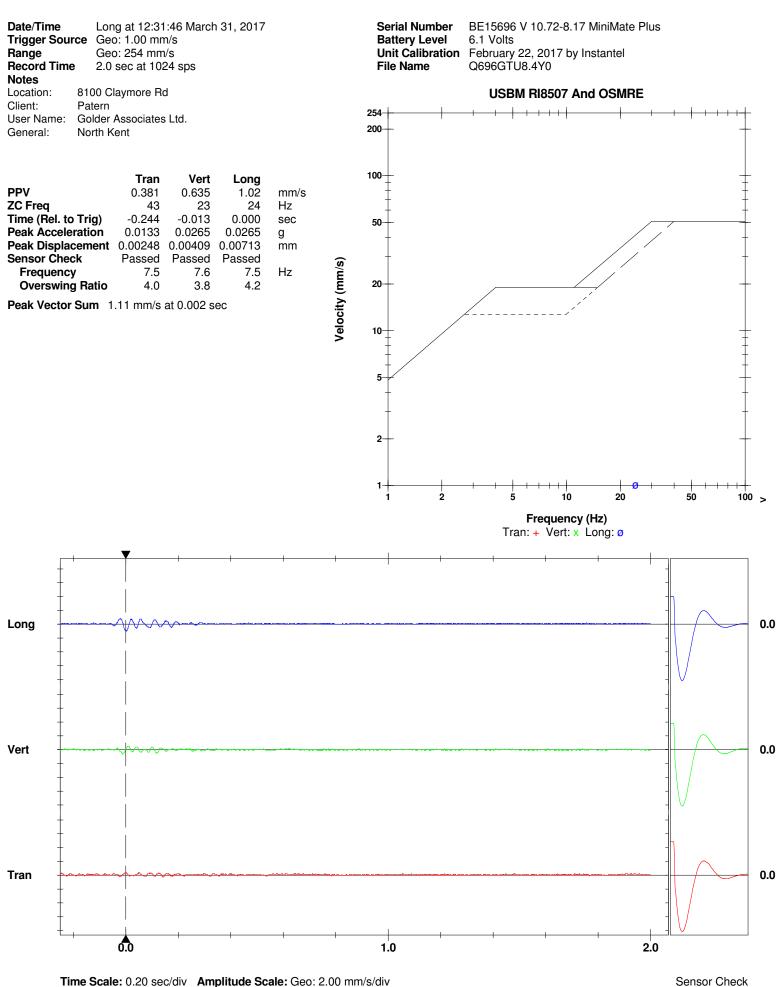
8100 Claymore Rd

Patern

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.1 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTU7.T40









Notes

Location: Client:

User Name:

FFT Report

 Date/Time
 Long at 12:31:46 March 31, 2017

 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

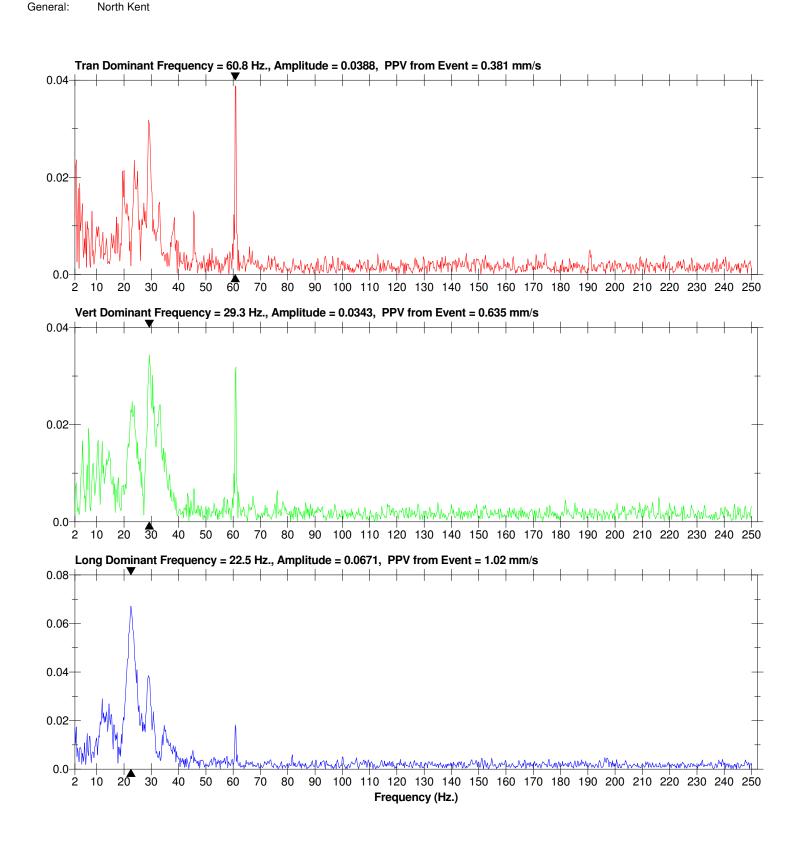
 Record Time
 2.0 sec at 1024 sps

8100 Claymore Rd

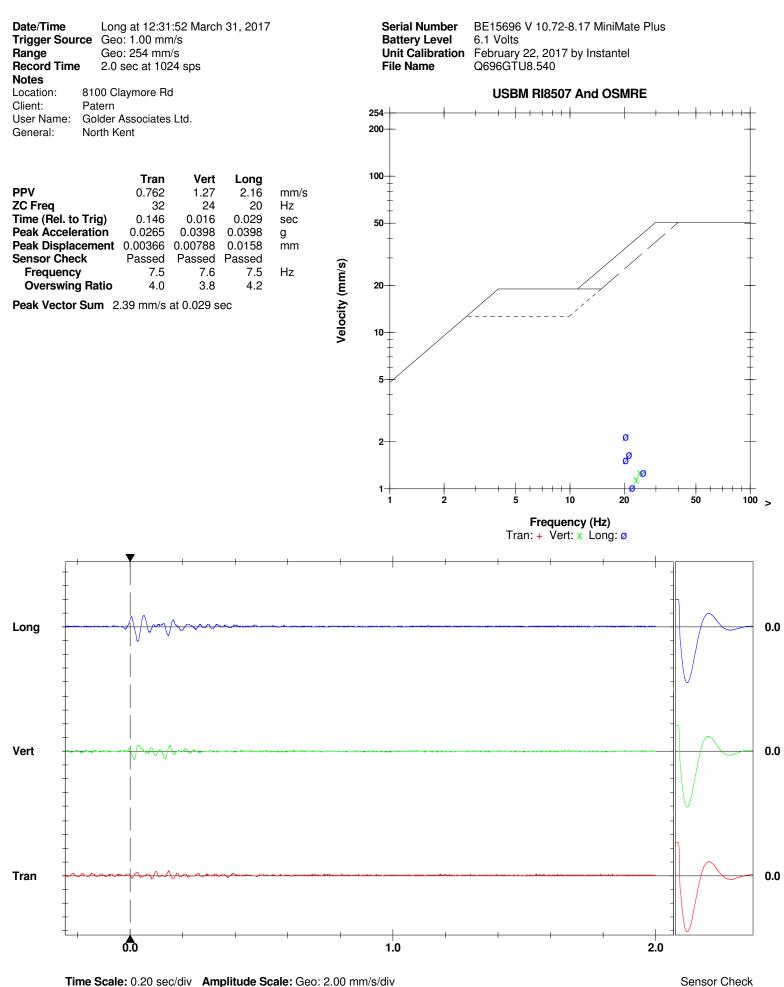
Golder Associates Ltd.

Patern

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.1 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTU8.4Y0









Notes

Location: Client:

General:

User Name:

FFT Report

 Date/Time
 Long at 12:31:52 March 31, 2017

 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

 Record Time
 2.0 sec at 1024 sps

8100 Claymore Rd

Golder Associates Ltd.

Patern

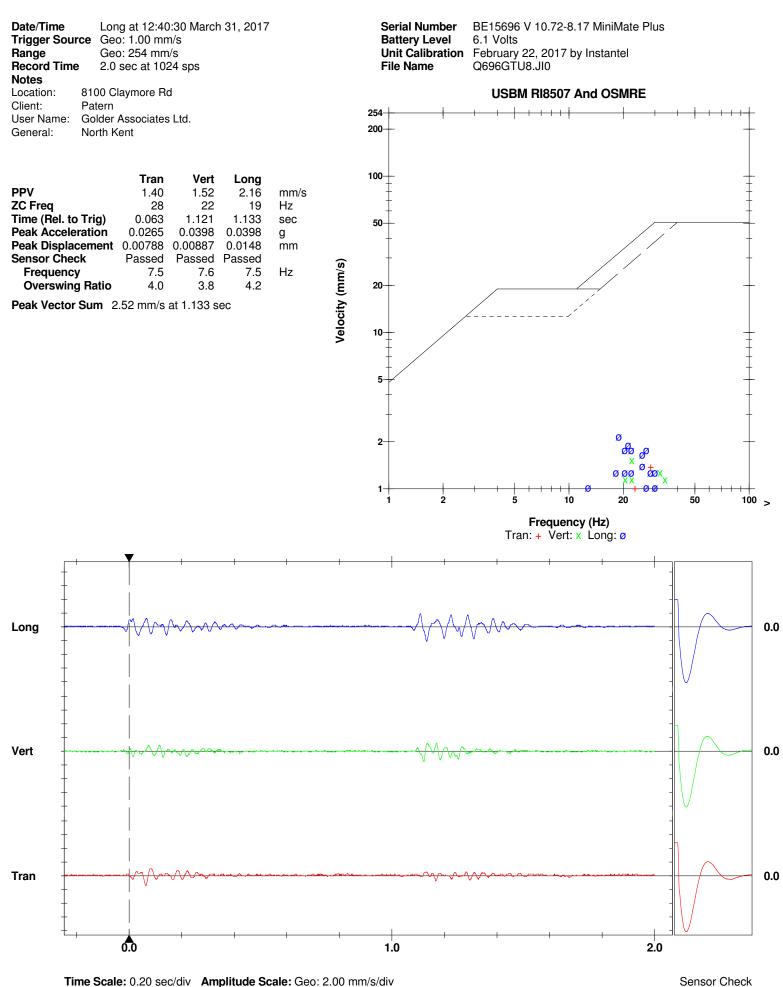
North Kent

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.1 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTU8.540

Tran Dominant Frequency = 27.8 Hz., Amplitude = 0.0455, PPV from Event = 0.762 mm/s V 0.06 0.04 0.02 0.0 80 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 2 10 20 30 40 50 60 70 Vert Dominant Frequency = 25.0 Hz., Amplitude = 0.0463, PPV from Event = 1.27 mm/s 0.06 V 0.04 0.02 When when when when we wanted 0.0 20 30 40 50 60 70 80 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 2 10 Long Dominant Frequency = 17.5 Hz., Amplitude = 0.104, PPV from Event = 2.16 mm/s 0.12+ _ _ _____ 0.08 0.04 www.personal.and.and.montal.montal.montal.and.prover and prover and prove and prov man 0.0 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 10 30 40 50 70 80 2 20 60

Frequency (Hz.)







 Date/Time
 Long at 12:40:30 March 31, 2017

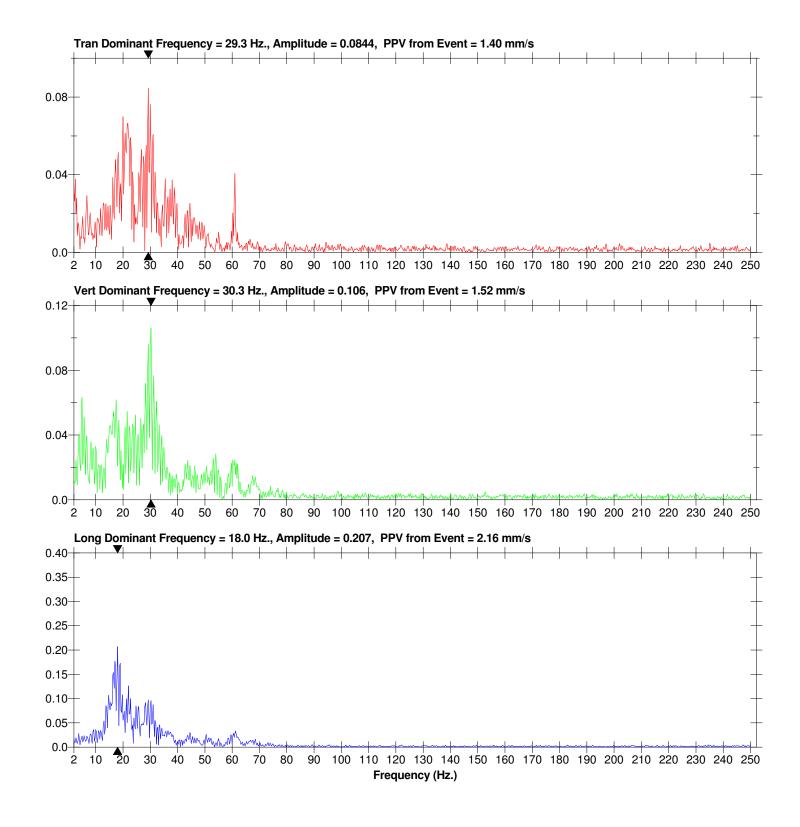
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

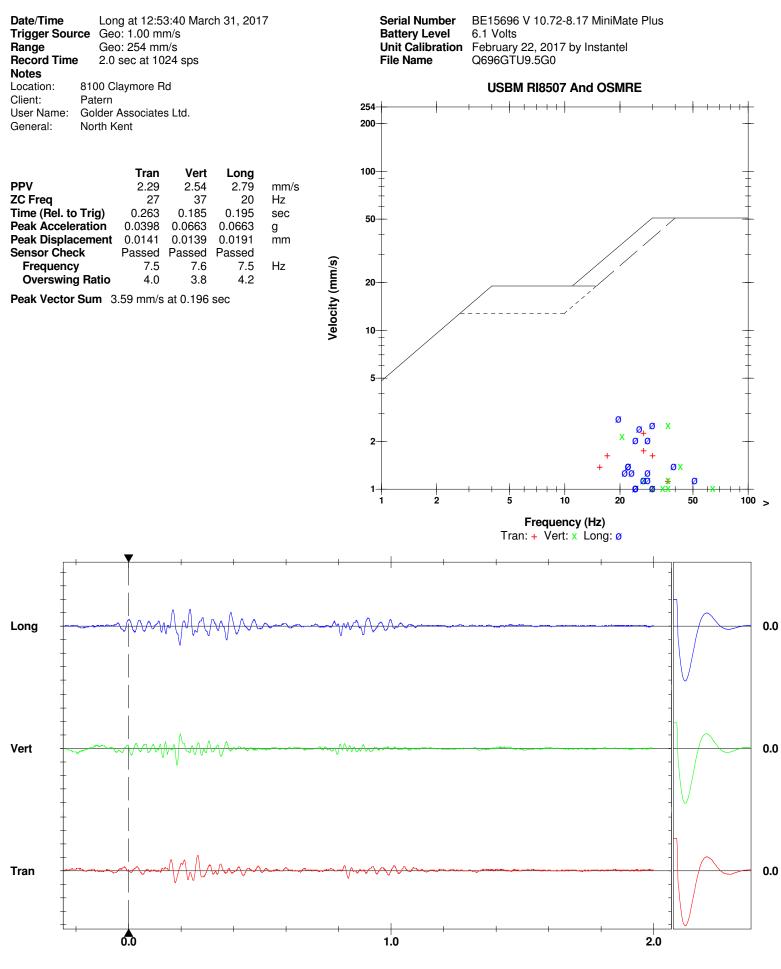
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.1 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTU8.JI0

Notes







Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger =



Notes

Location: Client:

User Name:

FFT Report

 Date/Time
 Long at 12:53:40 March 31, 2017

 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

 Record Time
 2.0 sec at 1024 sps

8100 Claymore Rd

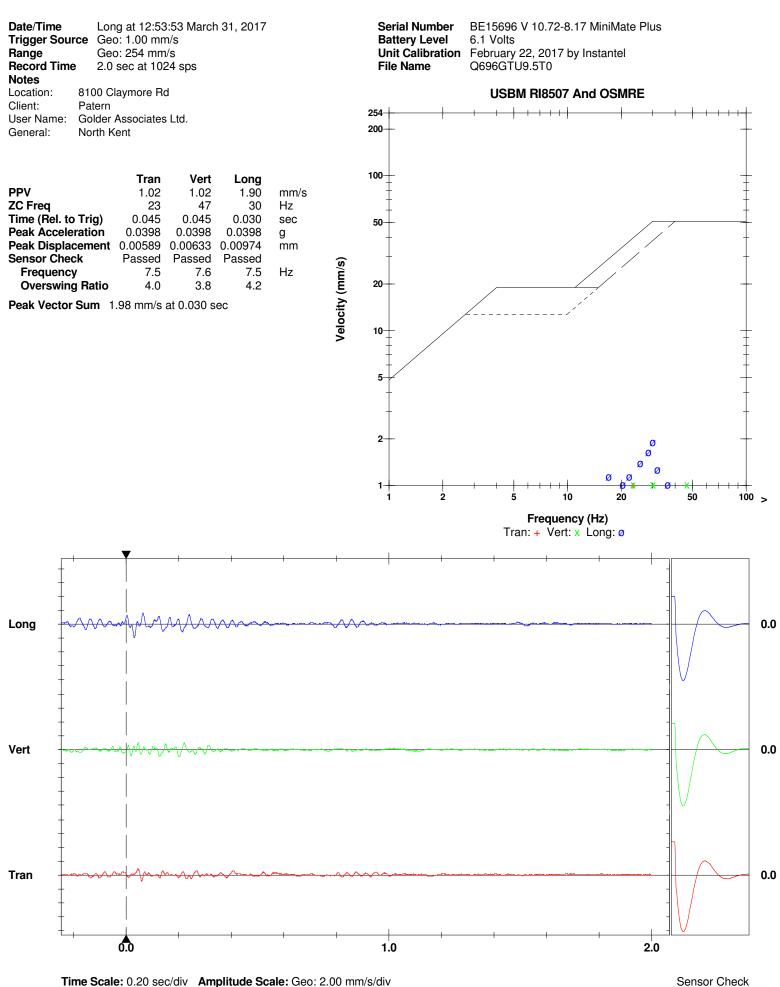
Golder Associates Ltd.

Patern

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.1 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTU9.5G0

General: North Kent Tran Dominant Frequency = 21.3 Hz., Amplitude = 0.115, PPV from Event = 2.29 mm/s 0.12-0.08 0.04 0.0 20 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 2 10 30 40 50 60 70 80 90 Vert Dominant Frequency = 29.5 Hz., Amplitude = 0.108, PPV from Event = 2.54 mm/s 0.12 0.08 0.04 0.0 30 40 50 60 70 80 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 2 10 20 Long Dominant Frequency = 25.3 Hz., Amplitude = 0.212, PPV from Event = 2.79 mm/s ____▼__ 0.40+ _ _ ____ _ 0.35 0.30-0.25 0.20-0.15 0.10 0.05 0.0 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 40 50 60 70 80 90 2 10 20 30 Frequency (Hz.)





Trigger = ► _____ Printed: April 24, 2017 (V 10.60 - 10.60)

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Notes

Client: User Name:

Location:

General:

FFT Report

 Date/Time
 Long at 12:53:53 March 31, 2017

 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

 Record Time
 2.0 sec at 1024 sps

8100 Claymore Rd

Golder Associates Ltd.

Patern

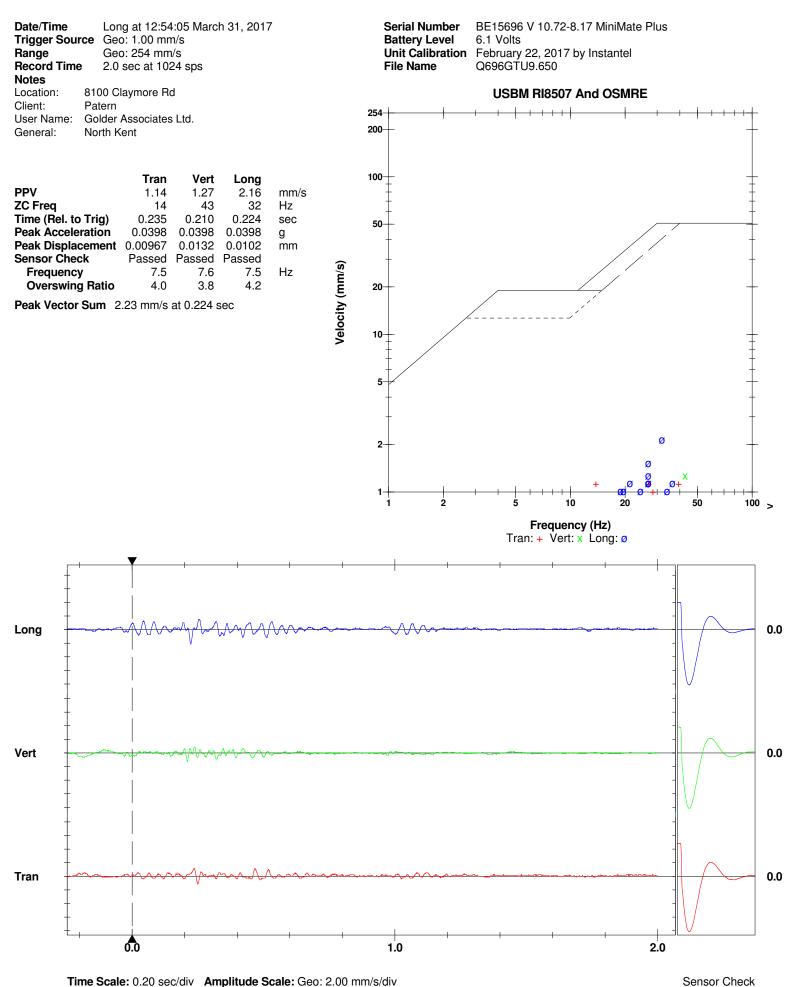
North Kent

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.1 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTU9.5T0

Tran Dominant Frequency = 26.3 Hz., Amplitude = 0.0564, PPV from Event = 1.02 mm/s 0.06 0.04 0.02 0.0 80 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 2 10 20 30 40 50 60 70 Vert Dominant Frequency = 29.8 Hz., Amplitude = 0.0729, PPV from Event = 1.02 mm/s 0.08-0.06 0.04 0.02 hand marker and the second second WMM 0.0 30 40 50 60 70 80 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 2 10 20 Long Dominant Frequency = 26.0 Hz., Amplitude = 0.125, PPV from Event = 1.90 mm/s + + + +____ _ 0.12 0.08 0.04 tamilational and the second 0.0 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 20 30 40 50 70 80 2 10 60









 Date/Time
 Long at 12:54:05 March 31, 2017

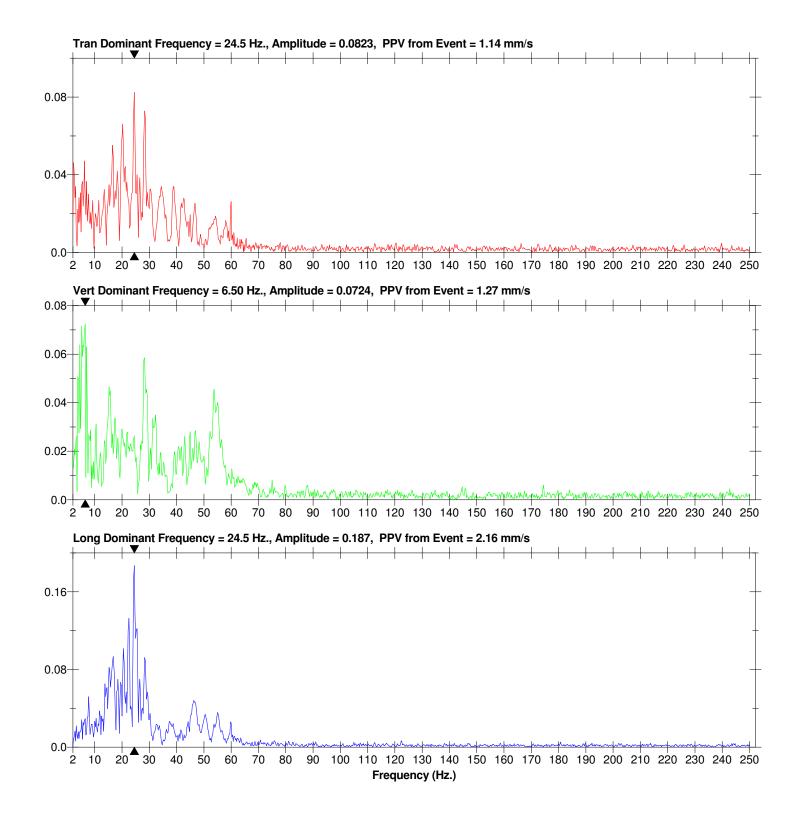
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

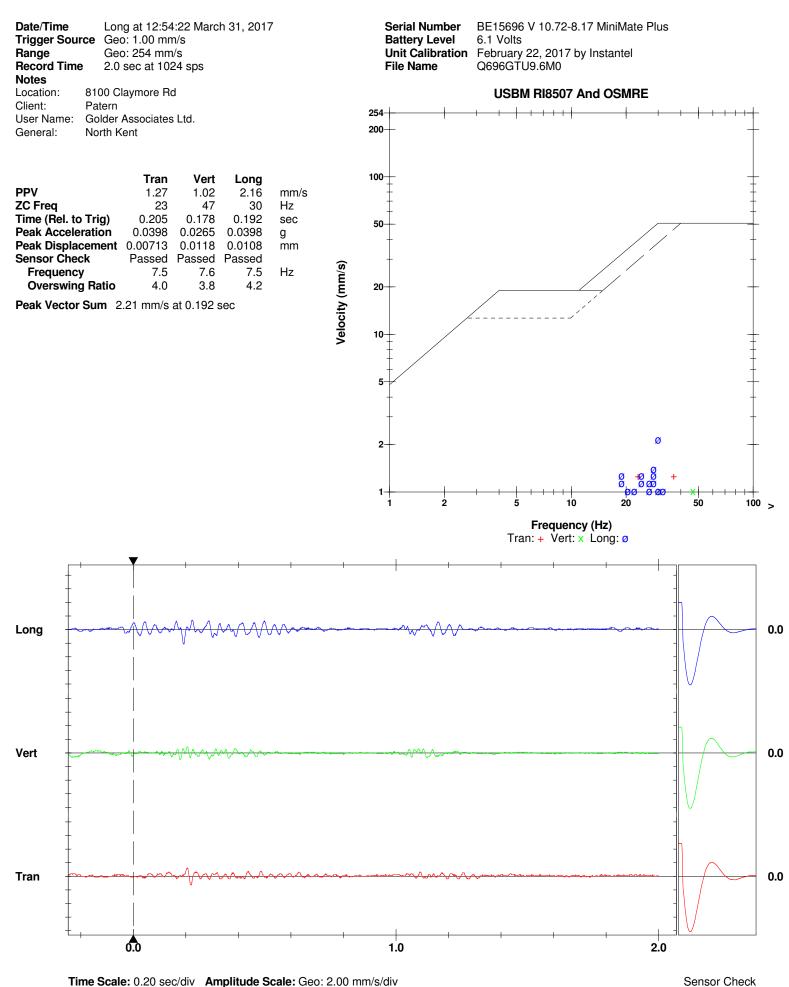
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.1 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTU9.650

Notes









 Date/Time
 Long at 12:54:22 March 31, 2017

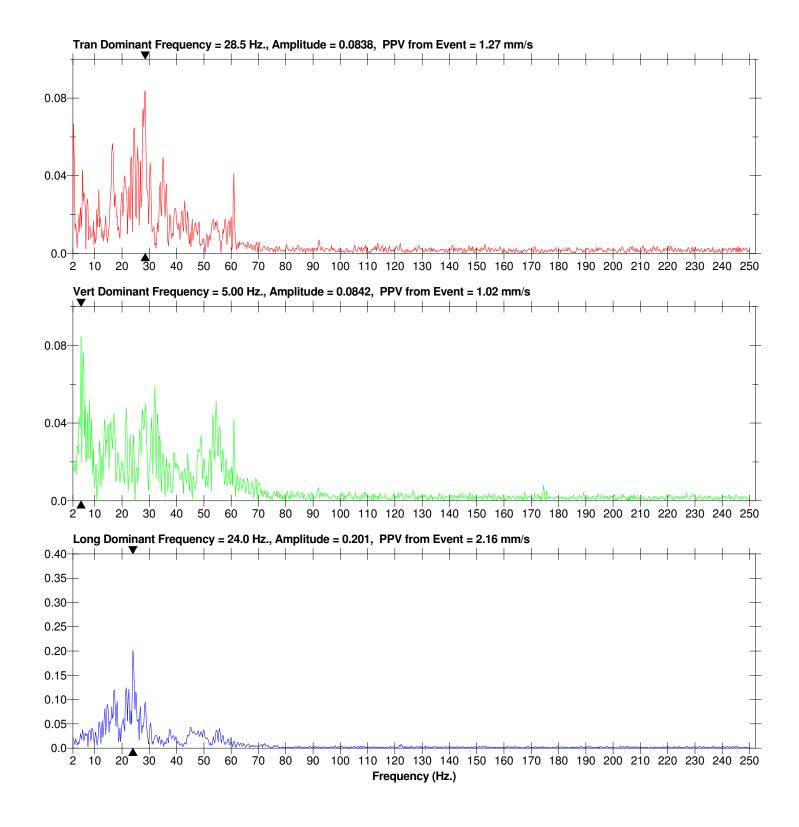
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

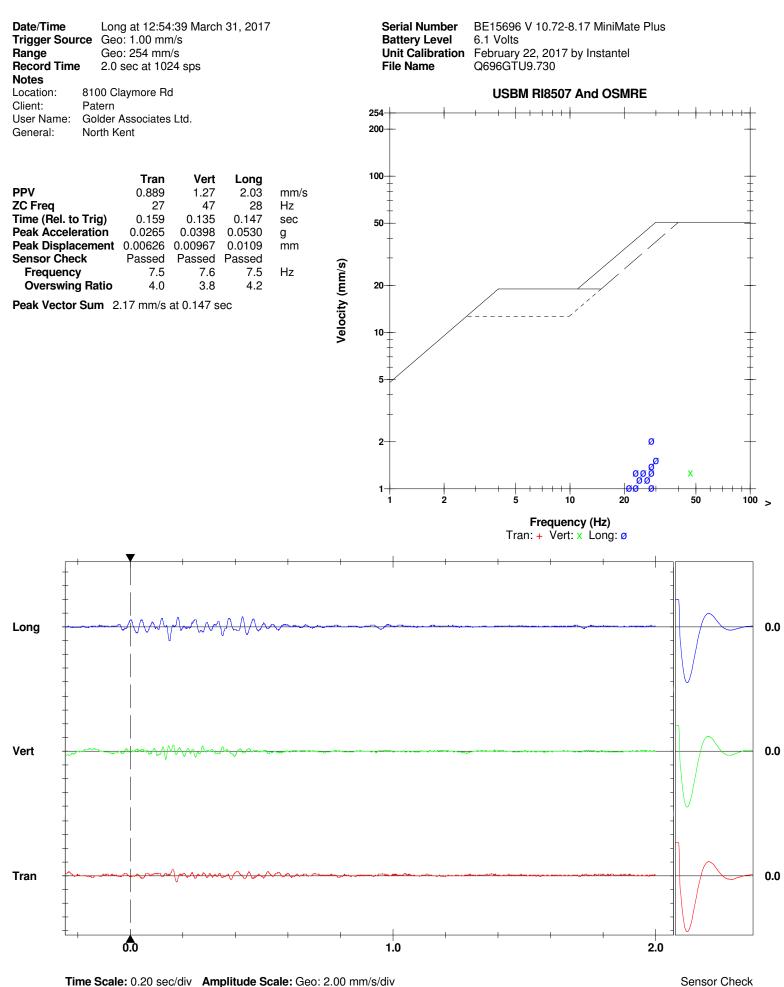
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.1 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTU9.6M0

Notes









 Date/Time
 Long at 12:54:39 March 31, 2017

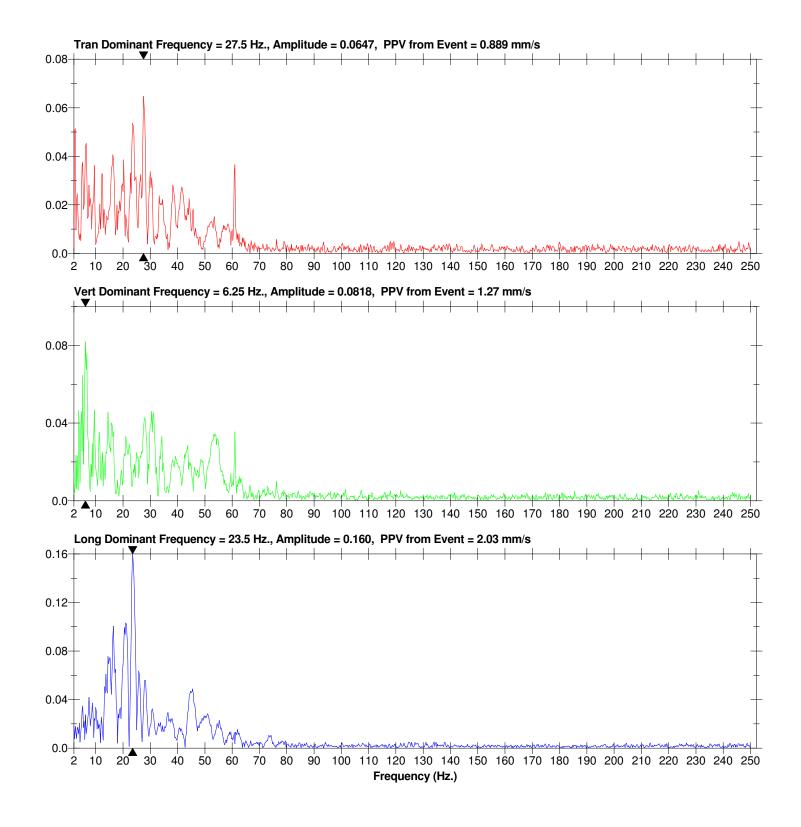
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

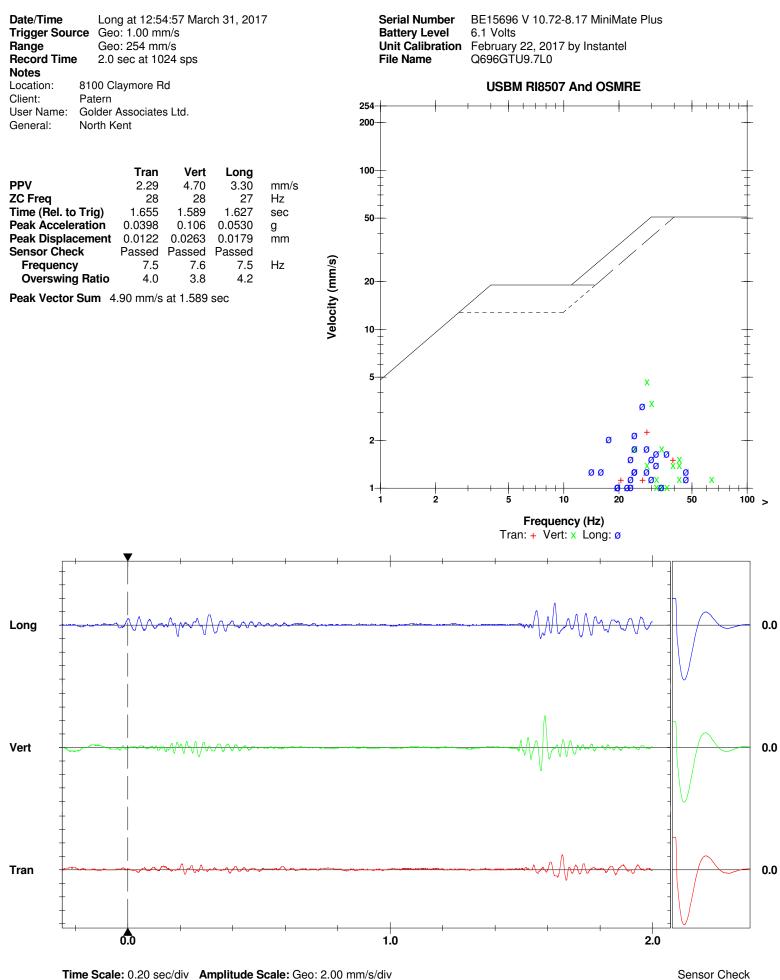
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.1 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTU9.730









Printed: April 24, 2017 (V 10.60 - 10.60)

Trigger = >

Format © 1995-2013 Xmark Corporation

Sensor Check



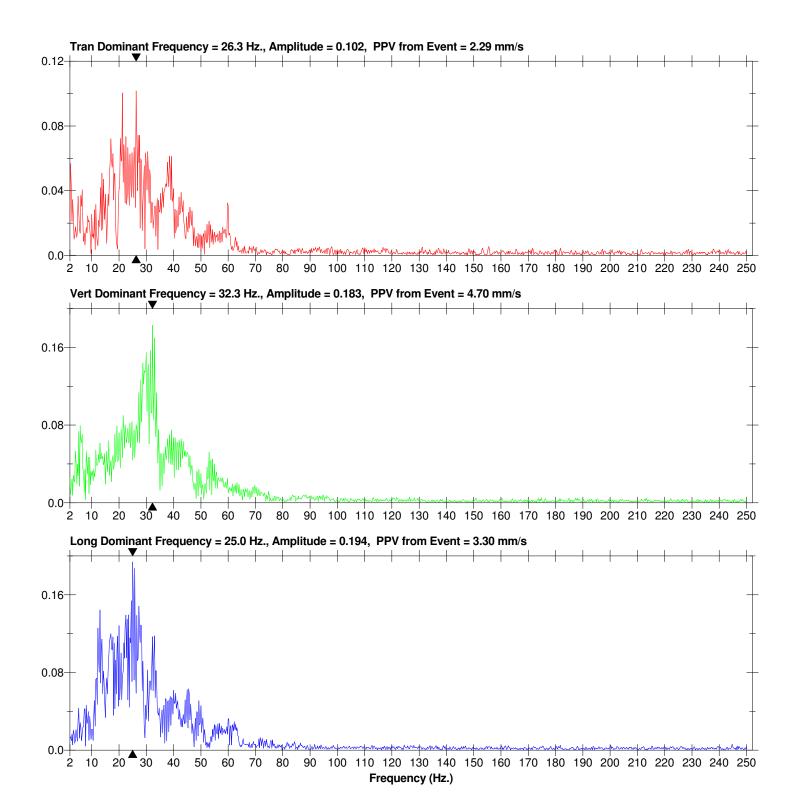
 Date/Time
 Long at 12:54:57 March 31, 2017

 Trigger Source
 Geo: 1.00 mm/s

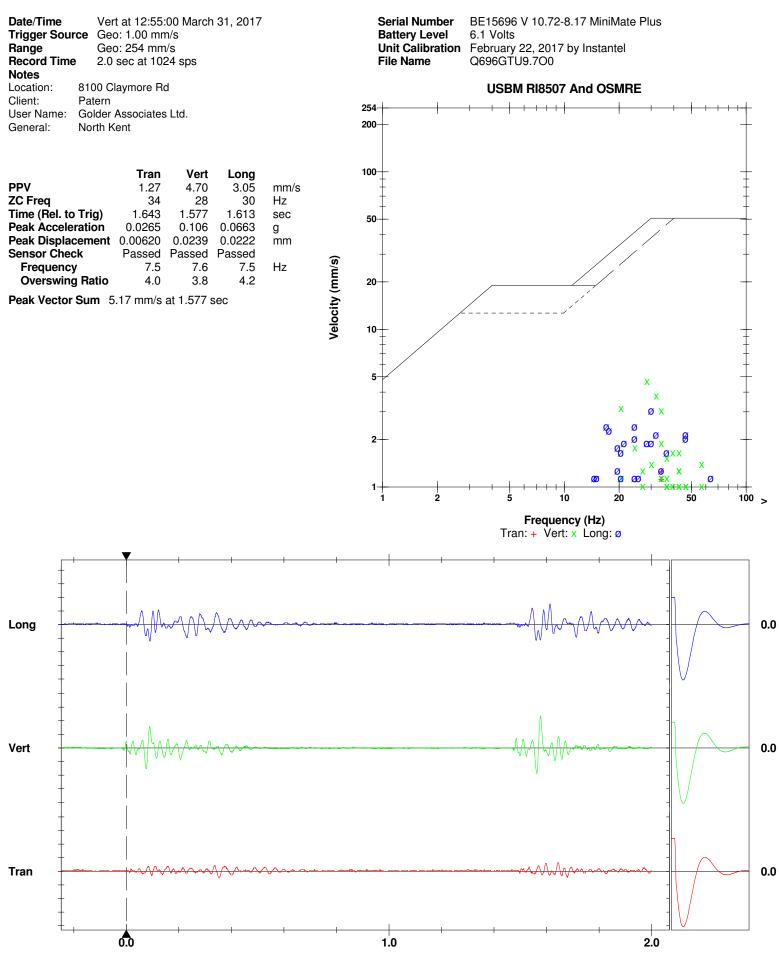
 Range
 Geo: 254 mm/s

 Record Time
 2.0 sec at 1024 sps

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.1 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTU9.7L0







Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger =



 Date/Time
 Vert at 12:55:00 March 31, 2017

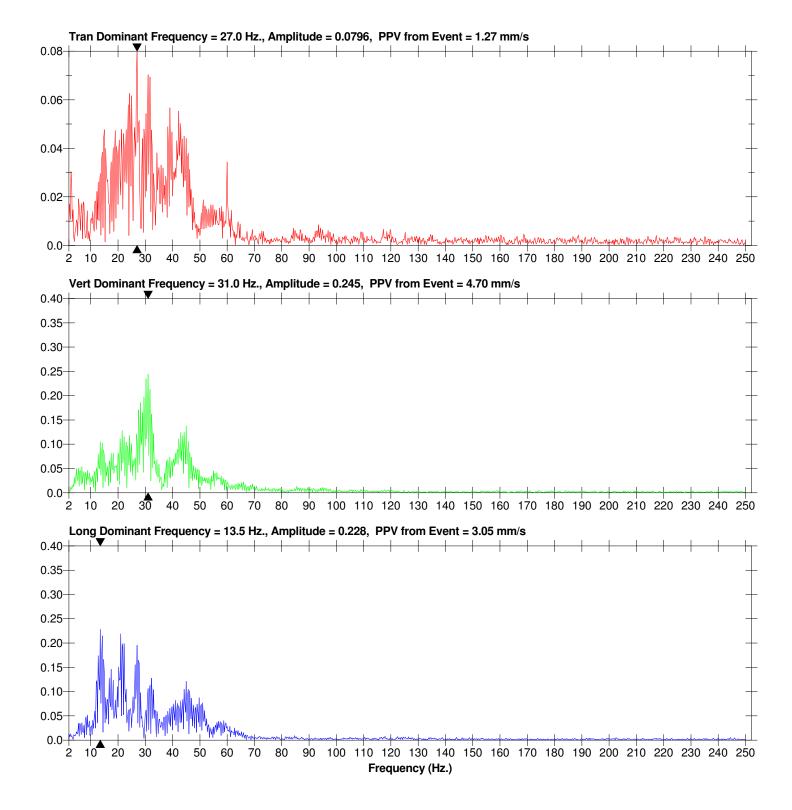
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

 Record Time
 2.0 sec at 1024 sps

Serial NumberBE15696 V 10.72-8.17 MiniMate PlusBattery Level6.1 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameQ696GTU9.700







Serial Number

Battery Level Unit Calibration

File Name

6.2 Volts

Q696GVJO.ZN0

BE15696 V 10.72-8.17 MiniMate Plus

February 22, 2017 by Instantel

Histogram Start Time	17
Histogram Finish Time	19
Number of Intervals	33
Sample Rate	20

17:09:23 May 3, 2017 19:00:31 May 3, 2017 3334.00 at 2 seconds 2048sps

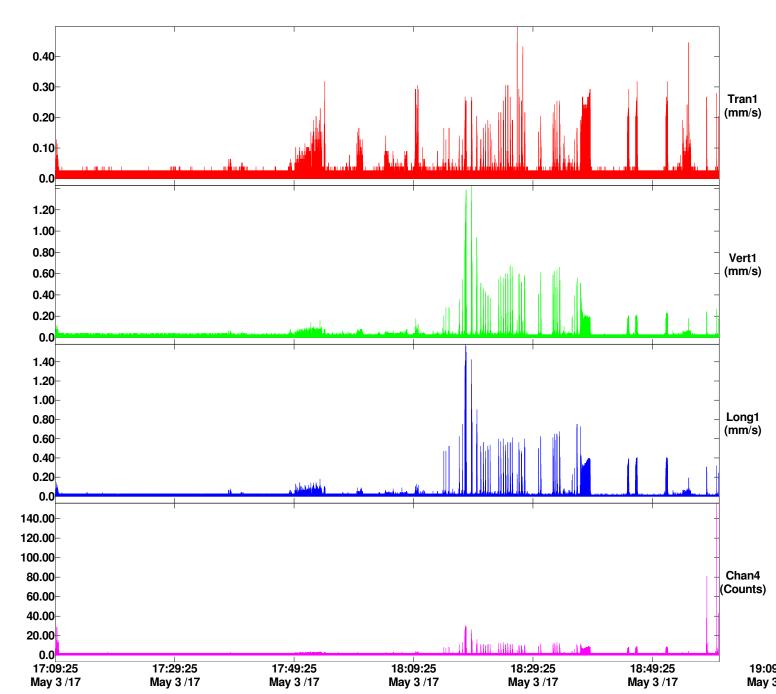
Notes

Location:	Т5
Client:	Pattern
User Name:	Golder Associates Ltd.
General:	50m, 1668031

Extended Notes

Geophone Anchored into subgrade and secured with sand bag

Channel	Name	Peak	Time (sec)	Gain	Range	Units
1	Tran1	0.495	1.133	1x	25.40	mm/s
2	Vert1	1.422	1.021	1x	25.40	mm/s
3	Long1	1.575	1.006	1x	25.40	mm/s
4	Chan4	155.0	1.622	1x	2000.0	Counts



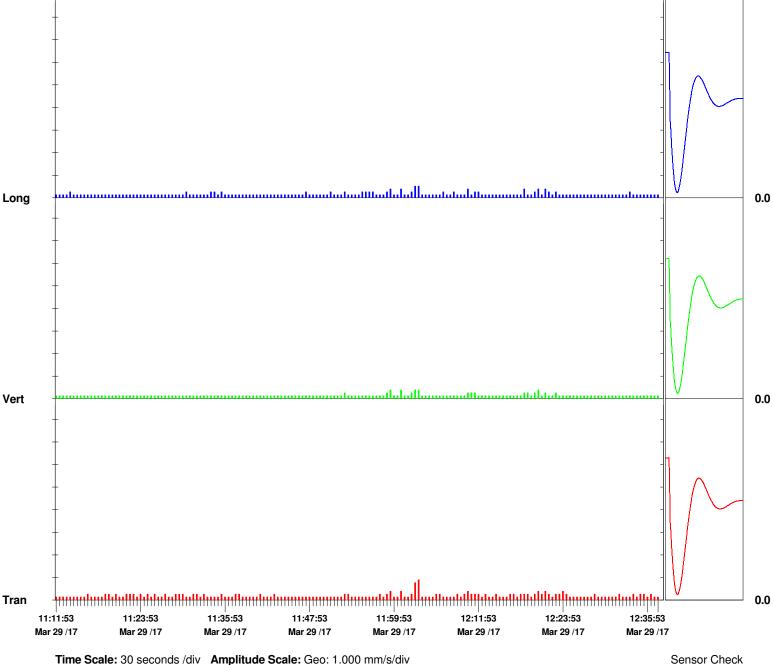


Histogram S Histogram F Number of Ir Range Sample Rate Notes	inish Time Itervals	11:11:23 March 29, 2017 12:37:21 March 29, 2017 2578.00 at 2 seconds Geo:254 mm/s 1024sps
Location: Client: User Name: General:	8100 Claym Patern Golder Asse North Kent	

Serial Number BE18695 V 10.72-8.17 MiniMate Plus **Battery Level** 6.3 Volts Unit Calibration February 22, 2017 by Instantel File Name T695GTQF.2Z0

	Tran	Vert	Long	
PPV	0.889	0.381	0.508	mm/s
ZC Freq	34	51	57	Hz
Date	Mar 29 /17	Mar 29 /17	Mar 29 /17	
Time	12:02:55	11:59:05	12:02:47	
Sensor Check	Passed	Passed	Passed	
Frequency	7.4	7.4	7.7	Hz
Overswing Ratio	3.8	3.7	3.8	

Peak Vector Sum 0.907 mm/s on March 29, 2017 at 12:02:55



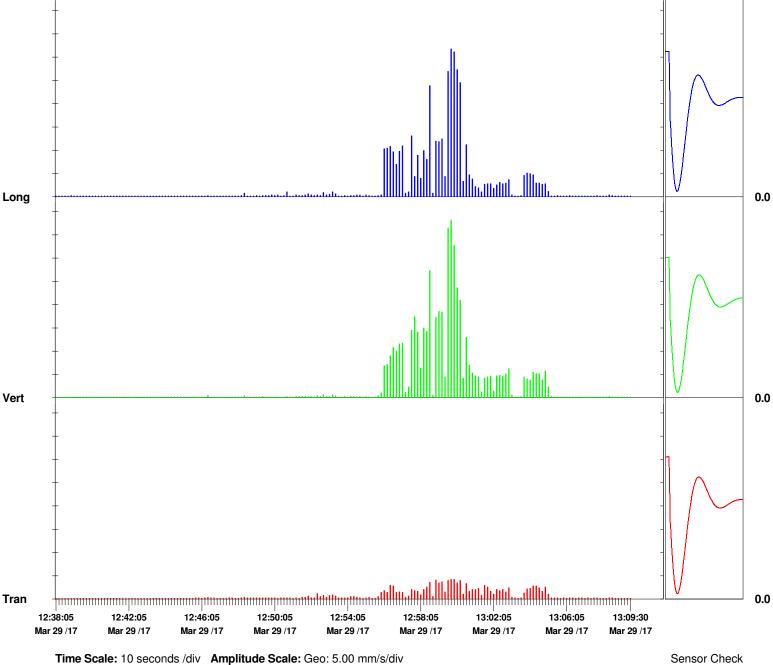


Histogram S Histogram F Number of Ir Range Sample Rate Notes	inish Time ntervals	12:37:55 March 29, 2017 13:09:30 March 29, 2017 947.00 at 2 seconds Geo:254 mm/s 1024sps
Location: Client: User Name: General:	8100 Clayn Patern Golder Ass North Kent	

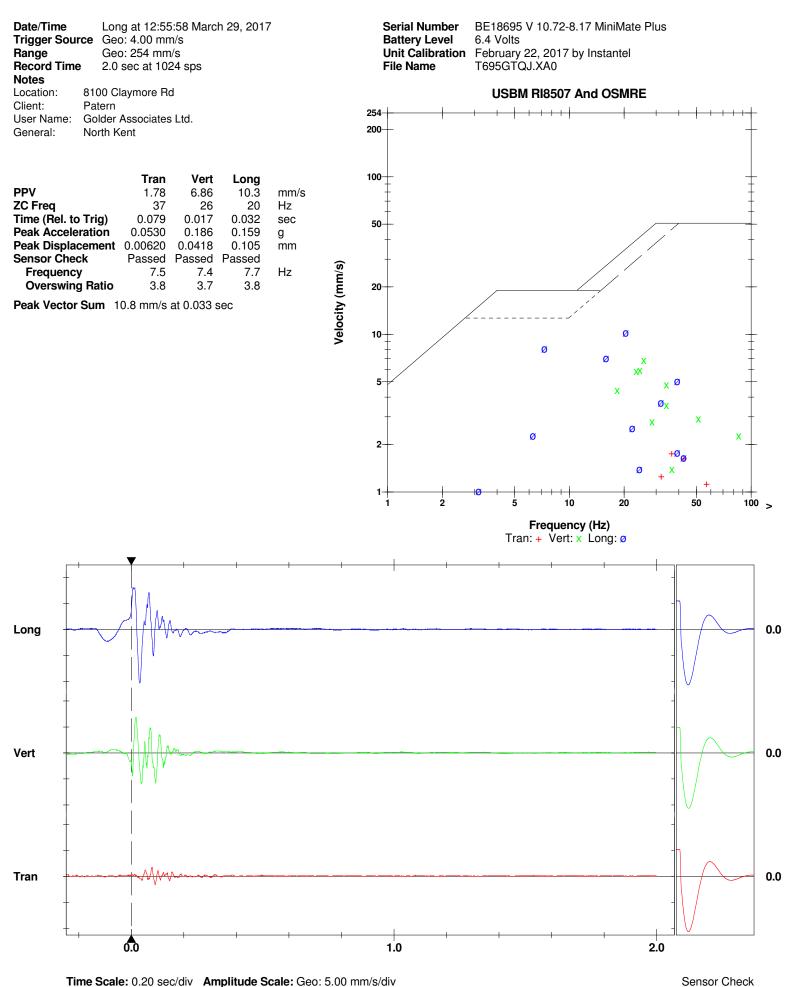
Serial Number BE18695 V 10.72-8.17 MiniMate Plus **Battery Level** 6.3 Volts Unit Calibration February 22, 2017 by Instantel File Name T695GTQJ.370

	Tran	Vert	Long	
PPV	4.19	38.1	31.7	mm/s
ZC Freq	>100	32	24	Hz
Date	Mar 29 /17	Mar 29 /17	Mar 29 /17	
Time	12:59:43	12:59:39	12:59:45	
Sensor Check	Passed	Passed	Passed	
Frequency	7.5	7.4	7.7	Hz
Overswing Ratio	3.8	3.7	3.8	

Peak Vector Sum 40.8 mm/s on March 29, 2017 at 12:59:39







Printed: April 24, 2017 (V 10.60 - 10.60)

Trigger = 🕨



Notes

Location: Client:

General:

User Name:

FFT Report

Long at 12:55:58 March 29, 2017 Date/Time Trigger Source Geo: 4.00 mm/s Range Geo: 254 mm/s **Record Time** 2.0 sec at 1024 sps

8100 Claymore Rd

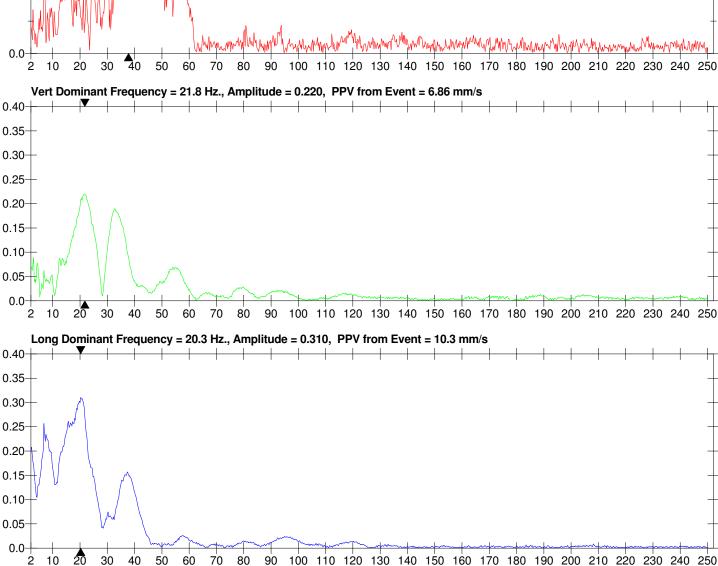
Golder Associates Ltd.

Patern

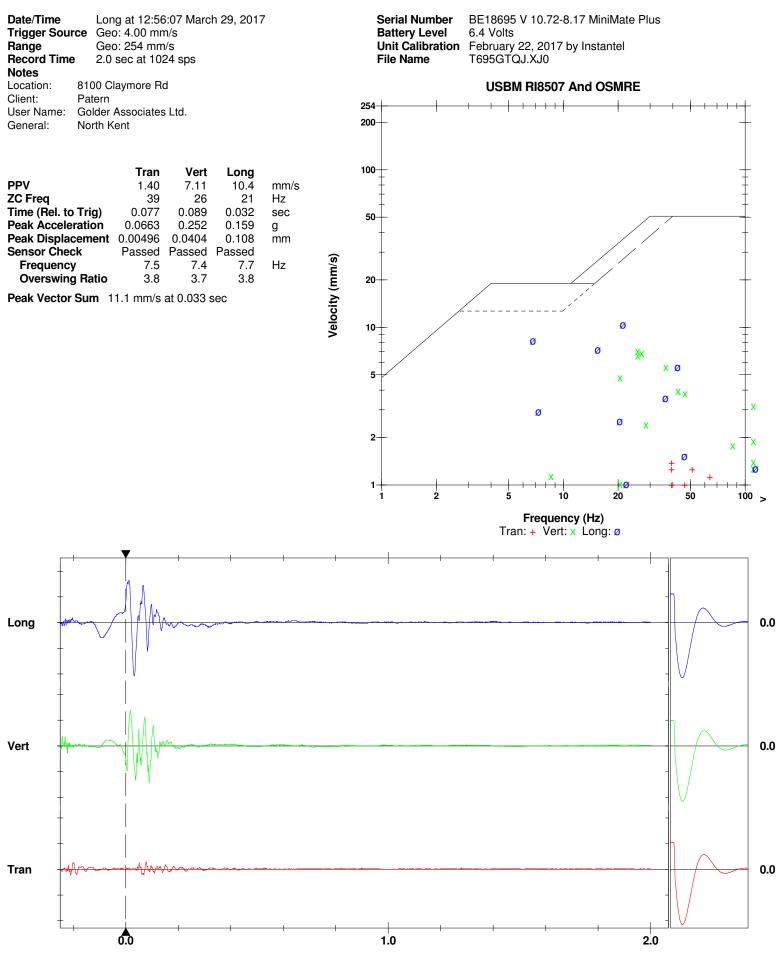
North Kent

Serial Number BE18695 V 10.72-8.17 MiniMate Plus **Battery Level** 6.4 Volts Unit Calibration February 22, 2017 by Instantel File Name T695GTQJ.XA0

Tran Dominant Frequency = 37.8 Hz., Amplitude = 0.0474, PPV from Event = 1.78 mm/s 0.06 0.04 0.02 0.0 100 2 10 20 30 40 50 60 70 80 90 Vert Dominant Frequency = 21.8 Hz., Amplitude = 0.220, PPV from Event = 6.86 mm/s 0.40 +0.35 0.30-0.25-0.20-0.15 0.10-0.05 0.0 20 10 30 40 50 60 70 80 2 Long Dominant Frequency = 20.3 Hz., Amplitude = 0.310, PPV from Event = 10.3 mm/s 0.40+ Y +____ -_ 0.35 0.30-0.25







Trigger = ► _____ Printed: April 24, 2017 (V 10.60 - 10.60)

Time Scale: 0.20 sec/div Amplitude Scale: Geo: 5.00 mm/s/div

Format © 1995-2013 Xmark Corporation

Sensor Check



Notes

Location: Client:

User Name:

FFT Report

 Date/Time
 Long at 12:56:07 March 29, 2017

 Trigger Source
 Geo: 4.00 mm/s

 Range
 Geo: 254 mm/s

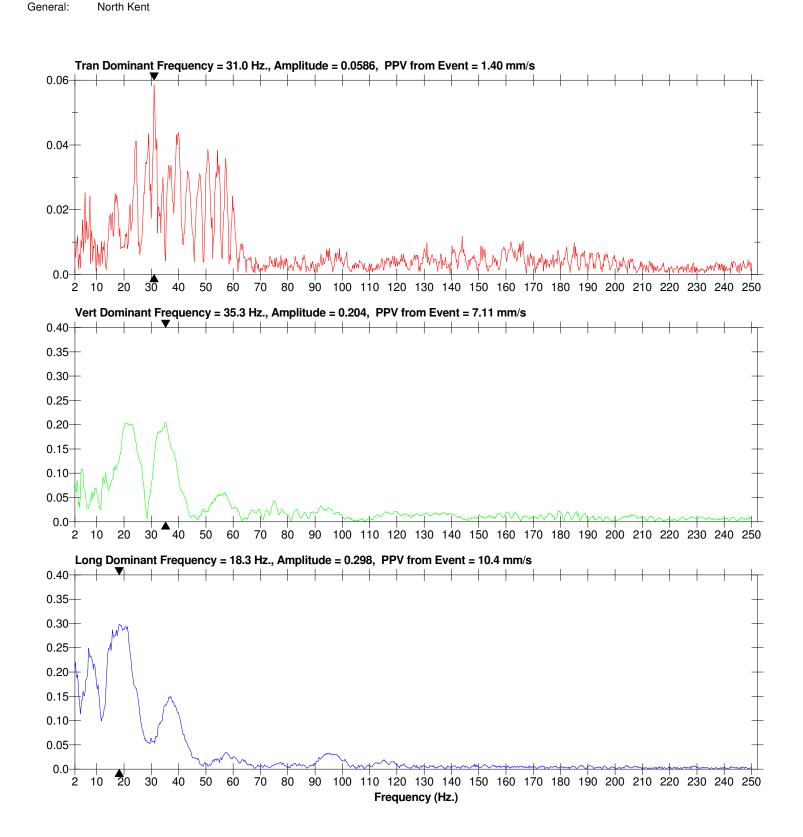
 Record Time
 2.0 sec at 1024 sps

8100 Claymore Rd

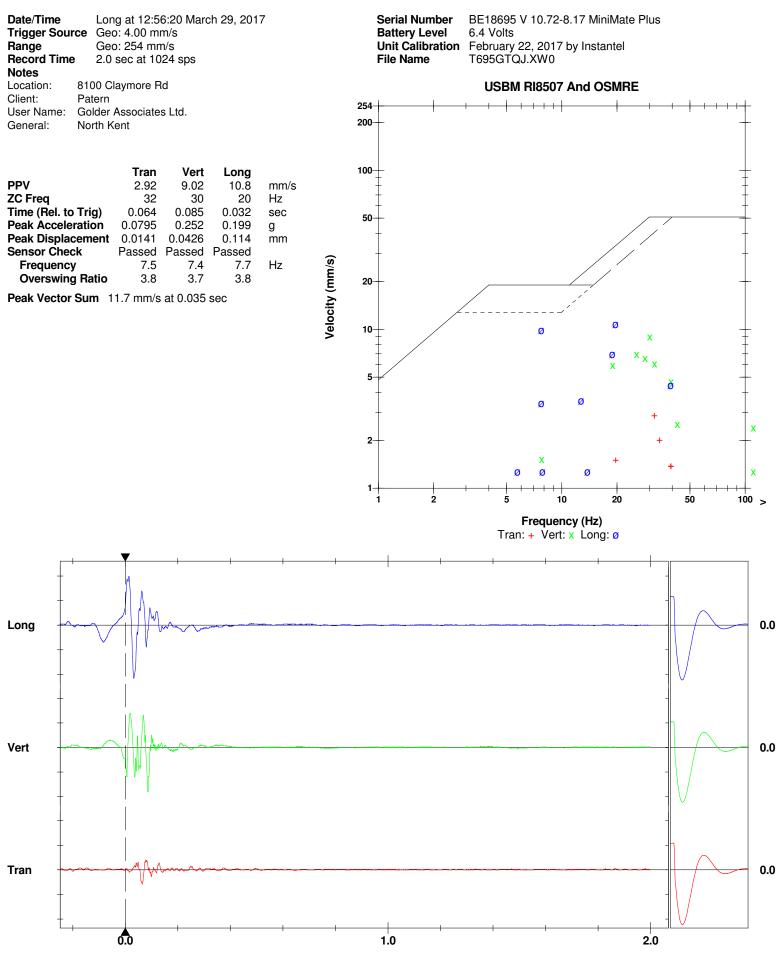
Golder Associates Ltd.

Patern

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQJ.XJ0







Time Scale: 0.20 sec/div Amplitude Scale: Geo: 5.00 mm/s/div Trigger = ► ____ ←



Notes

Location: Client:

General:

User Name:

FFT Report

 Date/Time
 Long at 12:56:20 March 29, 2017

 Trigger Source
 Geo: 4.00 mm/s

 Range
 Geo: 254 mm/s

 Record Time
 2.0 sec at 1024 sps

8100 Claymore Rd

Golder Associates Ltd.

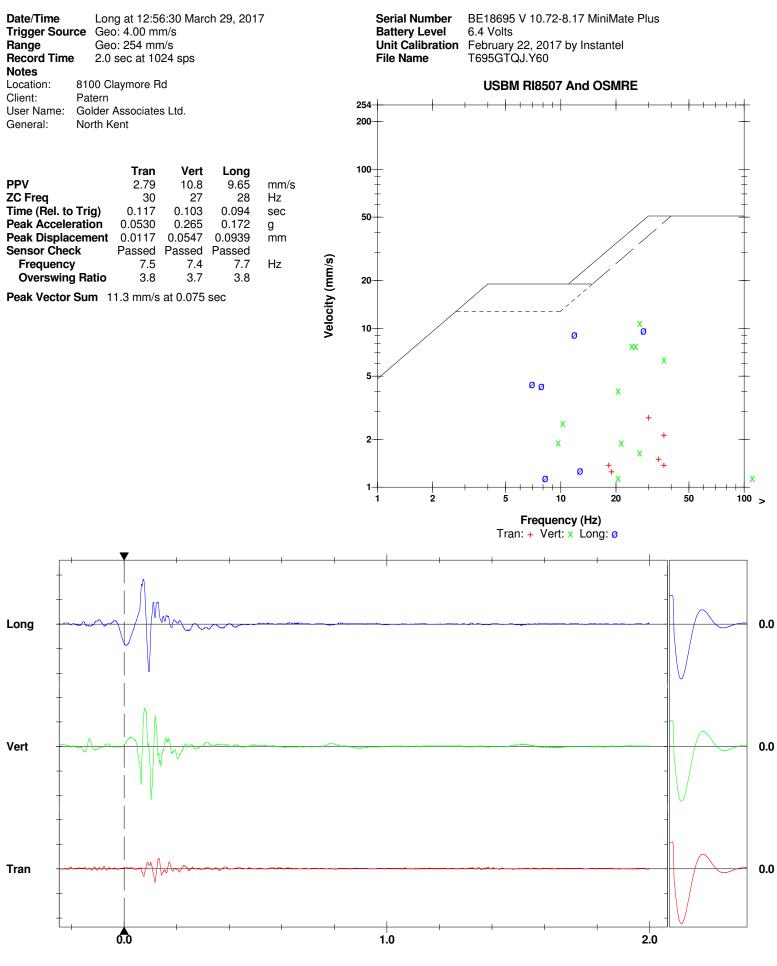
Patern

North Kent

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQJ.XW0

Tran Dominant Frequency = 24.3 Hz., Amplitude = 0.0791, PPV from Event = 2.92 mm/s 0.08 0.06 0.04 0.02 0.0 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 2 10 20 30 40 50 60 70 80 90 100 Vert Dominant Frequency = 21.8 Hz., Amplitude = 0.241, PPV from Event = 9.02 mm/s 0.40+ 0.35 0.30-0.25-0.20-0.15 0.10-0.05 0.0 20 10 30 40 50 60 70 80 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 2 Long Dominant Frequency = 17.0 Hz., Amplitude = 0.359, PPV from Event = 10.8 mm/s V | -0.40+ ____ _ _ -____ 0.35-0.30-0.25 0.20 0.15 0.10 0.05 0.0-





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 5.00 mm/s/div Trigger =

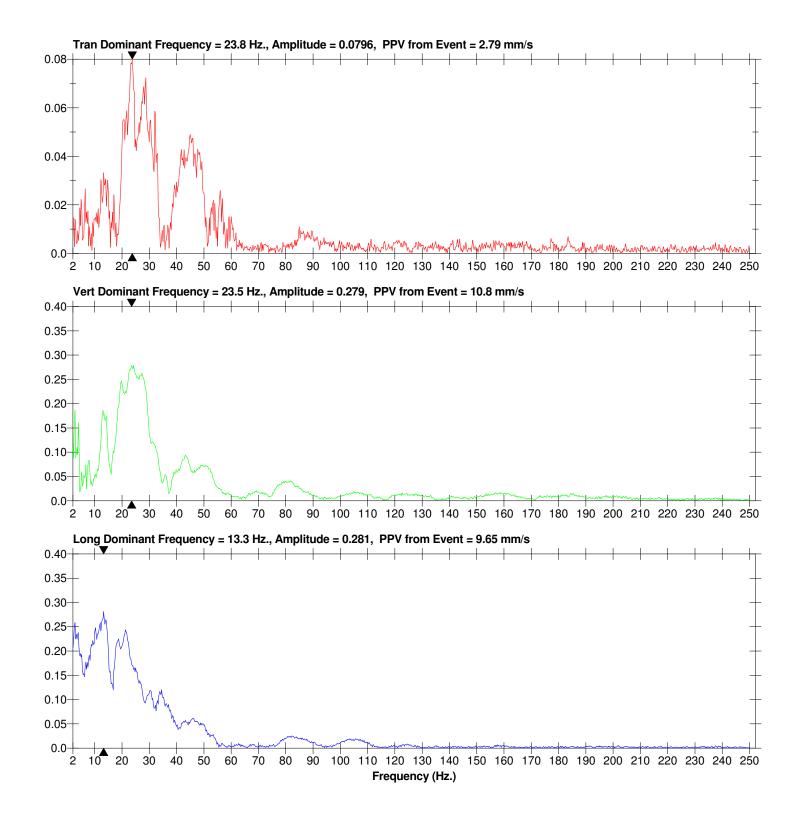


Long at 12:56:30 March 29, 2017 Date/Time Trigger Source Geo: 4.00 mm/s Range Geo: 254 mm/s **Record Time** 2.0 sec at 1024 sps

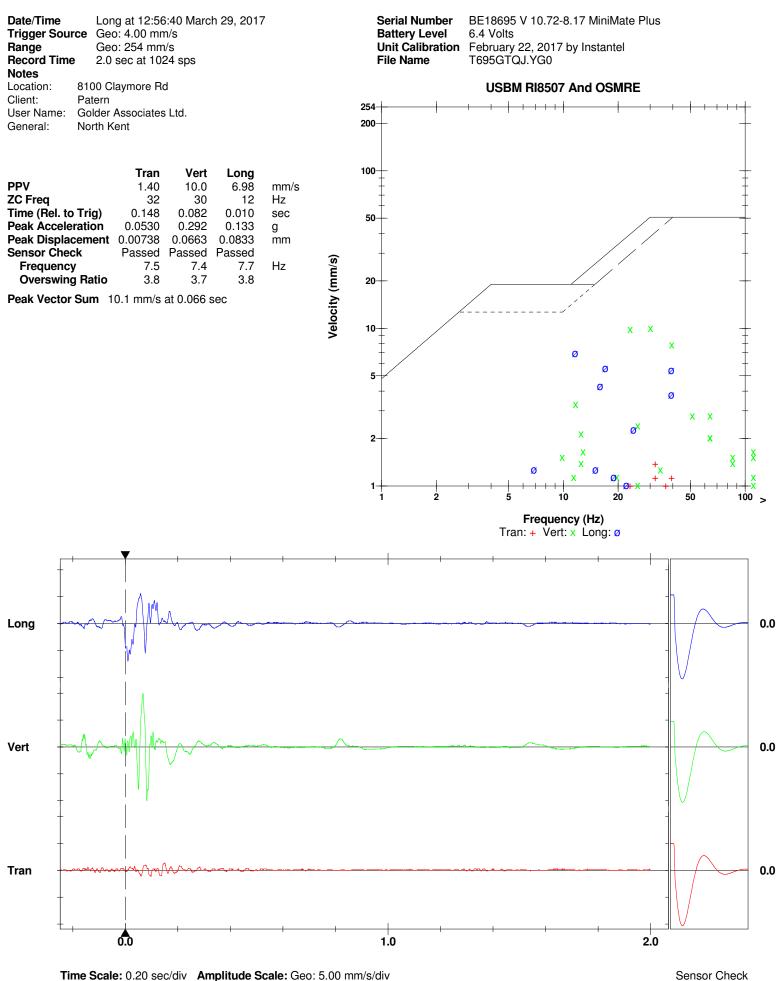
Serial Number BE18695 V 10.72-8.17 MiniMate Plus **Battery Level** 6.4 Volts Unit Calibration February 22, 2017 by Instantel File Name T695GTQJ.Y60

Notes Location: Client: Patern User Name: General: North Kent

8100 Claymore Rd Golder Associates Ltd.







Trigger = 🕨 Printed: April 24, 2017 (V 10.60 - 10.60)

Format © 1995-2013 Xmark Corporation

Sensor Check



Notes

Location: Client:

General:

User Name:

FFT Report

 Date/Time
 Long at 12:56:40 March 29, 2017

 Trigger Source
 Geo: 4.00 mm/s

 Range
 Geo: 254 mm/s

 Record Time
 2.0 sec at 1024 sps

8100 Claymore Rd

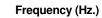
Golder Associates Ltd.

Patern

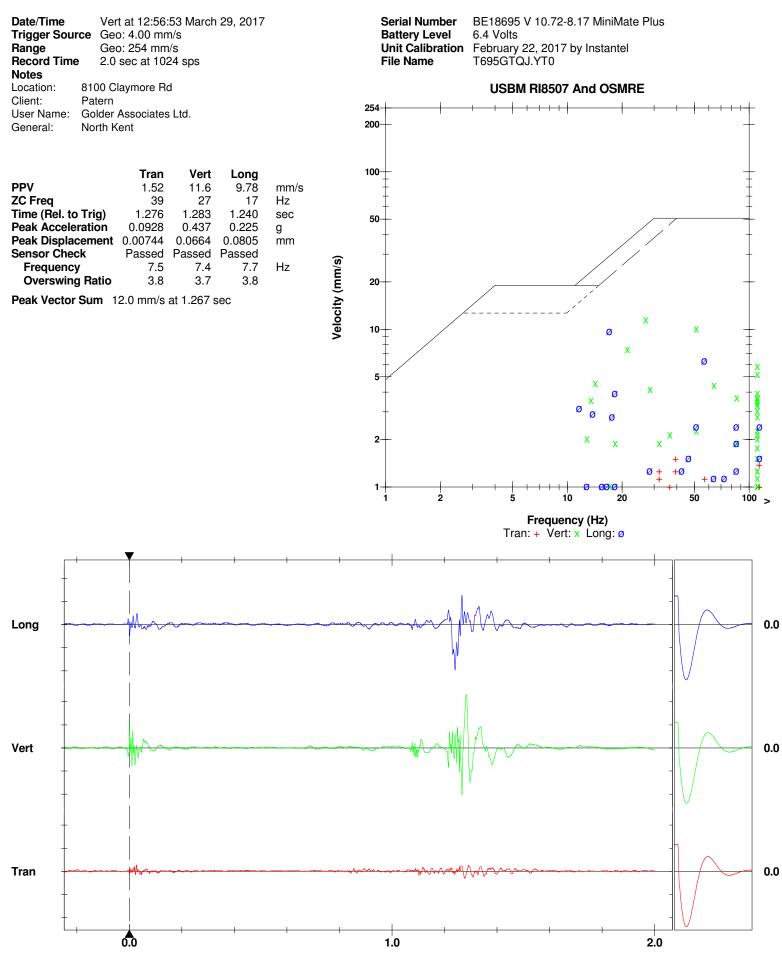
North Kent

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQJ.YG0

Tran Dominant Frequency = 31.0 Hz., Amplitude = 0.0645, PPV from Event = 1.40 mm/s 0.08 0.06 0.04 0.02 0.0 30 120 130 140 150 160 170 180 190 200 210 220 230 240 250 2 10 20 40 50 60 70 80 90 100 110 Vert Dominant Frequency = 22.5 Hz., Amplitude = 0.261, PPV from Event = 10.0 mm/s ∣**▼** 0.40+ 0.35 0.30-0.25 0.20 0.15 0.10 0.05 0.0 20 30 40 50 60 70 80 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 2 10 Long Dominant Frequency = 15.3 Hz., Amplitude = 0.301, PPV from Event = 6.98 mm/s _ **▼** | 0.40+ 1 _ _ _ ____ ____ 0.35-0.30-0.25 0.20 0.15 0.10 0.05 0.0-90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 2 30 10 20 40 50 60 70 80







Time Scale: 0.20 sec/div Amplitude Scale: Geo: 5.00 mm/s/div Trigger = ▶ ____ ◀



 Date/Time
 Vert at 12:56:53 March 29, 2017

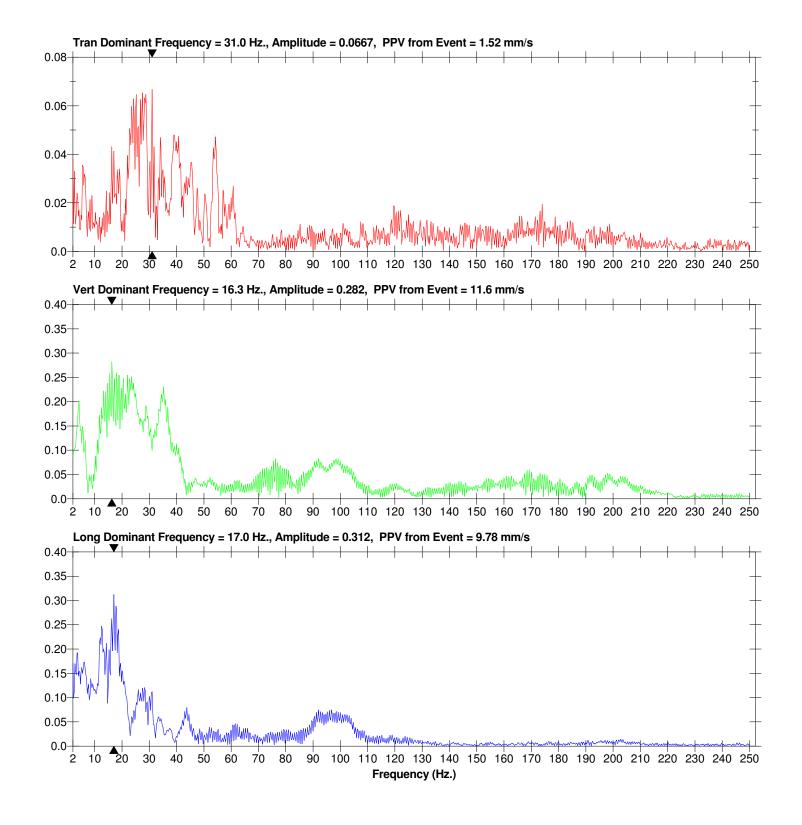
 Trigger Source
 Geo: 4.00 mm/s

 Range
 Geo: 254 mm/s

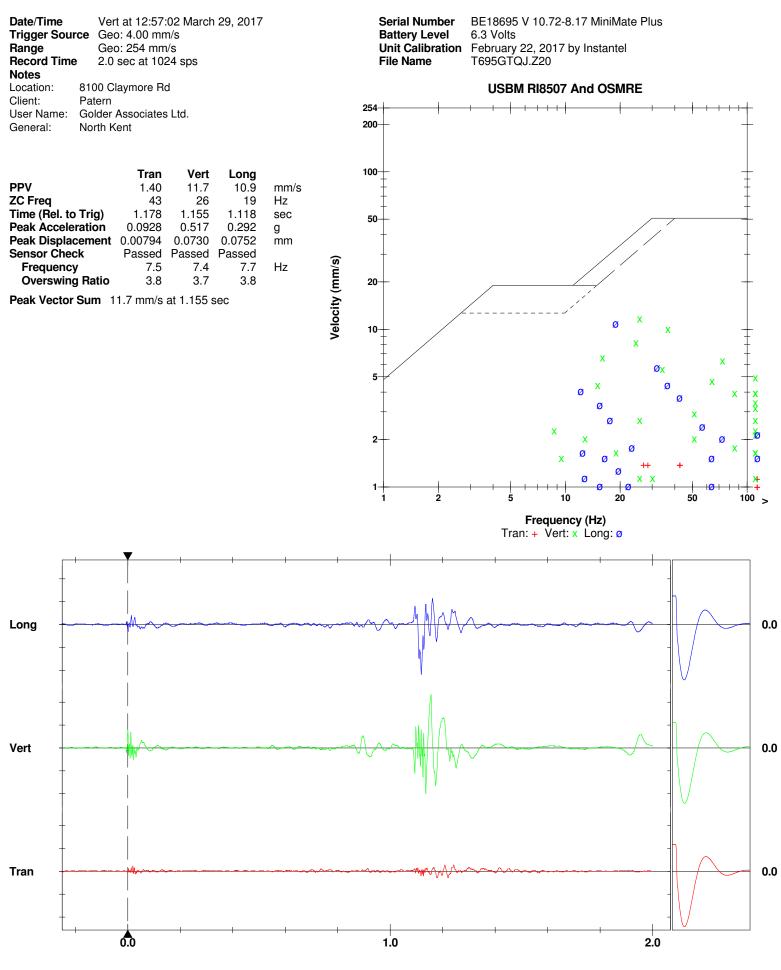
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQJ.YT0









Time Scale: 0.20 sec/div Amplitude Scale: Geo: 5.00 mm/s/div Trigger = ► ____ ←



 Date/Time
 Vert at 12:57:02 March 29, 2017

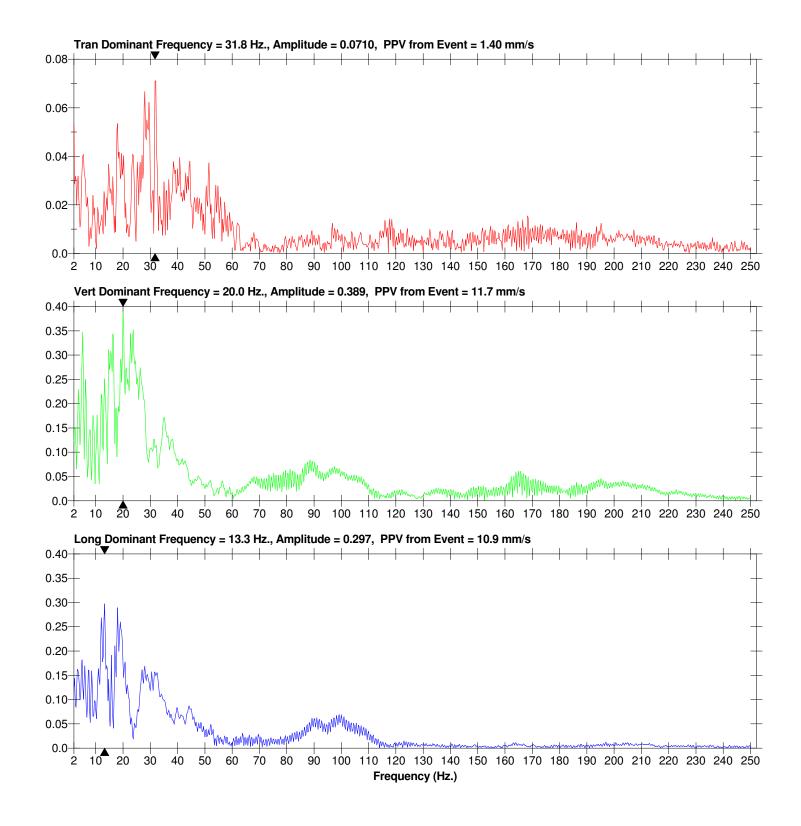
 Trigger Source
 Geo: 4.00 mm/s

 Range
 Geo: 254 mm/s

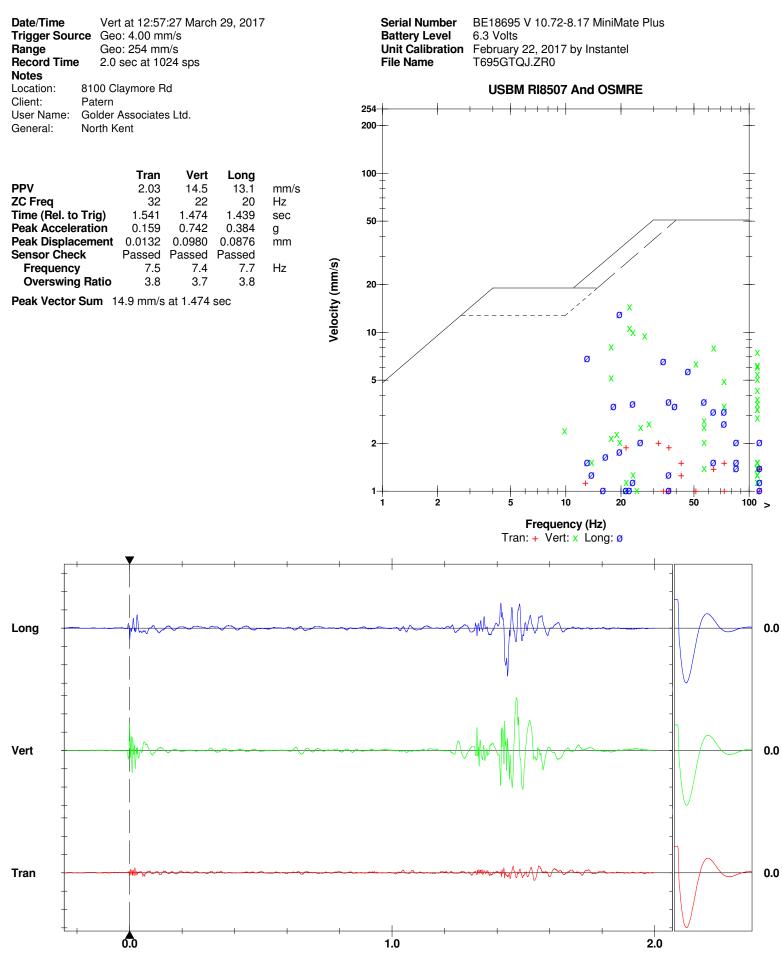
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQJ.Z20

Notes







Time Scale: 0.20 sec/div Amplitude Scale: Geo: 5.00 mm/s/div Trigger =



 Date/Time
 Vert at 12:57:27 March 29, 2017

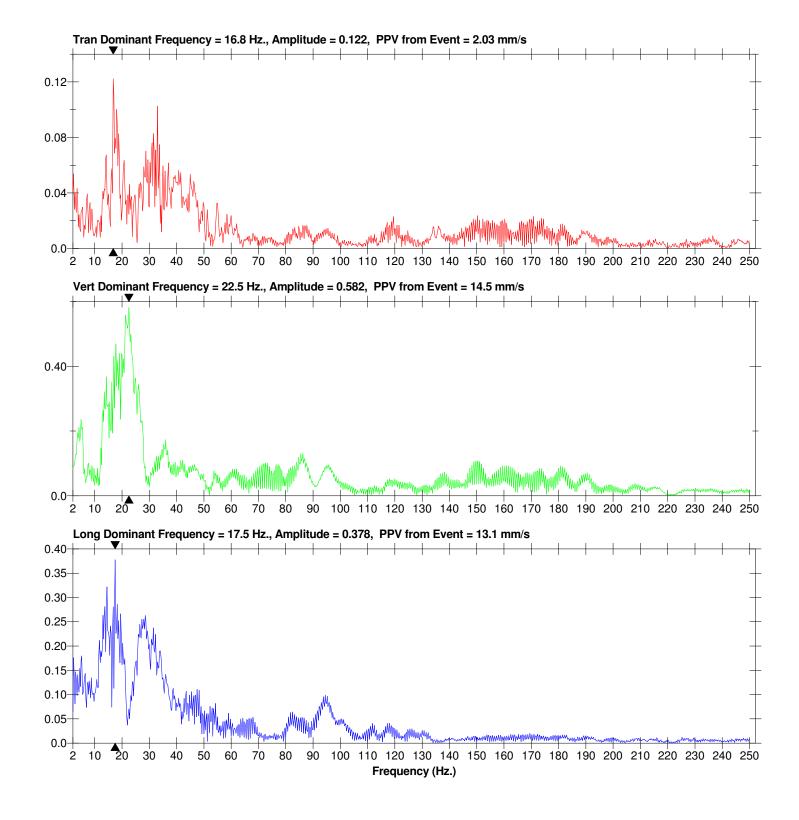
 Trigger Source
 Geo: 4.00 mm/s

 Range
 Geo: 254 mm/s

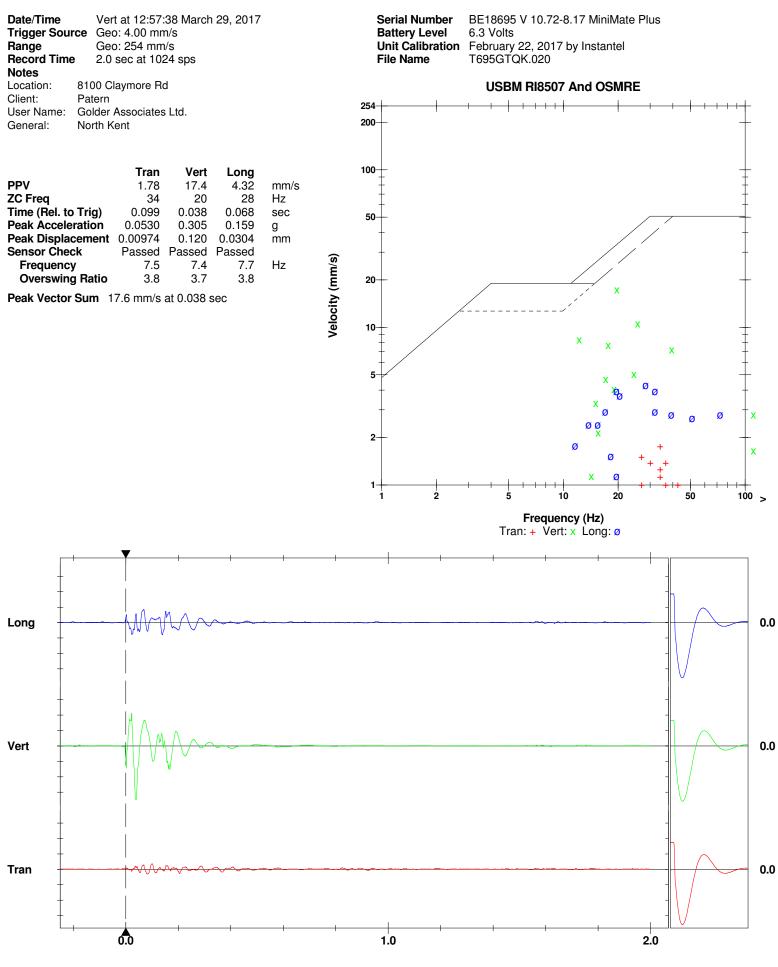
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQJ.ZR0









Time Scale: 0.20 sec/div Amplitude Scale: Geo: 5.00 mm/s/div Trigger = ▶── ──◀



 Date/Time
 Vert at 12:57:38 March 29, 2017

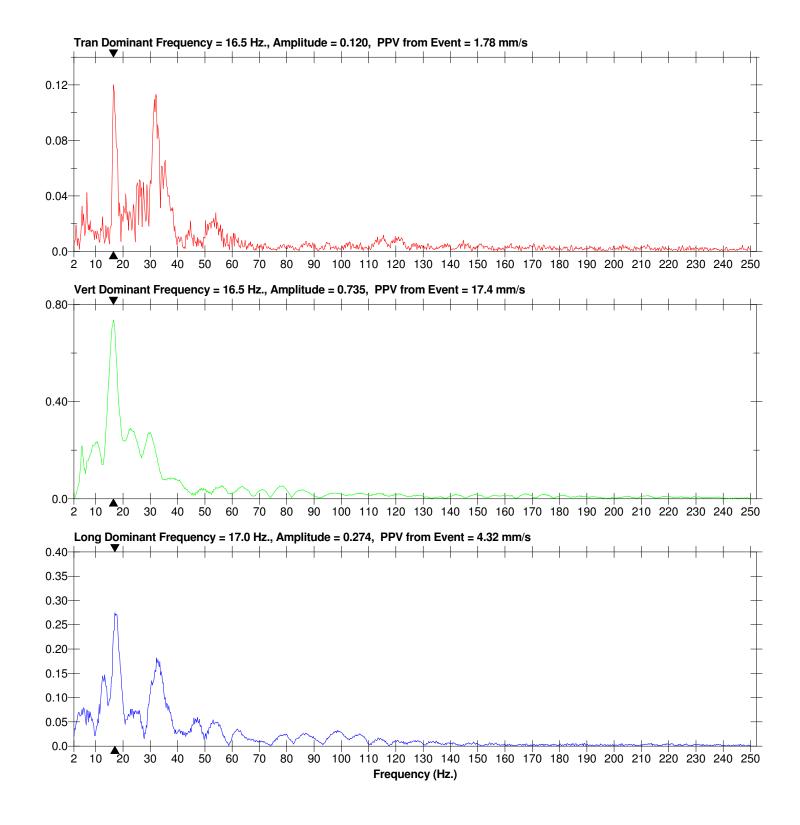
 Trigger Source
 Geo: 4.00 mm/s

 Range
 Geo: 254 mm/s

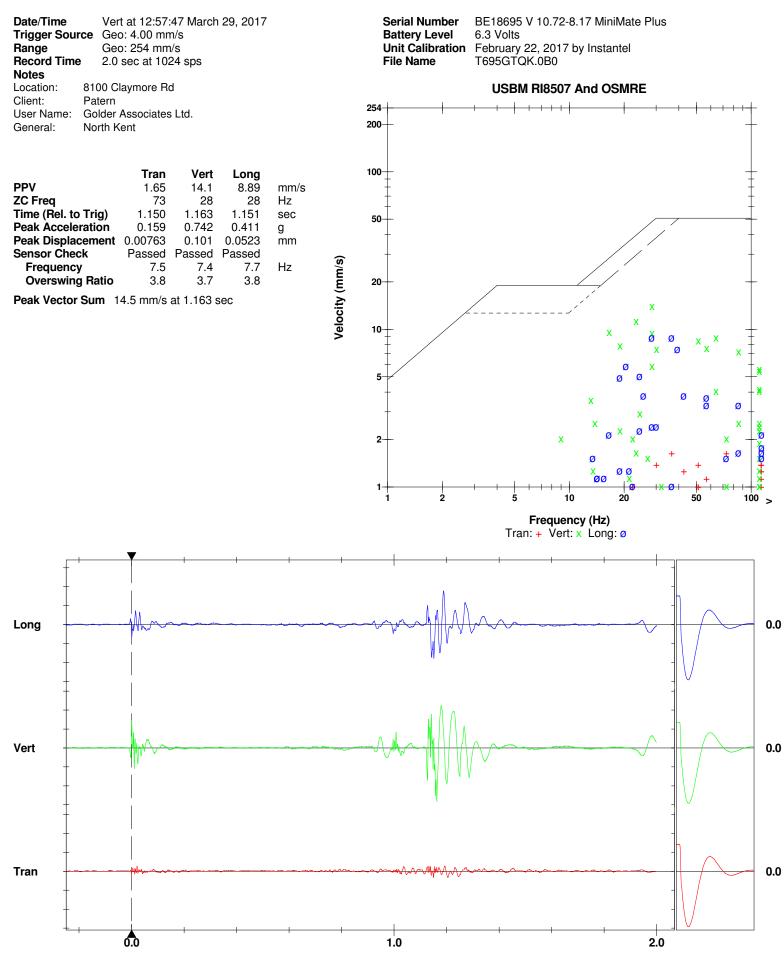
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQK.020

Notes







Trigger = ► _____ Printed: April 24, 2017 (V 10.60 - 10.60)

Time Scale: 0.20 sec/div Amplitude Scale: Geo: 5.00 mm/s/div

Format © 1995-2013 Xmark Corporation

Sensor Check



 Date/Time
 Vert at 12:57:47 March 29, 2017

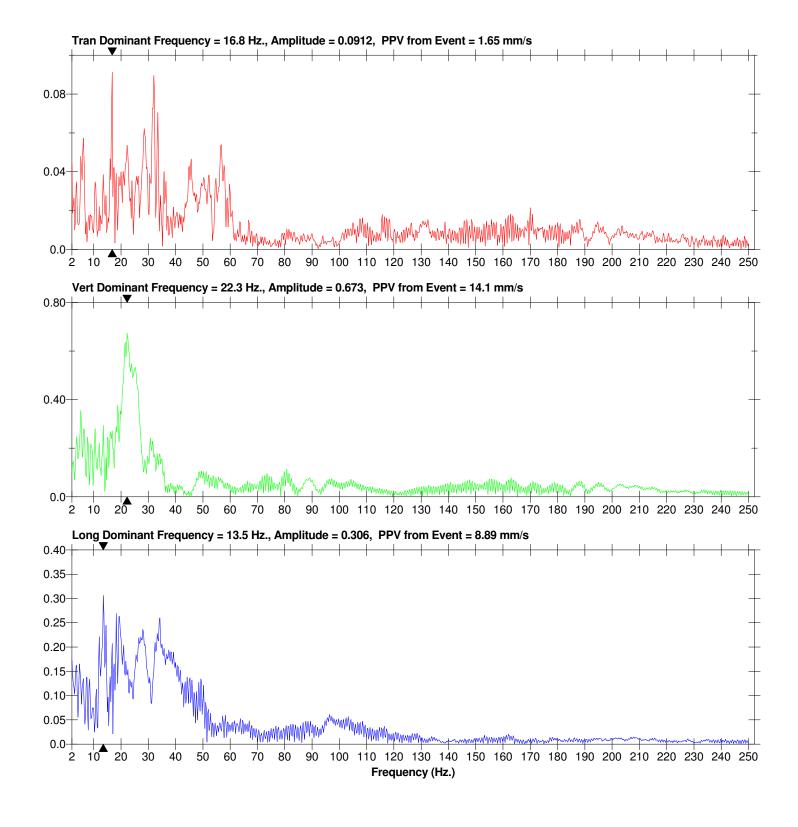
 Trigger Source
 Geo: 4.00 mm/s

 Range
 Geo: 254 mm/s

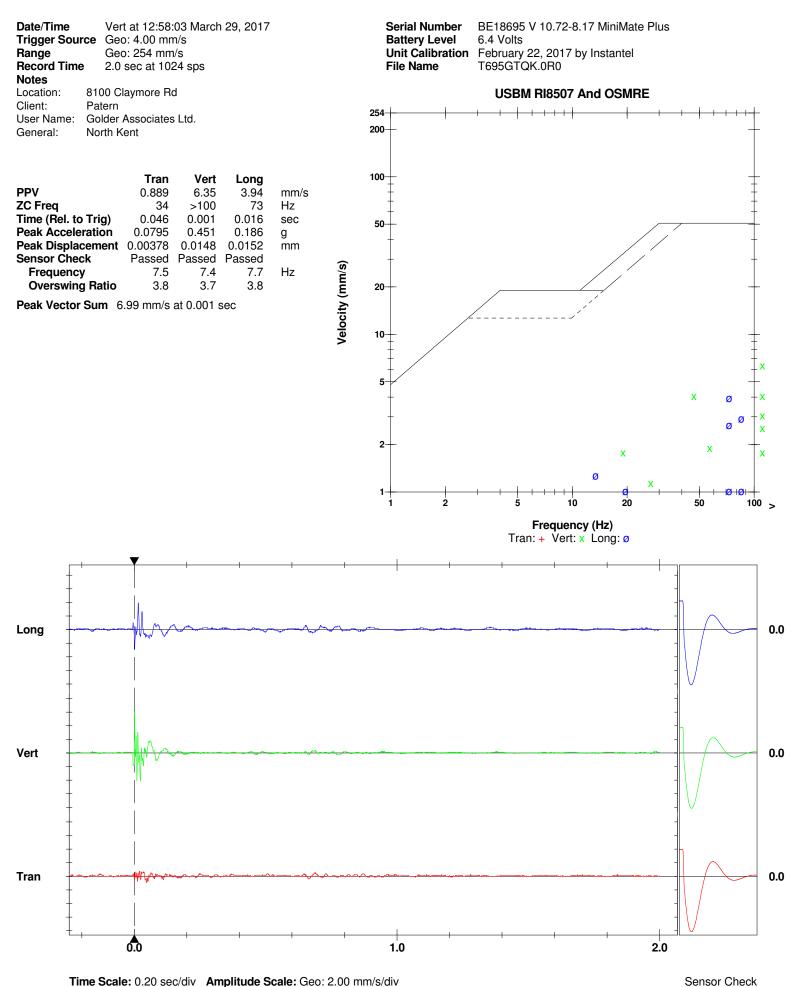
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQK.0B0











 Date/Time
 Vert at 12:58:03 March 29, 2017

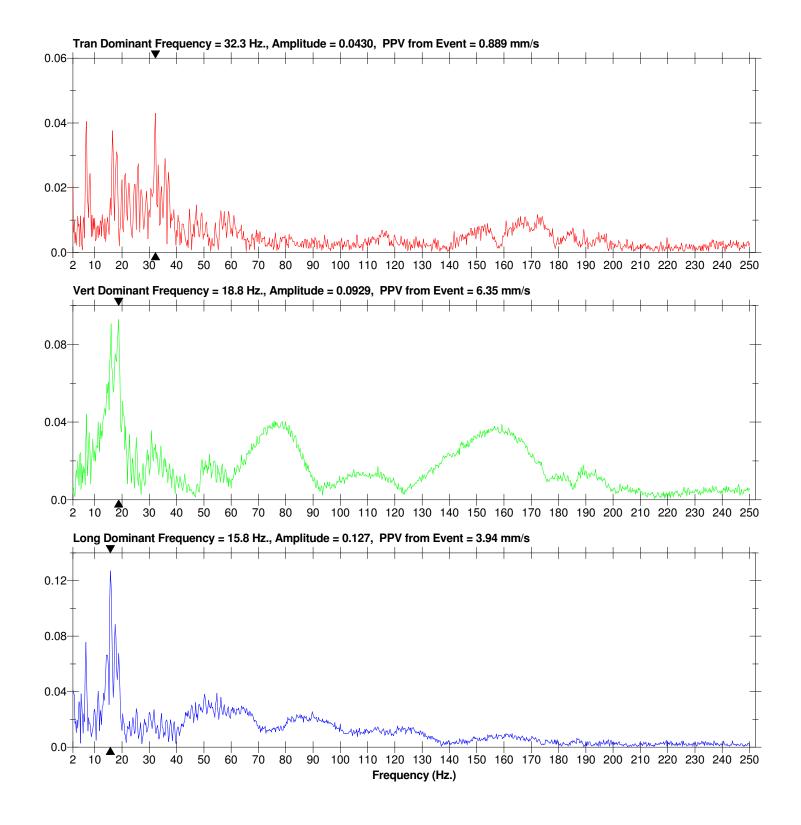
 Trigger Source
 Geo: 4.00 mm/s

 Range
 Geo: 254 mm/s

 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQK.0R0





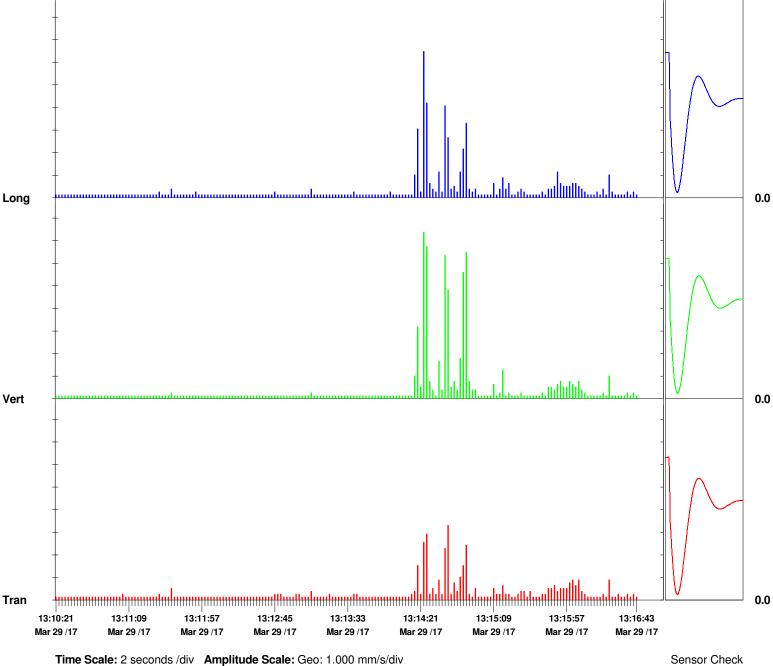


Histogram S Histogram F Number of Ir Range Sample Rate Notes	inish Time ntervals	13:10:19 March 29, 2017 13:16:44 March 29, 2017 192.00 at 2 seconds Geo:254 mm/s 1024sps
Location: Client: User Name: General:	8100 Claym Patern Golder Asse North Kent	

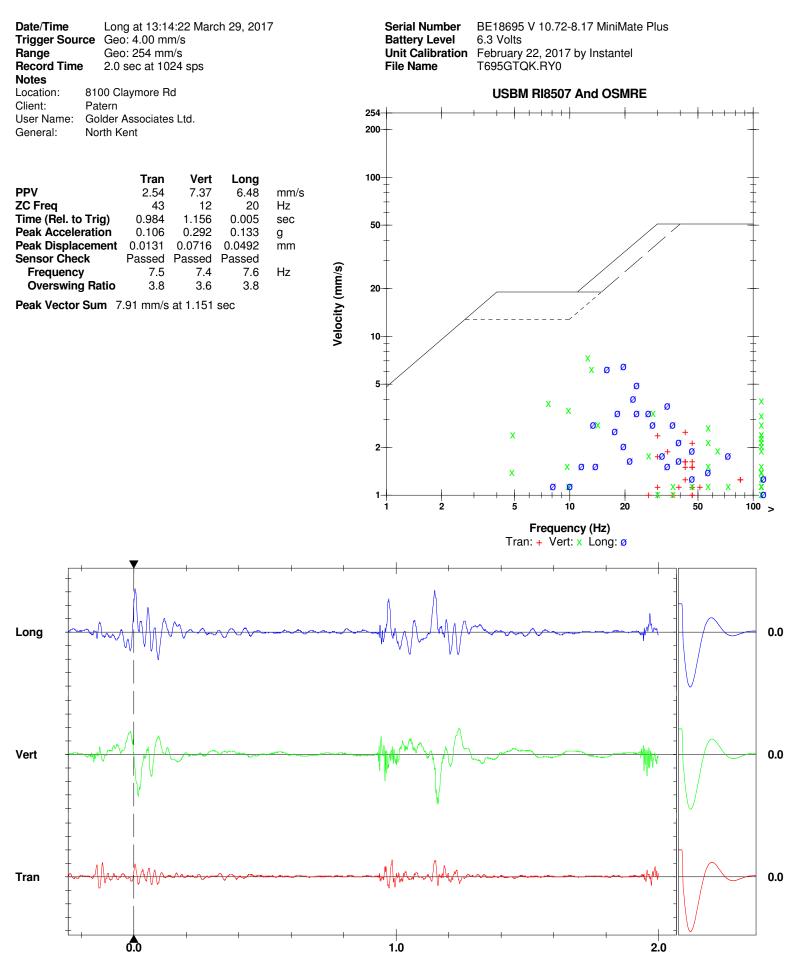
Serial Number BE18695 V 10.72-8.17 MiniMate Plus **Battery Level** 6.4 Volts Unit Calibration February 22, 2017 by Instantel File Name T695GTQK.L70

	Tran	Vert	Long	
PPV	3.30	7.37	6.48	mm/s
ZC Freq	39	12	20	Hz
Date	Mar 29 /17	Mar 29 /17	Mar 29 /17	
Time	13:14:39	13:14:23	13:14:23	
Sensor Check	Passed	Passed	Passed	
Frequency	7.5	7.4	7.6	Hz
Overswing Ratio	3.8	3.6	3.8	

Peak Vector Sum 7.91 mm/s on March 29, 2017 at 13:14:23







Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger = ▶─── ◀



Notes

Location: Client:

General:

User Name:

FFT Report

 Date/Time
 Long at 13:14:22 March 29, 2017

 Trigger Source
 Geo: 4.00 mm/s

 Range
 Geo: 254 mm/s

 Record Time
 2.0 sec at 1024 sps

8100 Claymore Rd

Golder Associates Ltd.

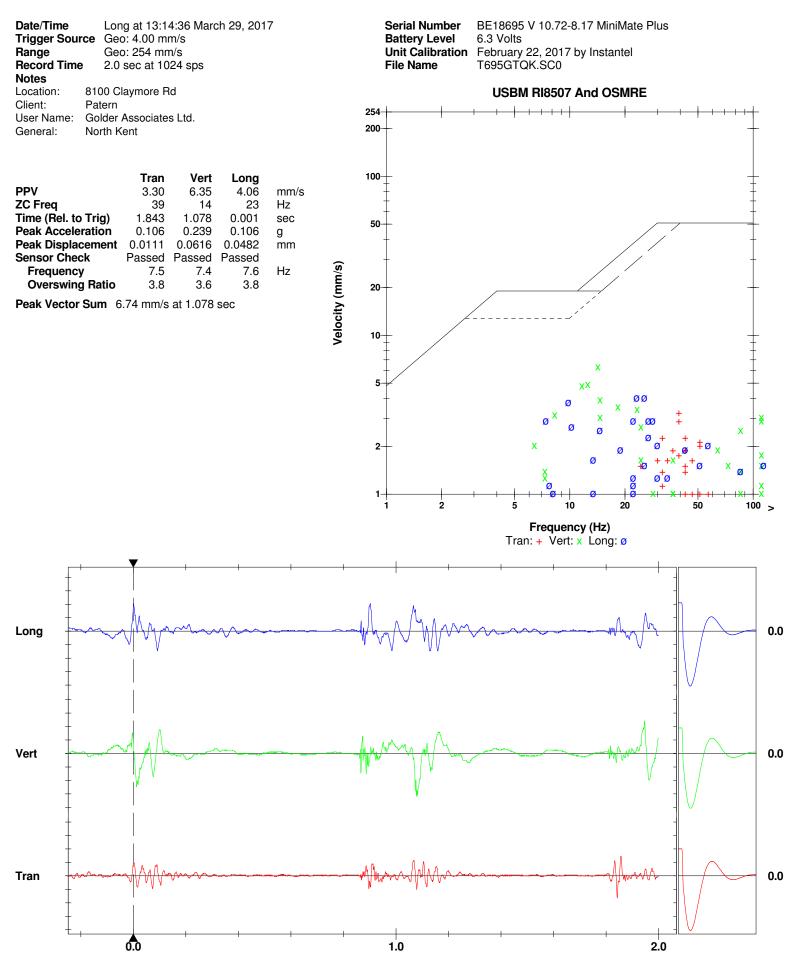
Patern

North Kent

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQK.RY0

Tran Dominant Frequency = 40.0 Hz., Amplitude = 0.139, PPV from Event = 2.54 mm/s 0.12 0.08 0.04 mmmmm 0.0 40 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 2 10 20 30 50 60 70 80 Vert Dominant Frequency = 4.50 Hz., Amplitude = 0.550, PPV from Event = 7.37 mm/s 0.40 mpmmypMmmmymm $\sim m_{m} m m m$ MMMM4 mm 0.0 2 30 40 50 60 70 80 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 10 20 Long Dominant Frequency = 5.25 Hz., Amplitude = 0.333, PPV from Event = 6.48 mm/s 0.40 _ +_ _ _ + 0.35 0.30-0.25 0.20 0.15 0.10 0.05 www.p.M.M.M. Marine man and marine marin 0.0 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 20 30 40 50 60 70 80 2 10 Frequency (Hz.)





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger = ► ____ ←



Notes

Location: Client:

User Name:

FFT Report

 Date/Time
 Long at 13:14:36 March 29, 2017

 Trigger Source
 Geo: 4.00 mm/s

 Range
 Geo: 254 mm/s

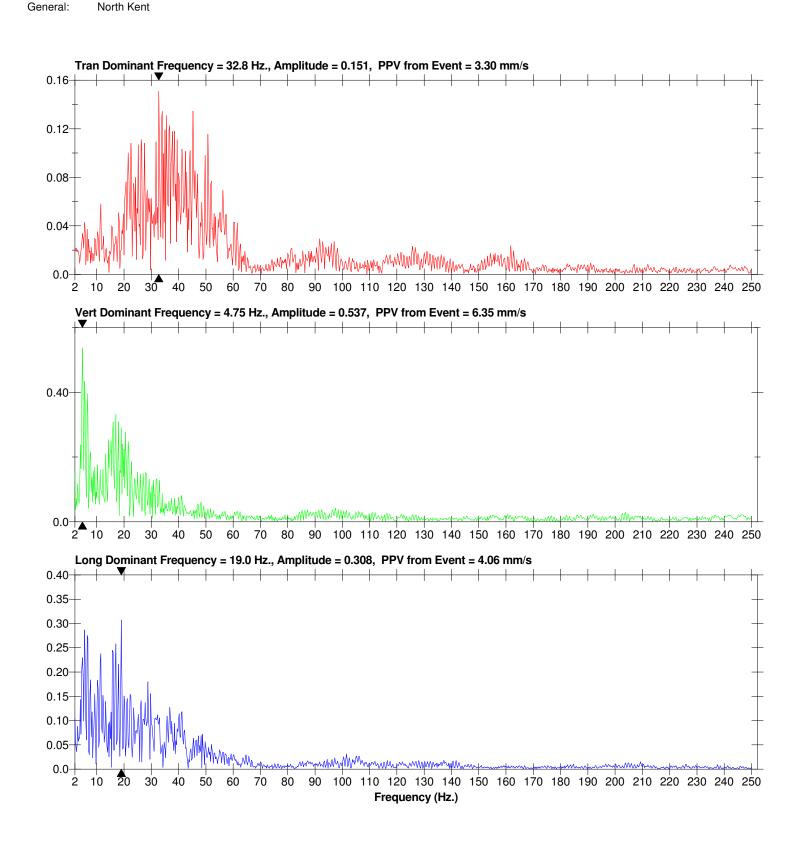
 Record Time
 2.0 sec at 1024 sps

8100 Claymore Rd

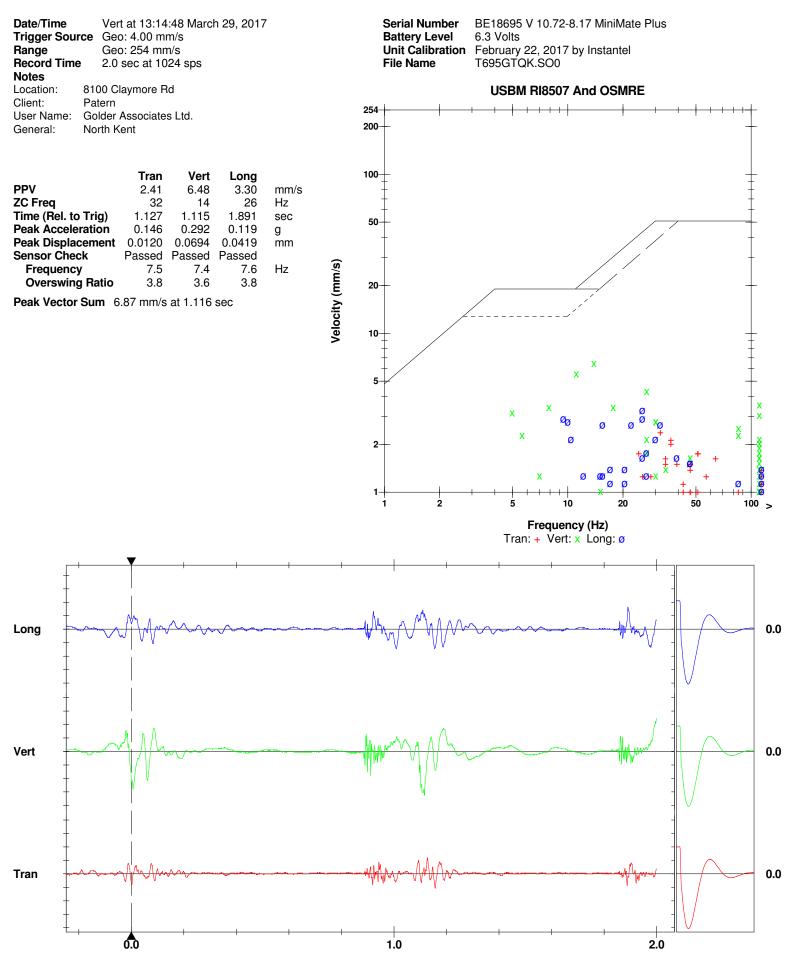
Golder Associates Ltd.

Patern

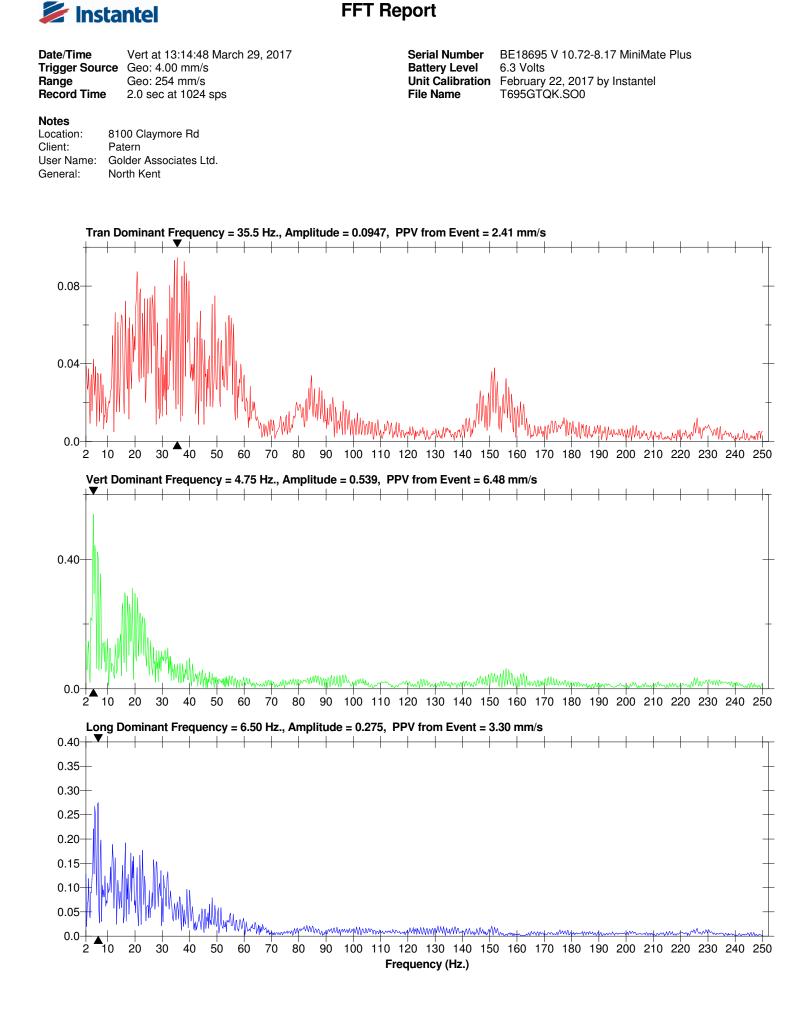
Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQK.SC0







Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger = ► ____



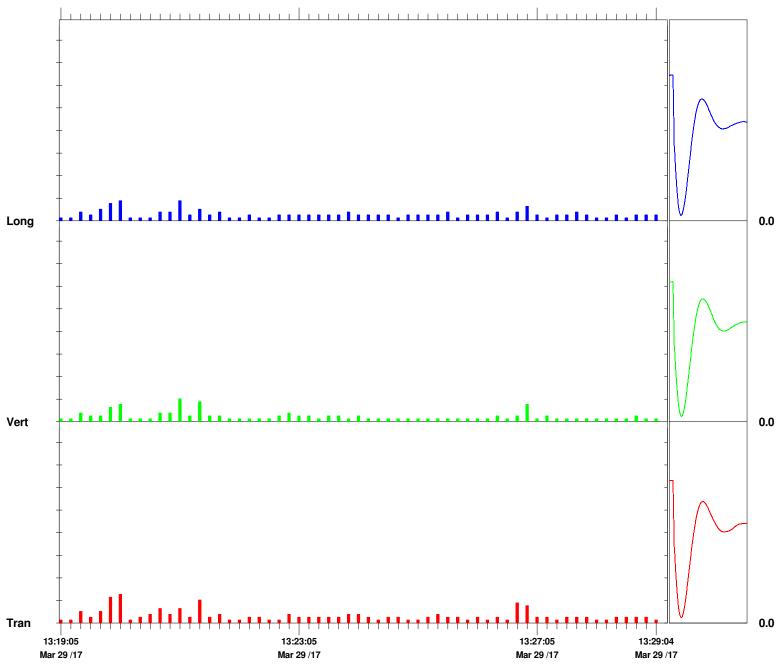


Histogram S Histogram F Number of Ir Range Sample Rate Notes	inish Time ntervals	13:18:55 March 29, 2017 13:29:04 March 29, 2017 304.00 at 2 seconds Geo:254 mm/s 1024sps
Location: Client: User Name: General:	8100 Clayn Patern Golder Ass North Kent	

Serial Number
Battery LevelBE18695 V 10.72-8.17 MiniMate Plus
6.3 VoltsUnit Calibration
File NameFebruary 22, 2017 by Instantel
T695GTQK.ZJ0

	Iran	Vert	Long	
PPV	1.27	1.02	0.889	mm/s
ZC Freq	22	28	14	Hz
Date	Mar 29 /17	Mar 29 /17	Mar 29 /17	
Time	13:19:59	13:20:57	13:19:59	
Sensor Check	Passed	Passed	Passed	
Frequency	7.5	7.5	7.7	Hz
Overswing Ratio	3.9	3.7	3.8	

Peak Vector Sum 1.35 mm/s on March 29, 2017 at 13:19:59



Time Scale: 10 seconds /div Amplitude Scale: Geo: 1.000 mm/s/div

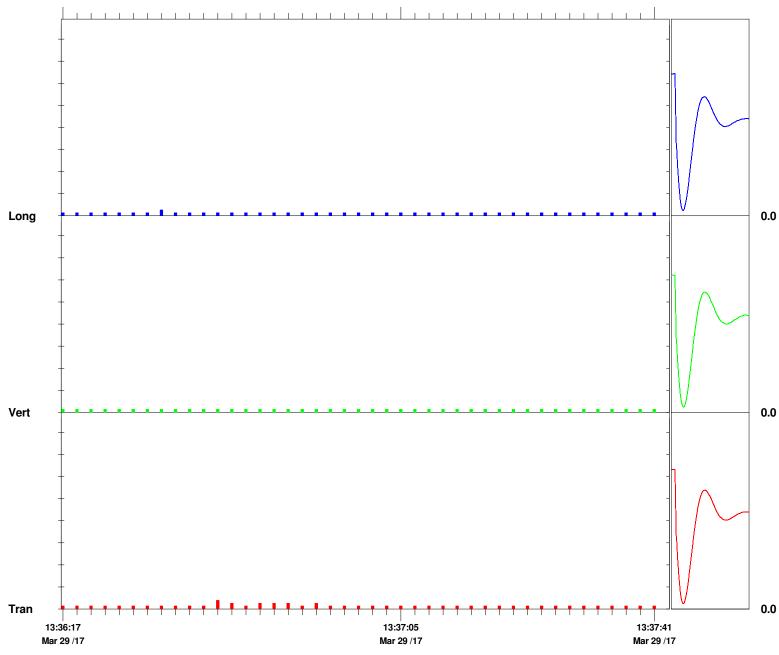


Histogram S Histogram F Number of Ir Range Sample Rate Notes	inish Time ntervals	13:36:15 March 29, 2017 13:37:42 March 29, 2017 43.00 at 2 seconds Geo:254 mm/s 1024sps
Location: Client: User Name: General:	8100 Claym Patern Golder Asse North Kent	

Serial Number
Battery LevelBE18695 V 10.72-8.17 MiniMate Plus
6.4 VoltsUnit Calibration
File NameFebruary 22, 2017 by Instantel
T695GTQL.SF0

	Tran	Vert	Long	
PPV	0.381	0.127	0.254	mm/s
ZC Freq	47	N/A	>100	Hz
Date	Mar 29 /17	Mar 29 /17	Mar 29 /17	
Time	13:36:39	13:36:17	13:36:31	
Sensor Check	Passed	Passed	Passed	
Frequency	7.5	7.5	7.7	Hz
Overswing Ratio	3.8	3.6	3.8	

Peak Vector Sum 0.402 mm/s on March 29, 2017 at 13:36:39 N/A: Not Applicable



Time Scale: 2 seconds /div Amplitude Scale: Geo: 1.000 mm/s/div

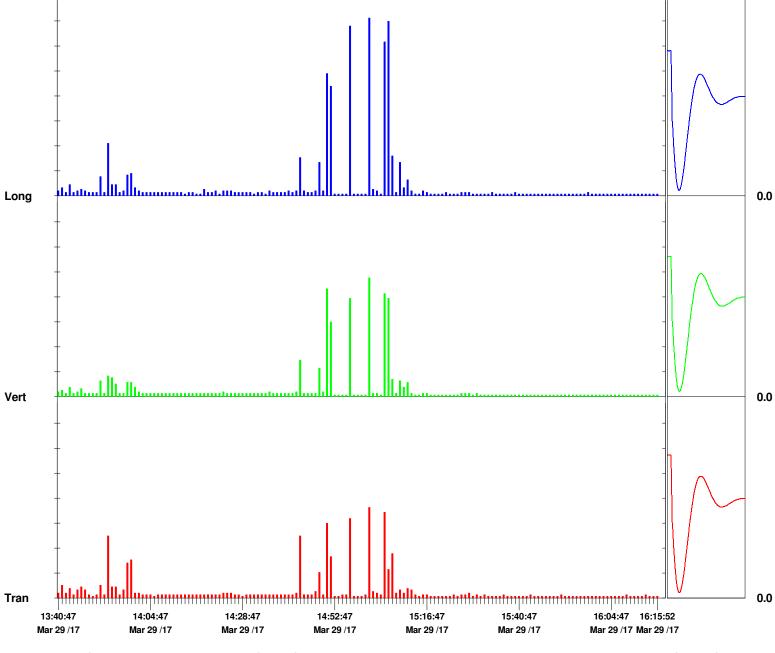


Histogram S Histogram F Number of Ir Range Sample Rate Notes	inish Time ntervals	13:39:47 March 29, 2017 16:15:52 March 29, 2017 4682.00 at 2 seconds Geo:254 mm/s 1024sps
Location: Client: User Name: General:	8100 Claym Patern Golder Asse North Kent	

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQL.YB0

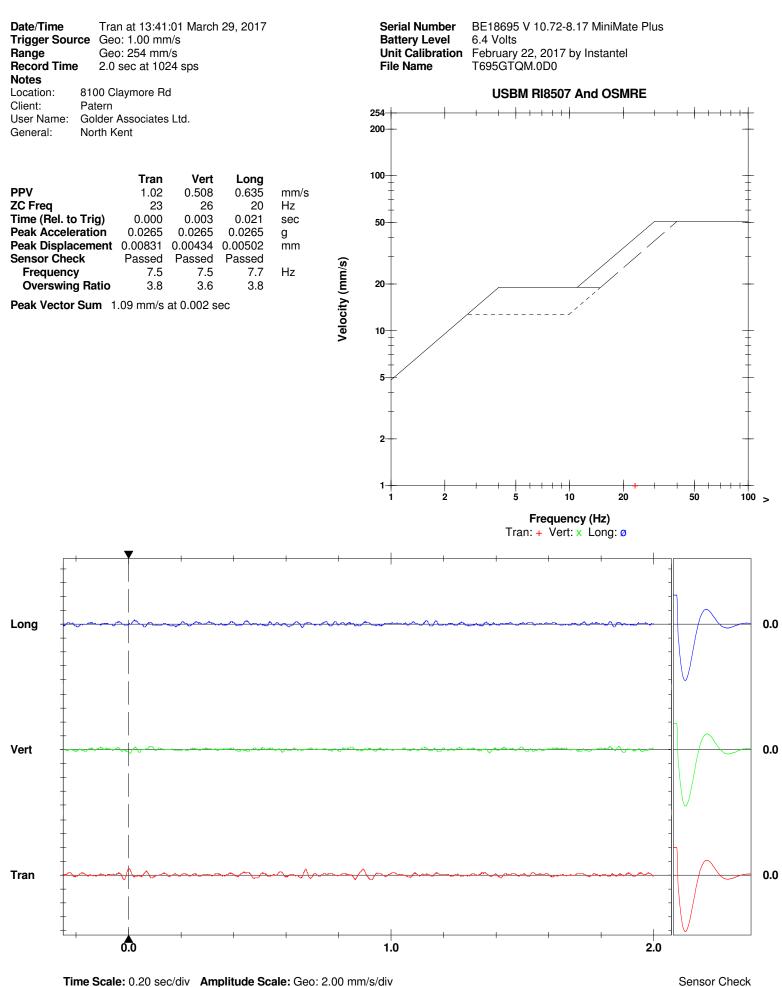
	Tran	Vert	Long	
PPV	7.24	9.52	14.2	mm/s
ZC Freq	39	>100	28	Hz
Date	Mar 29 /17	Mar 29 /17	Mar 29 /17	
Time	15:01:03	15:01:13	15:01:13	
Sensor Check	Passed	Passed	Passed	
Frequency	7.5	7.5	7.7	Hz
Overswing Ratio	3.8	3.6	3.8	

Peak Vector Sum 14.5 mm/s on March 29, 2017 at 15:01:13



Time Scale: 1 minute /div Amplitude Scale: Geo: 2.00 mm/s/div





Trigger = ► _____ Printed: April 24, 2017 (V 10.60 - 10.60)



 Date/Time
 Tran at 13:41:01 March 29, 2017

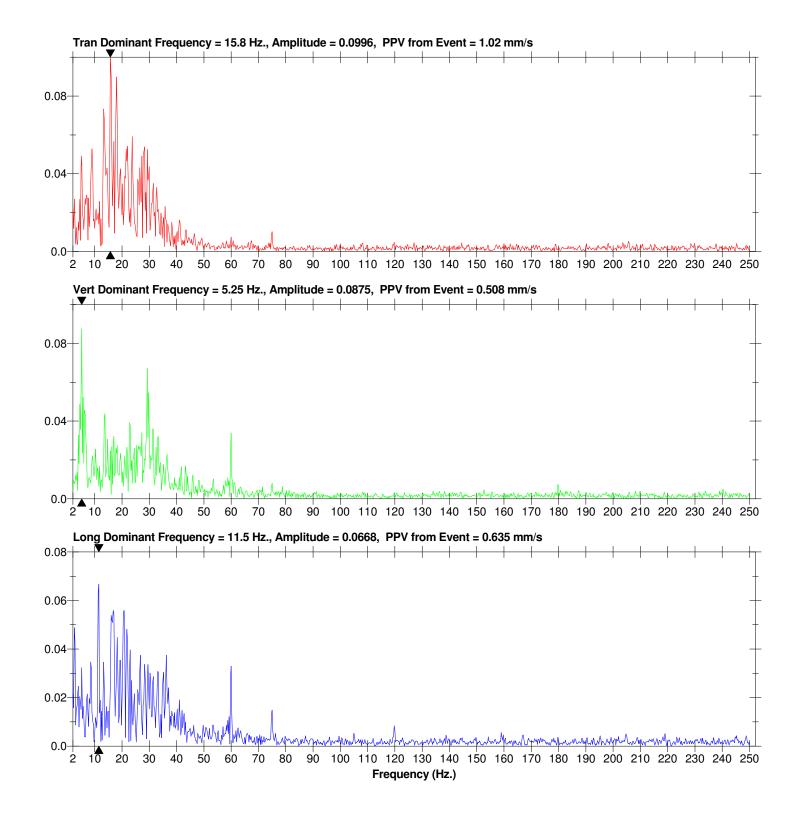
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

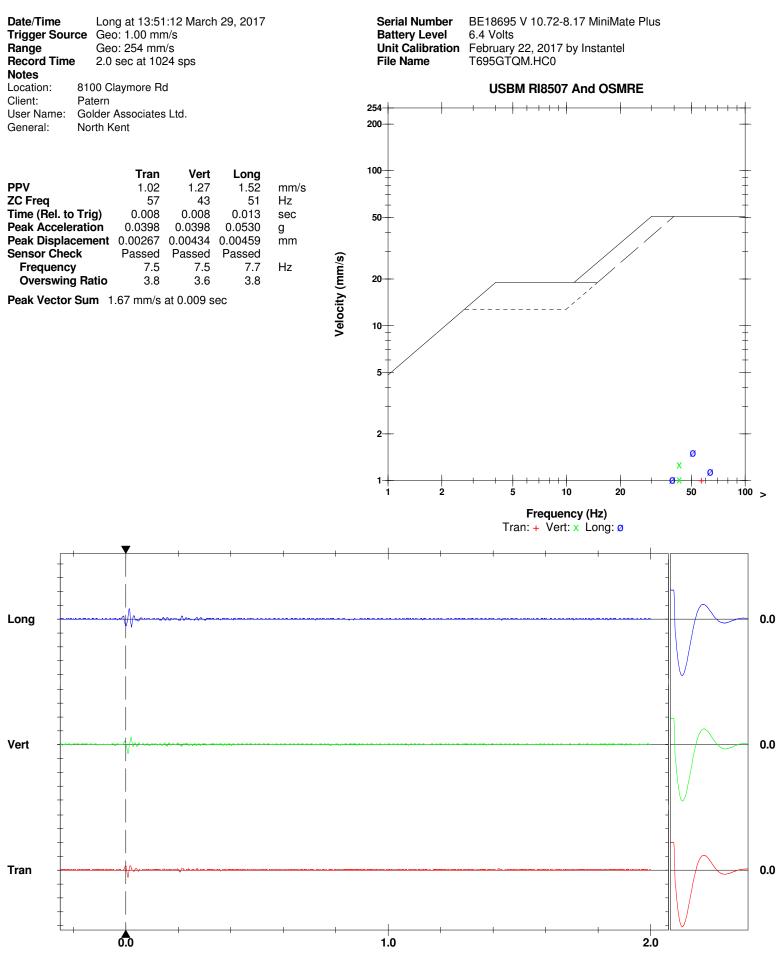
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQM.0D0

Notes







Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger = > _____



 Date/Time
 Long at 13:51:12 March 29, 2017

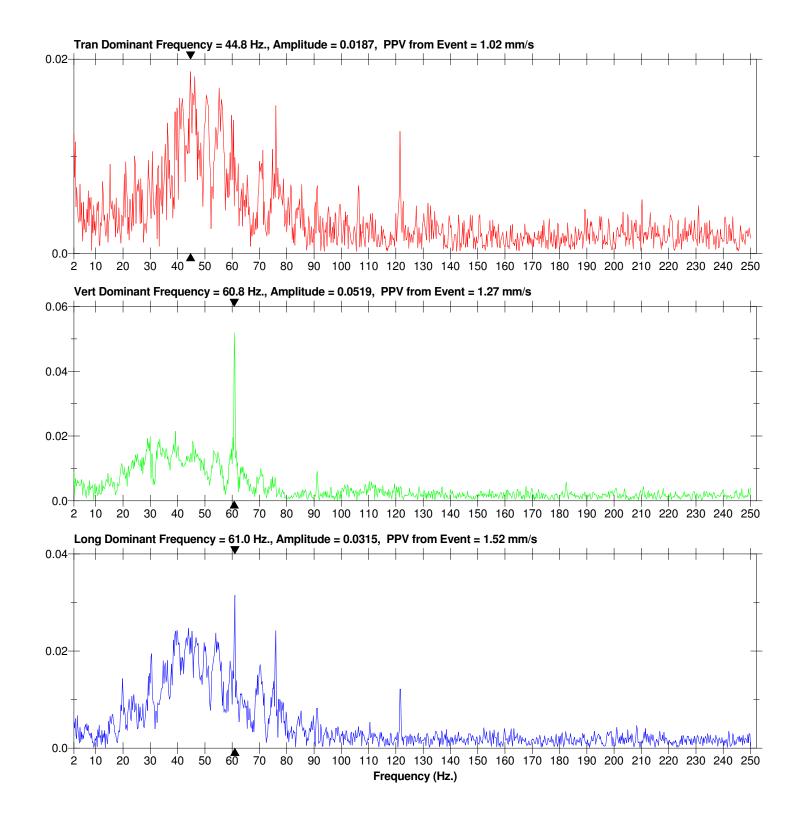
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

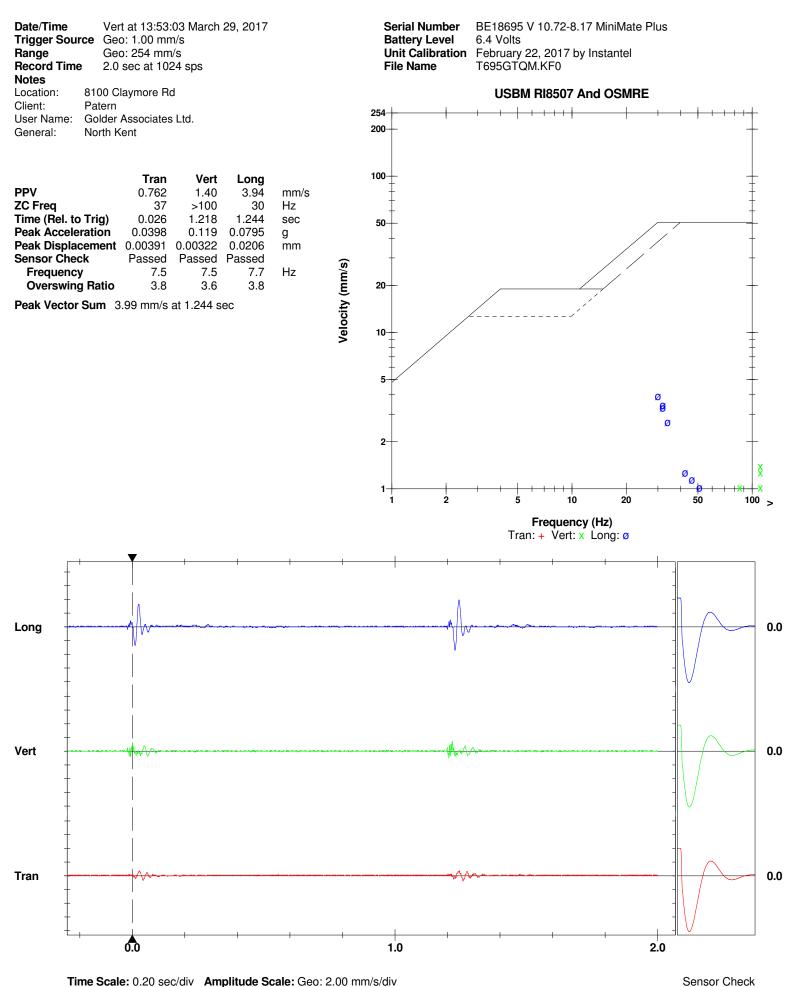
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQM.HC0

Notes







Trigger = ► _____ Printed: April 24, 2017 (V 10.60 - 10.60)



 Date/Time
 Vert at 13:53:03 March 29, 2017

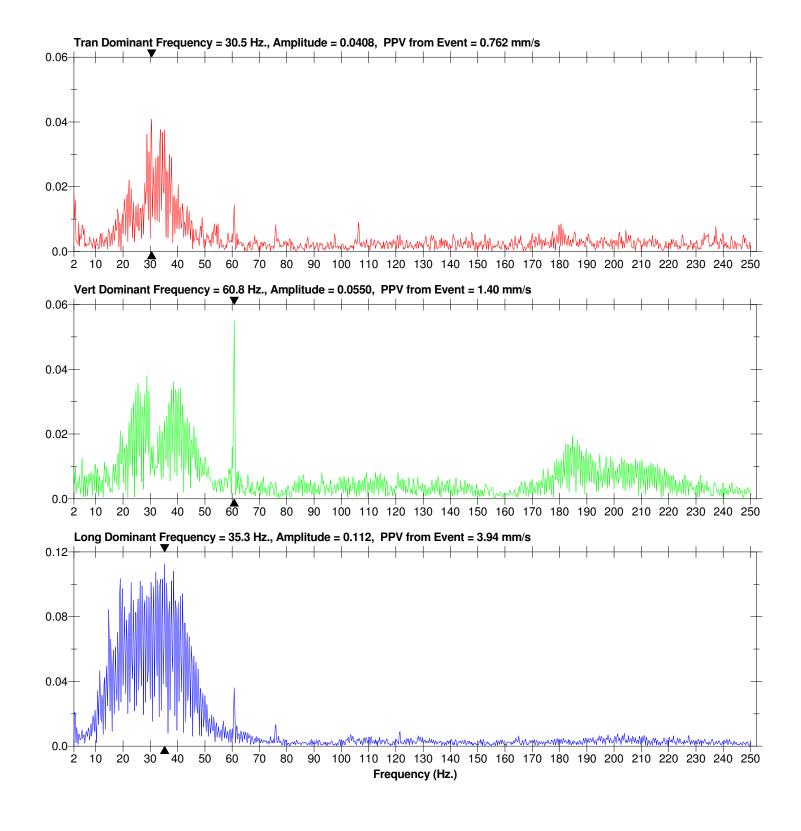
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

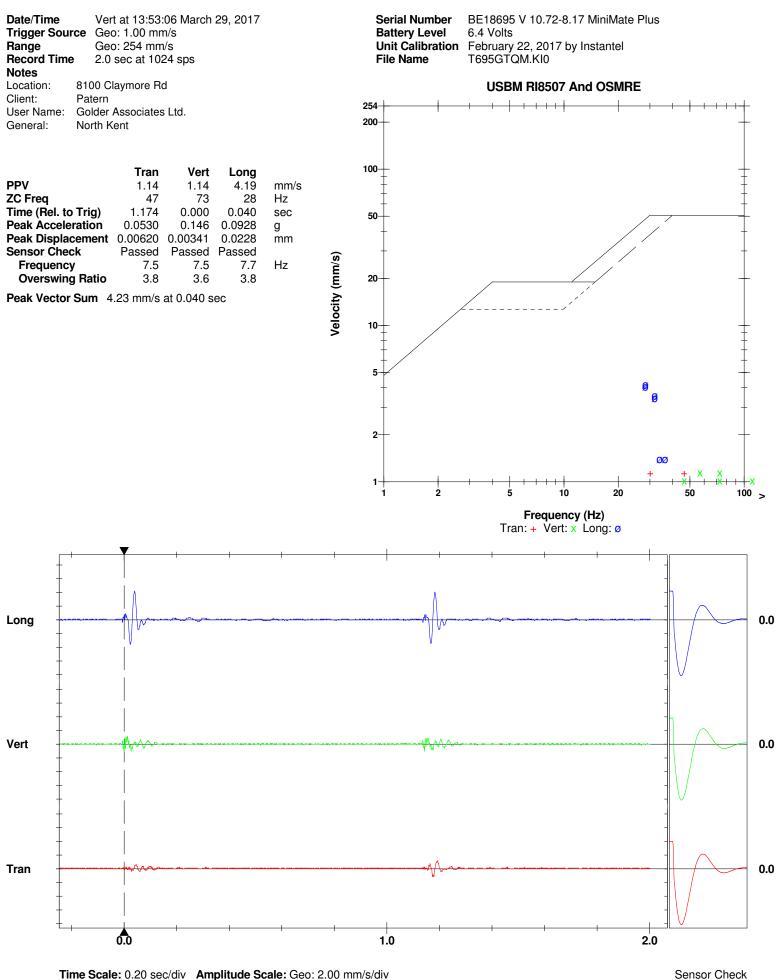
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQM.KF0









Trigger = 🕨 Printed: April 24, 2017 (V 10.60 - 10.60) Sensor Check



Notes

Location: Client:

FFT Report

 Date/Time
 Vert at 13:53:06 March 29, 2017

 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

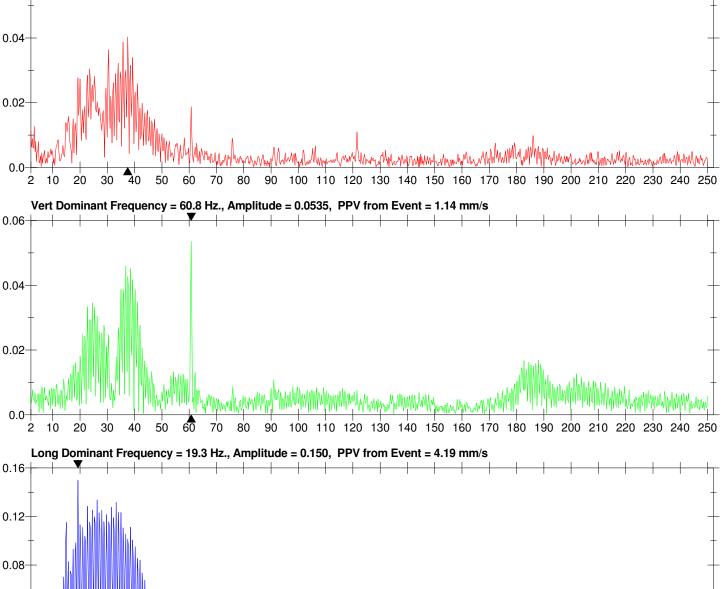
 Record Time
 2.0 sec at 1024 sps

8100 Claymore Rd

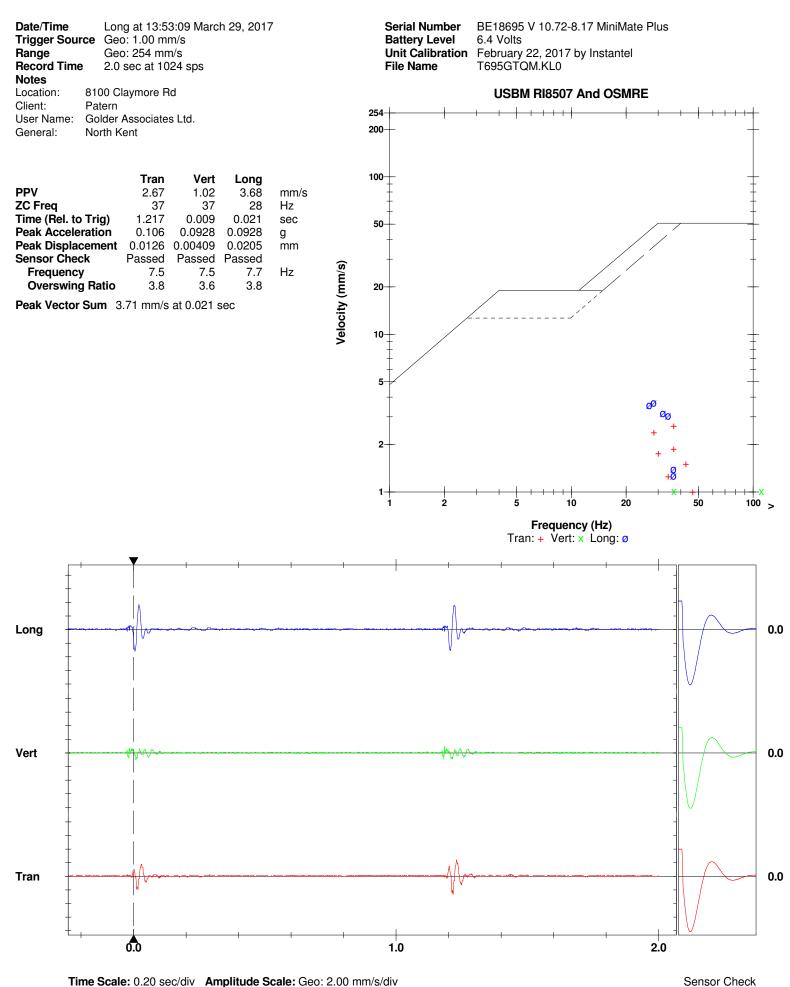
Patern

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQM.KI0

User Name: Golder Associates Ltd. General: North Kent Tran Dominant Frequency = 37.5 Hz., Amplitude = 0.0403, PPV from Event = 1.14 mm/s 0.06 0.04 0.040.02



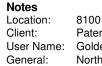




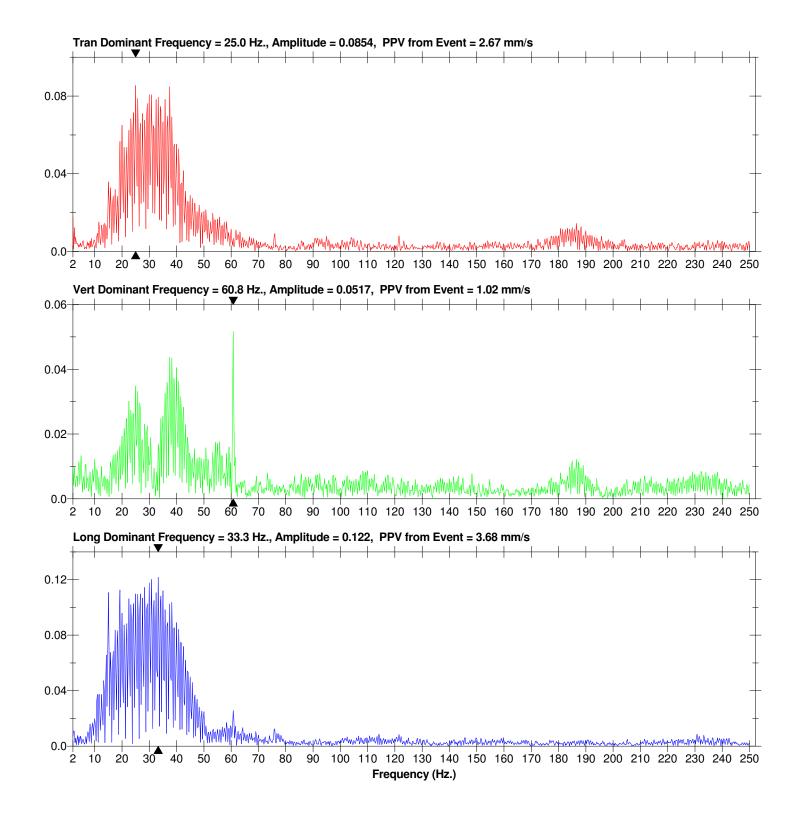


Long at 13:53:09 March 29, 2017 Date/Time Trigger Source Geo: 1.00 mm/s Range Geo: 254 mm/s **Record Time** 2.0 sec at 1024 sps

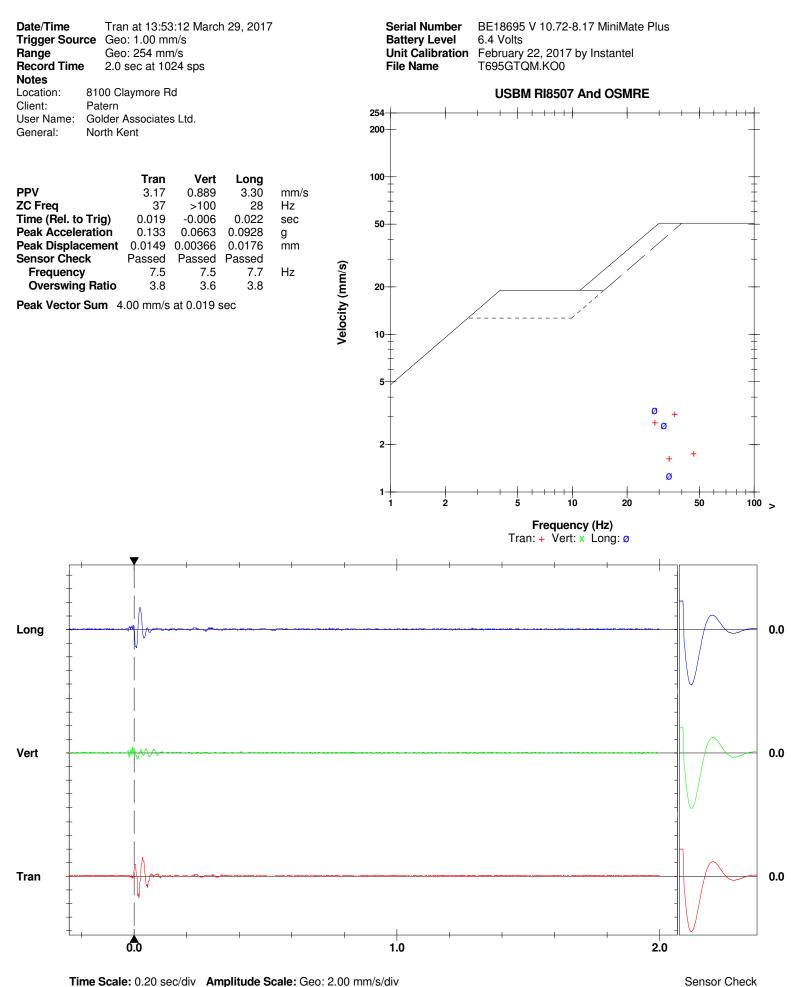
Serial Number BE18695 V 10.72-8.17 MiniMate Plus **Battery Level** 6.4 Volts Unit Calibration February 22, 2017 by Instantel File Name T695GTQM.KL0



8100 Claymore Rd Patern Golder Associates Ltd. North Kent









 Date/Time
 Tran at 13:53:12 March 29, 2017

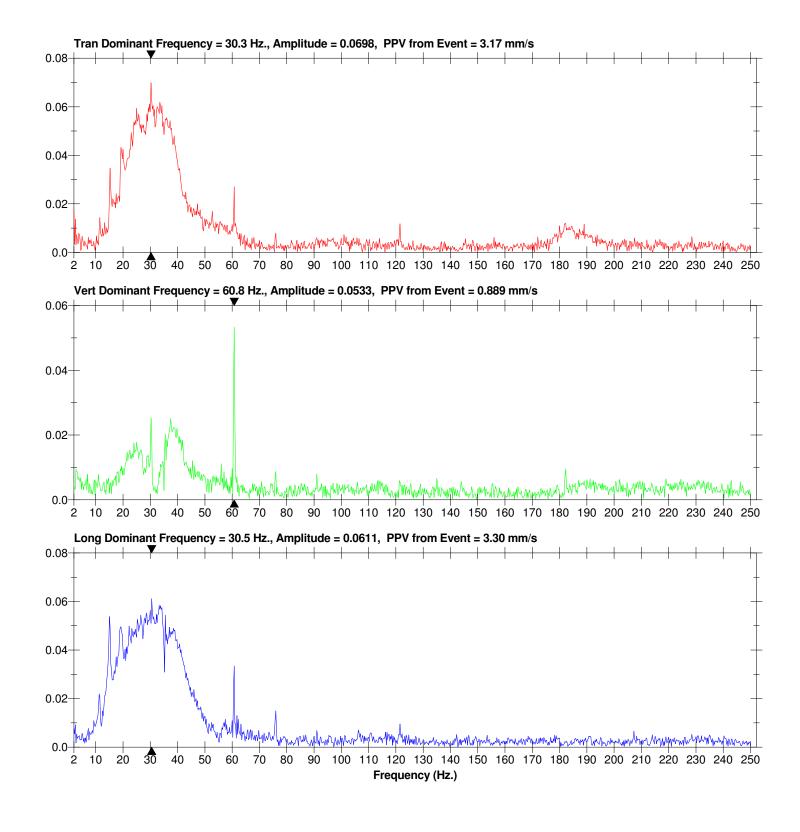
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

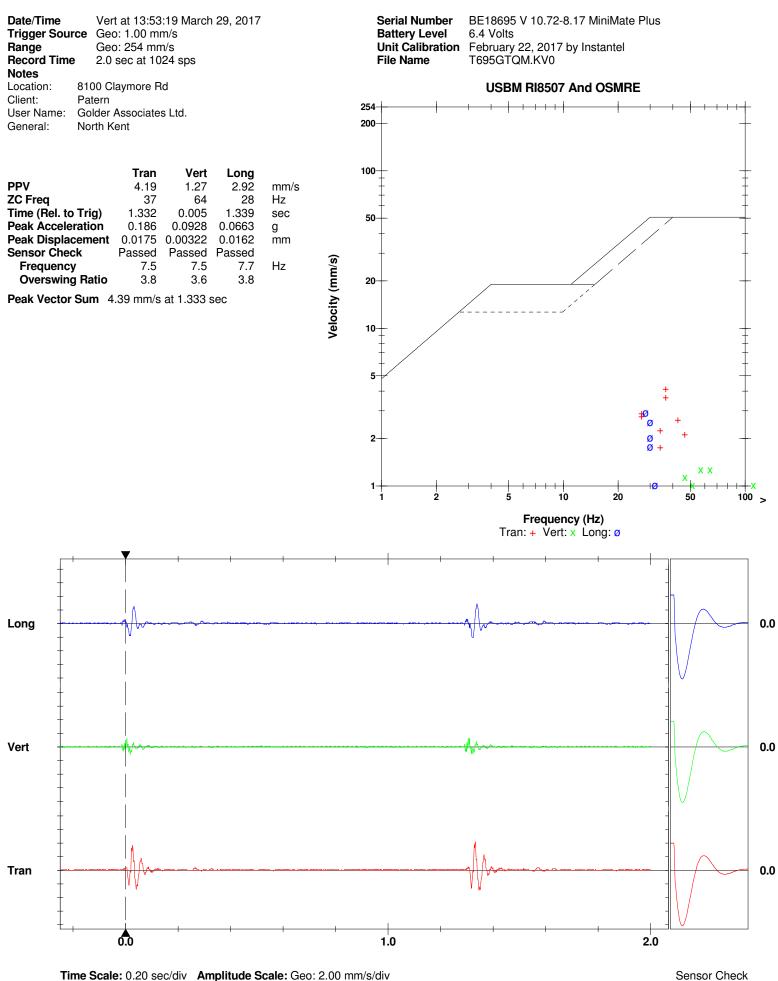
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQM.KO0









Trigger = 🕨 Printed: April 24, 2017 (V 10.60 - 10.60) Sensor Check



 Date/Time
 Vert at 13:53:19 March 29, 2017

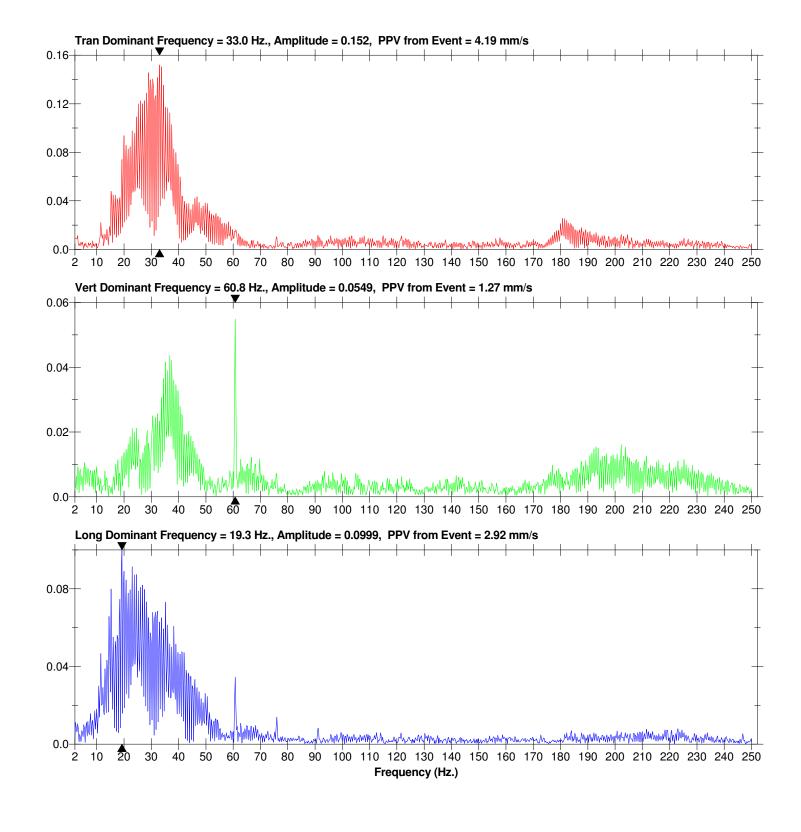
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

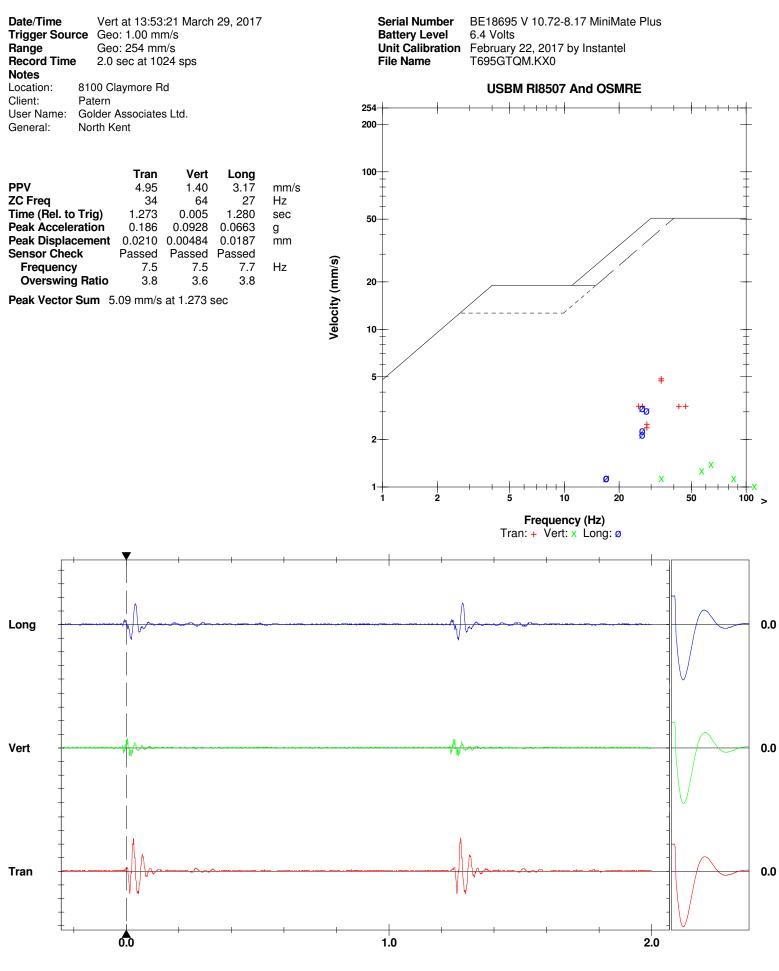
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQM.KV0

Notes









 Date/Time
 Vert at 13:53:21 March 29, 2017

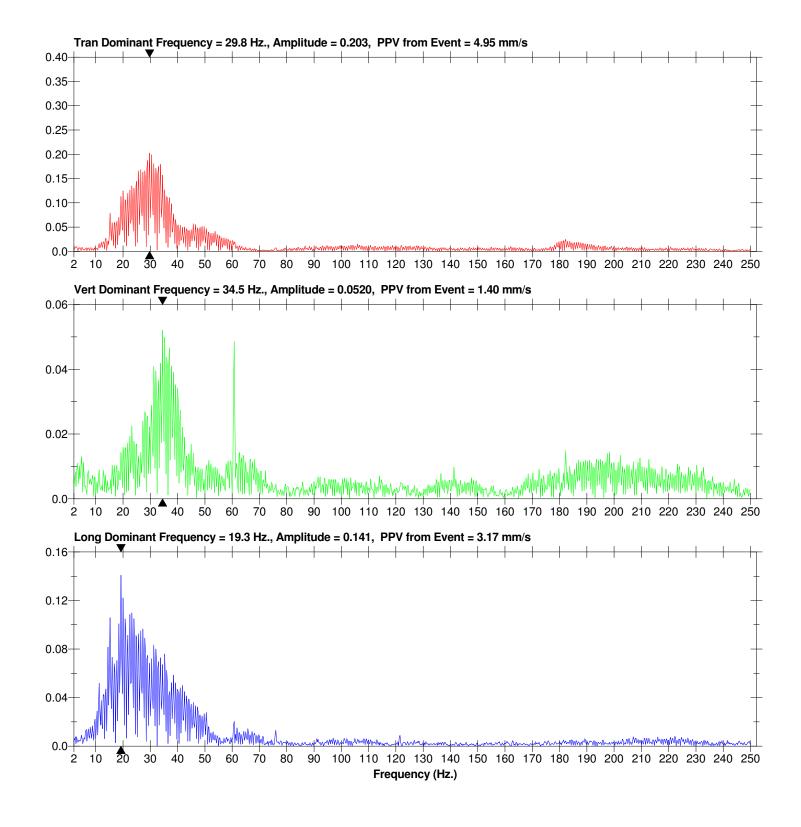
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

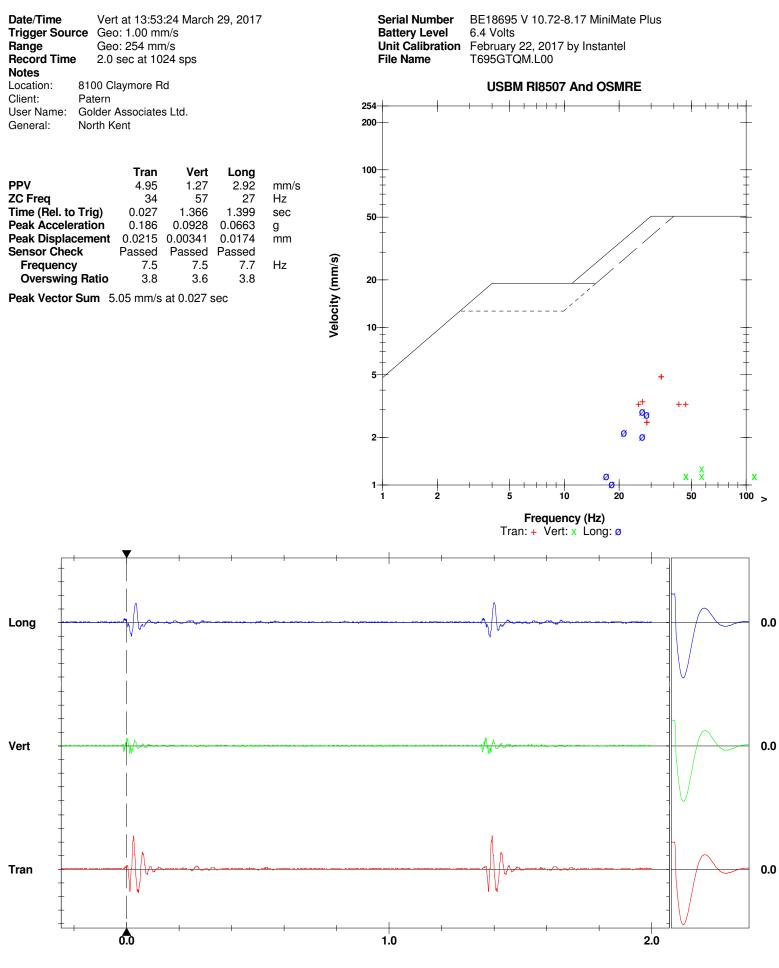
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQM.KX0

Notes







Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger =



 Date/Time
 Vert at 13:53:24 March 29, 2017

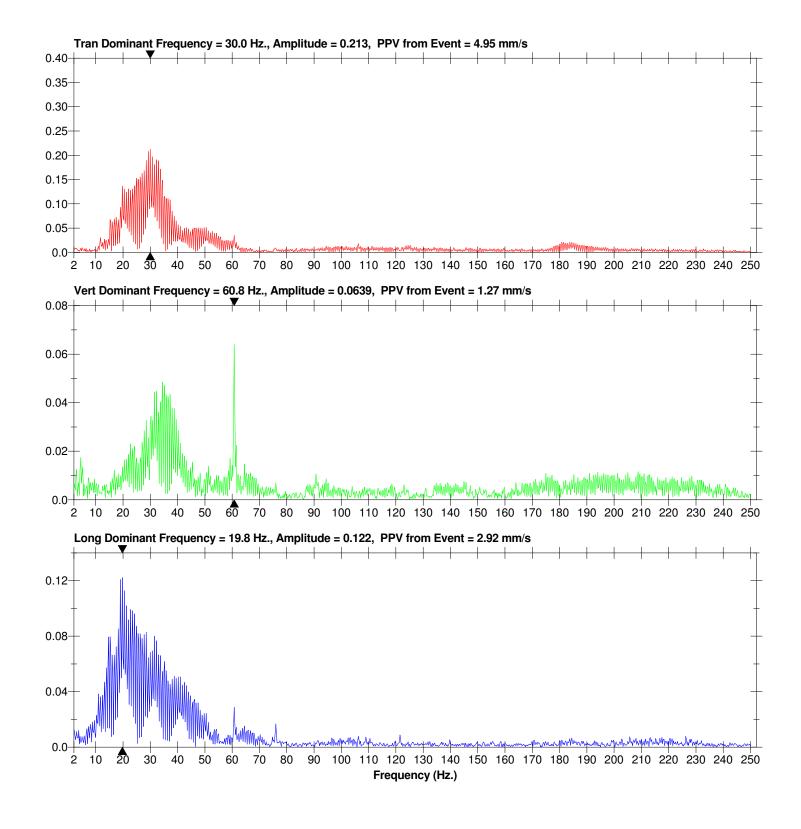
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

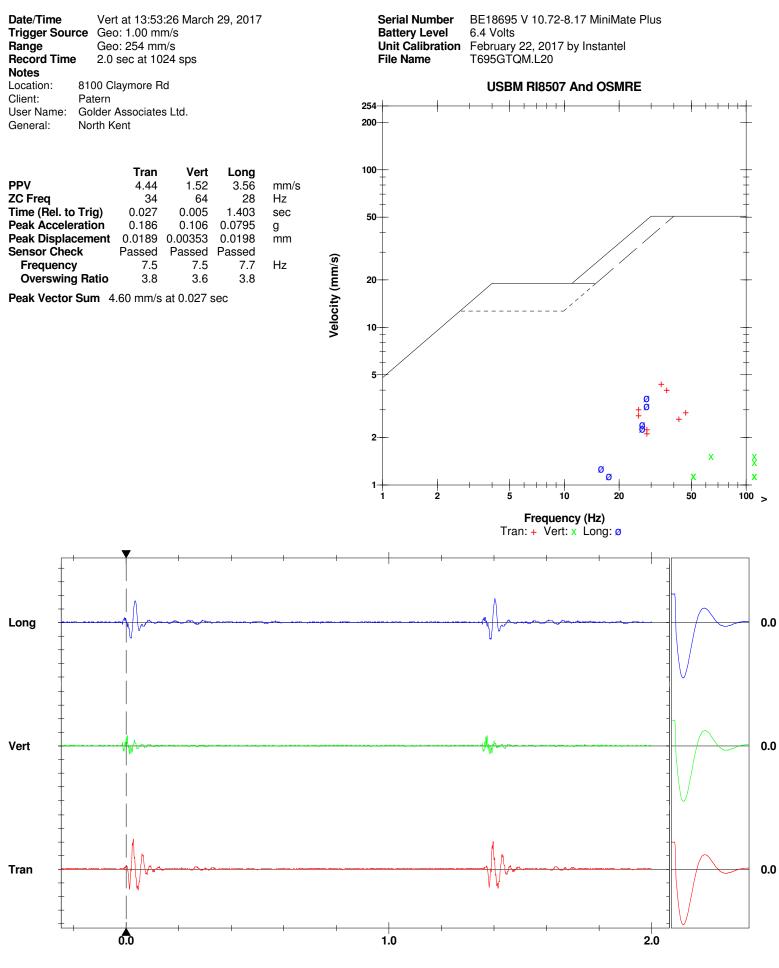
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQM.L00

Notes







Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger =



 Date/Time
 Vert at 13:53:26 March 29, 2017

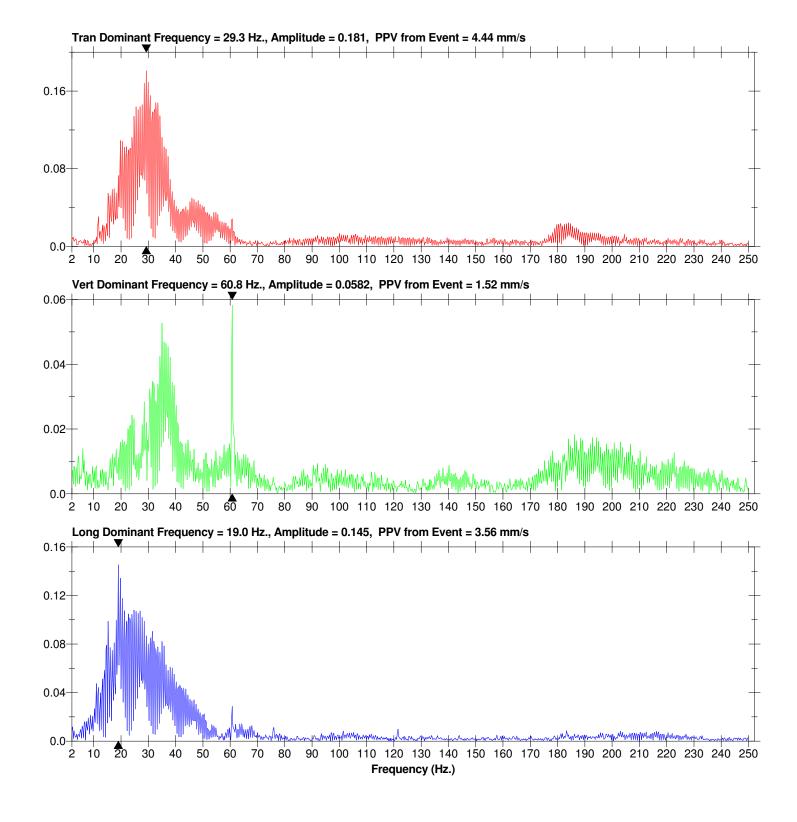
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

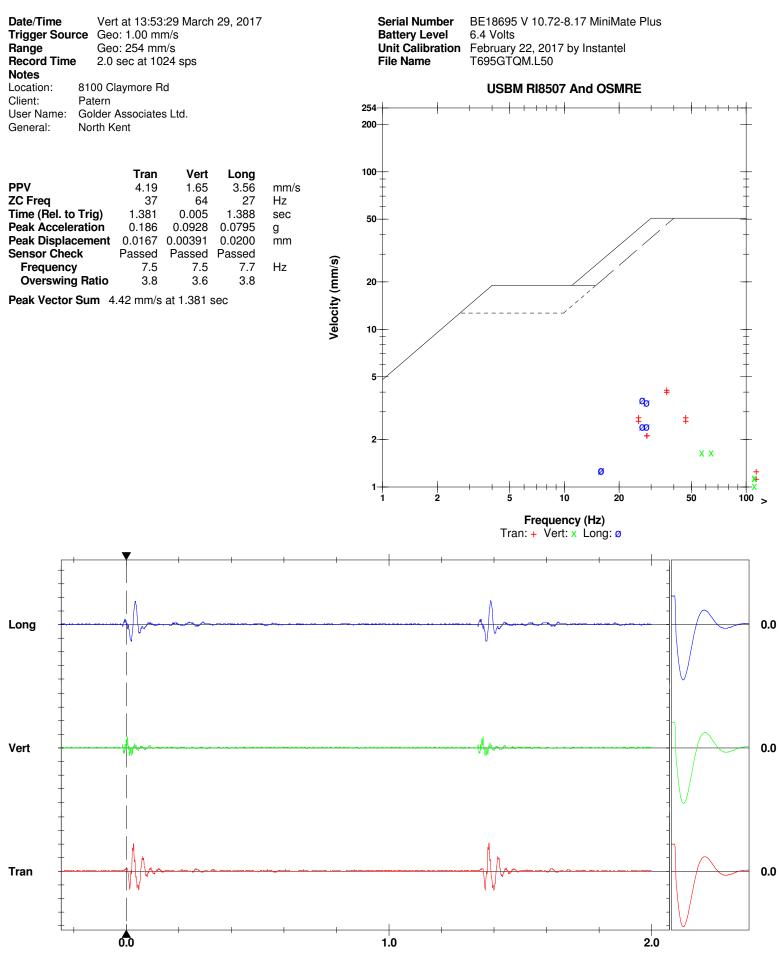
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQM.L20









Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger =



 Date/Time
 Vert at 13:53:29 March 29, 2017

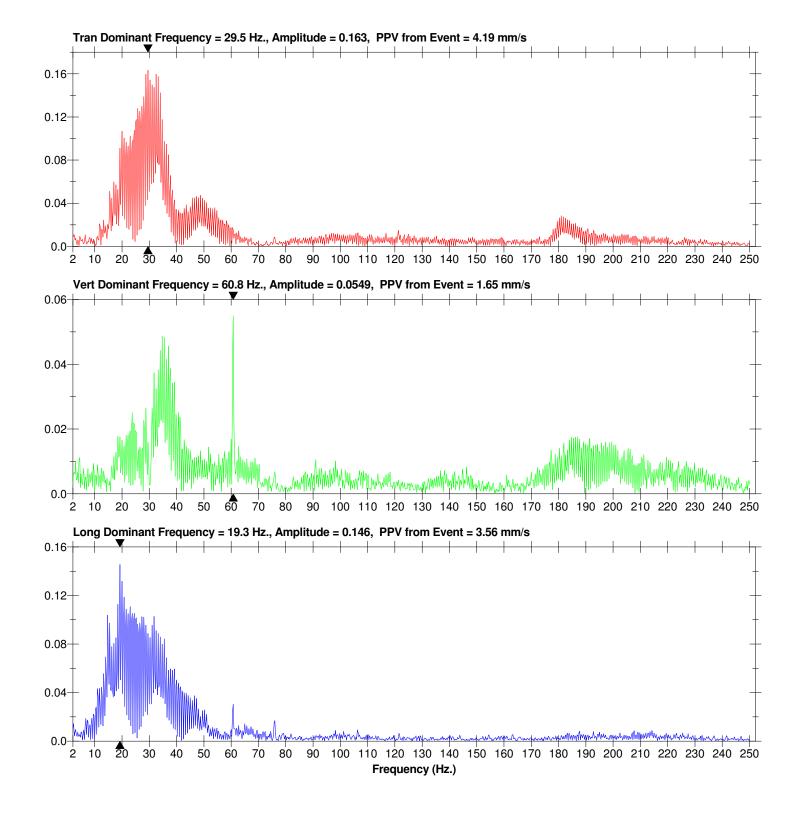
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTQM.L50





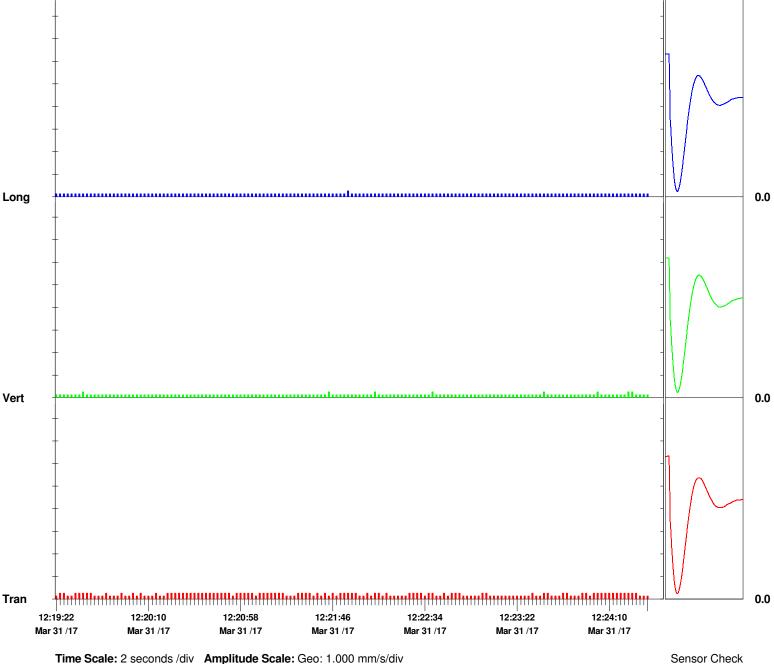


Histogram S Histogram F Number of Ir Range Sample Rate Notes	inish Time ntervals	12:19:20 March 31, 2017 12:24:30 March 31, 2017 155.00 at 2 seconds Geo:254 mm/s 1024sps
Location:	8100 Claymore Rd	
Client:	Patern	
User Name:	Golder Associates Ltd.	
General:	North Kent	

Serial Number BE18695 V 10.72-8.17 MiniMate Plus Battery Level Unit Calibration 6.3 Volts February 22, 2017 by Instantel File Name T695GTU7.K80

	Tran	Vert	Long	
PPV	0.254	0.254	0.254	mm/s
ZC Freq	>100	>100	>100	Hz
Date	Mar 31 /17	Mar 31 /17	Mar 31 /17	
Time	12:19:24	12:19:36	12:21:54	
Sensor Check	Passed	Passed	Passed	
Frequency	7.5	7.4	7.6	Hz
Overswing Ratio	3.9	3.7	3.9	

Peak Vector Sum 0.311 mm/s on March 31, 2017 at 12:19:32



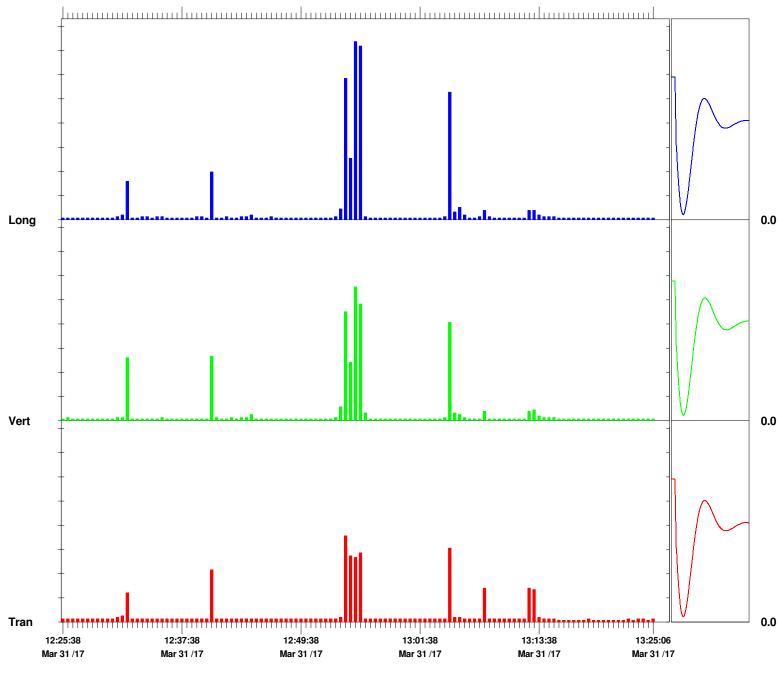


Histogram S Histogram F Number of Ir Range Sample Rate Notes	inish Time ntervals	12:25:08 March 31, 2017 13:25:06 March 31, 2017 1799.00 at 2 seconds Geo:254 mm/s 1024sps
Location: Client: User Name: General:	8100 Claym Patern Golder Asse North Kent	

Serial Number
Battery LevelBE18695 V 10.72-8.17 MiniMate Plus
6.3 VoltsUnit Calibration
File NameFebruary 22, 2017 by Instantel
T695GTU7.TW0

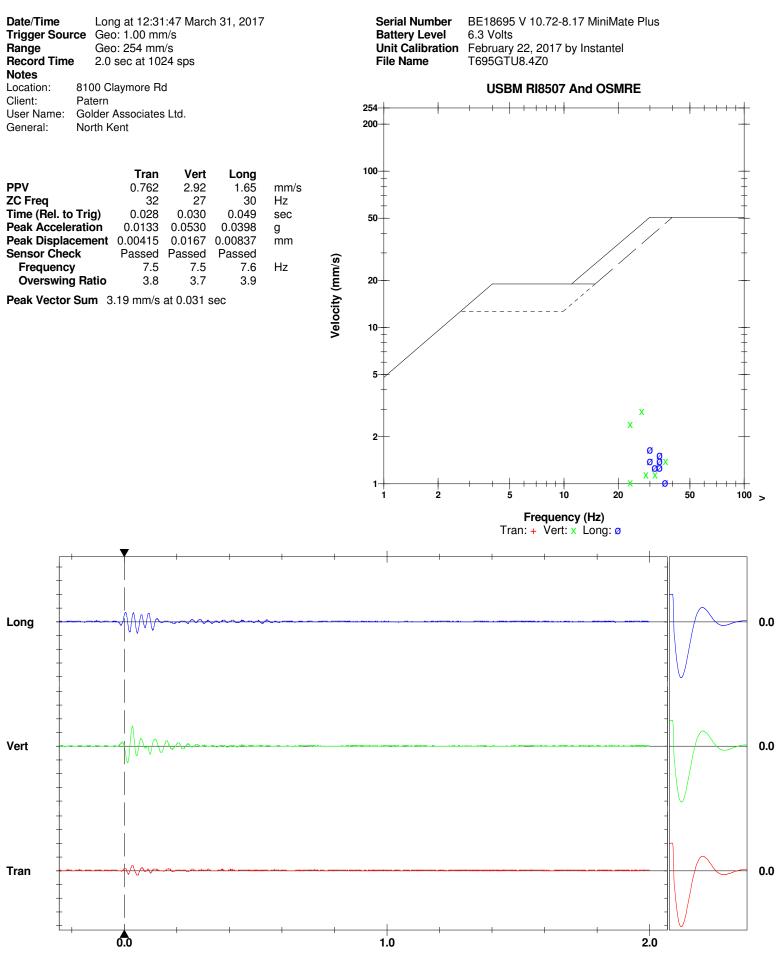
	Tran	Vert	Long	
PPV	7.11	11.0	14.7	mm/s
ZC Freq	32	28	32	Hz
Date	Mar 31 /17	Mar 31 /17	Mar 31 /17	
Time	12:53:42	12:55:04	12:55:04	
Sensor Check	Passed	Passed	Passed	
Frequency	7.5	7.5	7.6	Hz
Overswing Ratio	3.8	3.7	3.9	

Peak Vector Sum 17.0 mm/s on March 31, 2017 at 12:55:04



Time Scale: 30 seconds /div Amplitude Scale: Geo: 2.00 mm/s/div





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger =

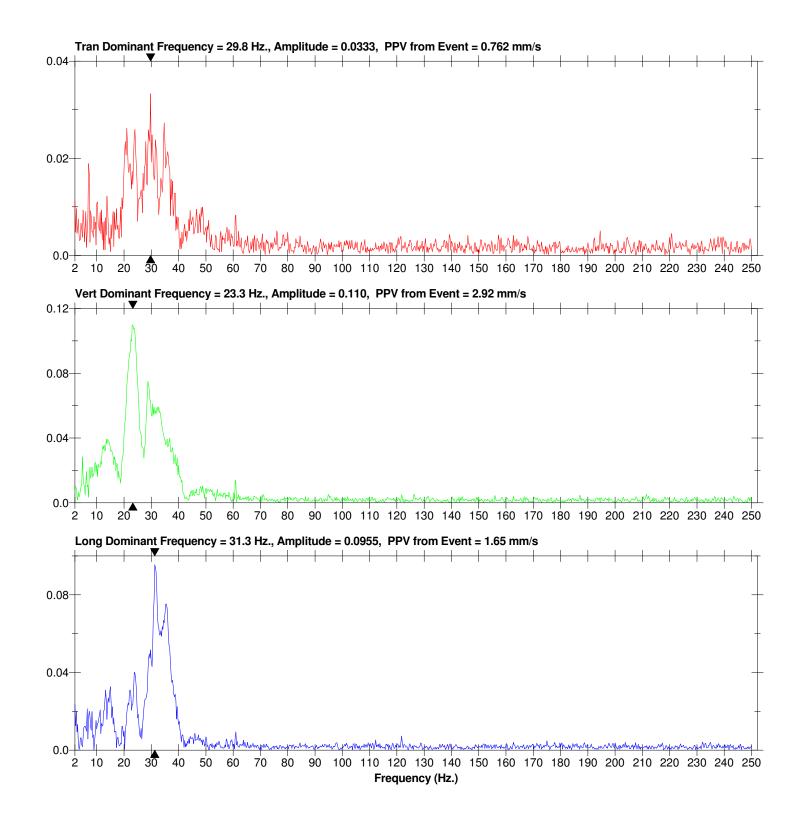


Long at 12:31:47 March 31, 2017 Date/Time Trigger Source Geo: 1.00 mm/s Range Geo: 254 mm/s **Record Time** 2.0 sec at 1024 sps

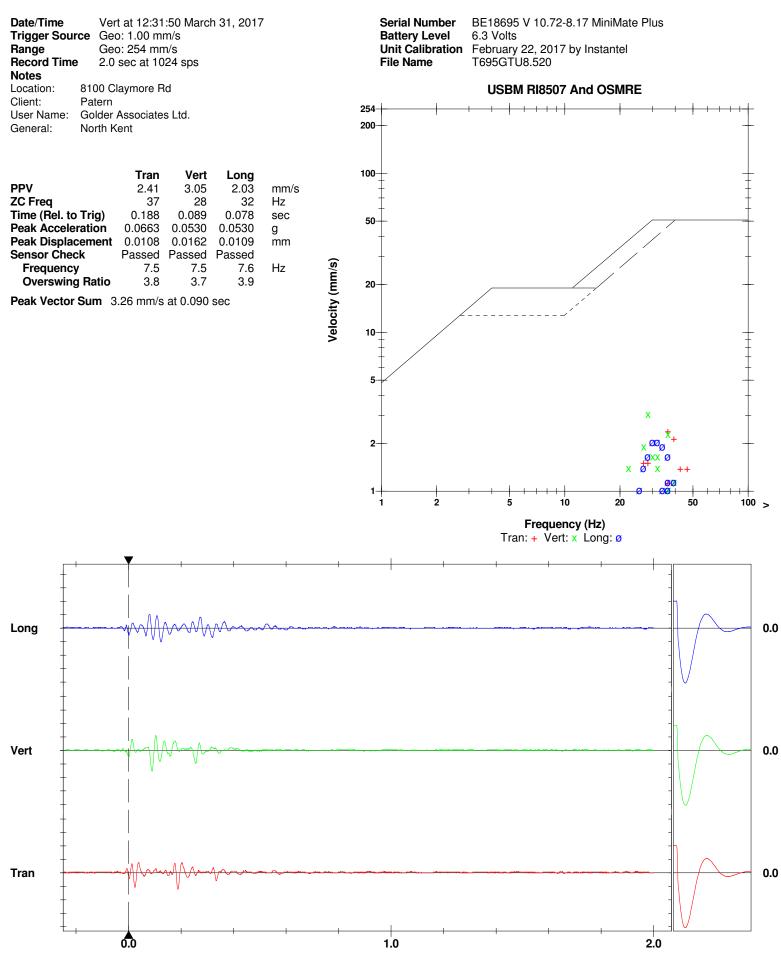
Serial Number BE18695 V 10.72-8.17 MiniMate Plus **Battery Level** 6.3 Volts Unit Calibration February 22, 2017 by Instantel File Name T695GTU8.4Z0

Notes Location: Client: Patern User Name: General:

8100 Claymore Rd Golder Associates Ltd. North Kent







Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger =



 Date/Time
 Vert at 12:31:50 March 31, 2017

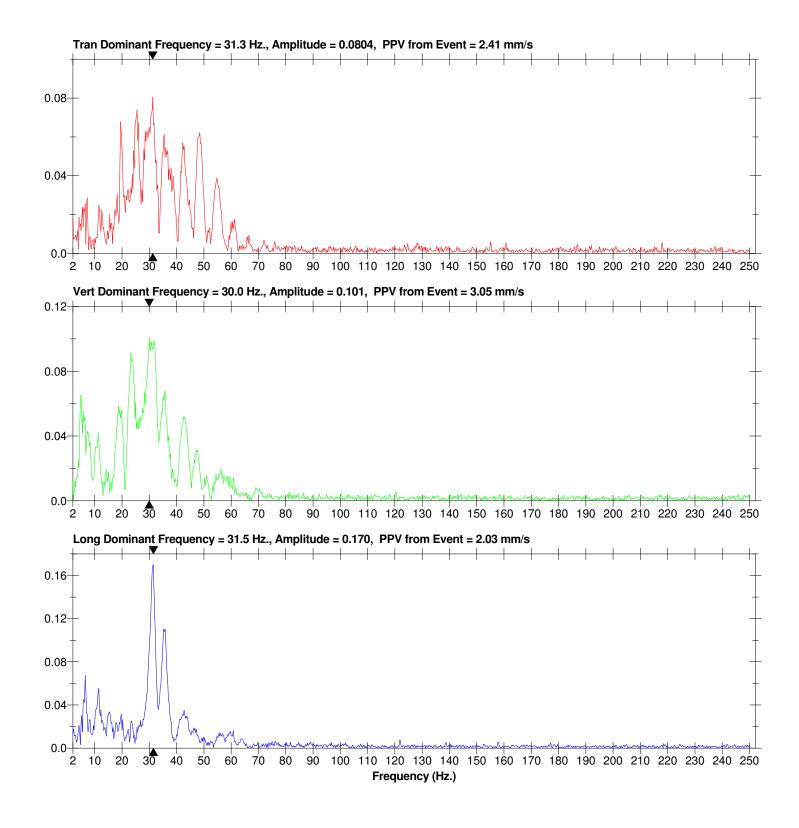
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

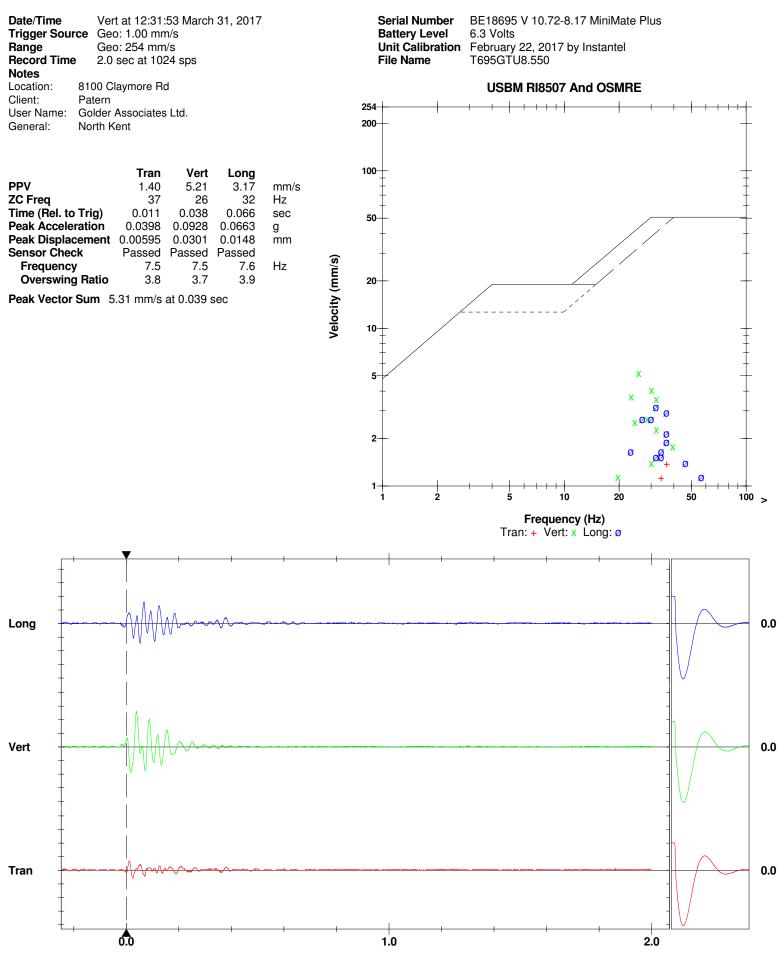
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTU8.520

Notes







Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger = ▶── ──◀



 Date/Time
 Vert at 12:31:53 March 31, 2017

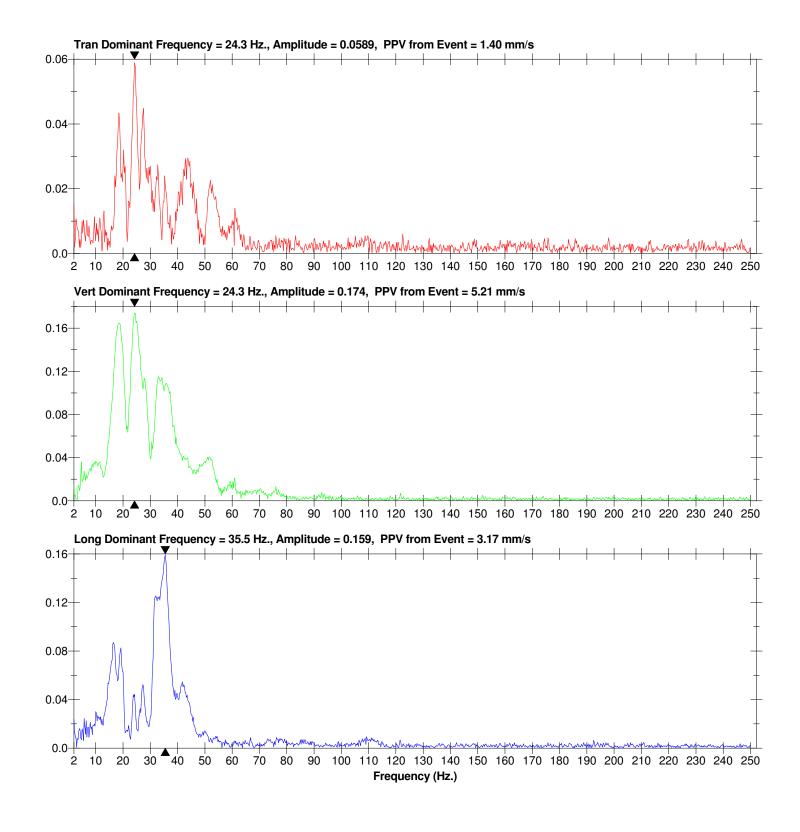
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

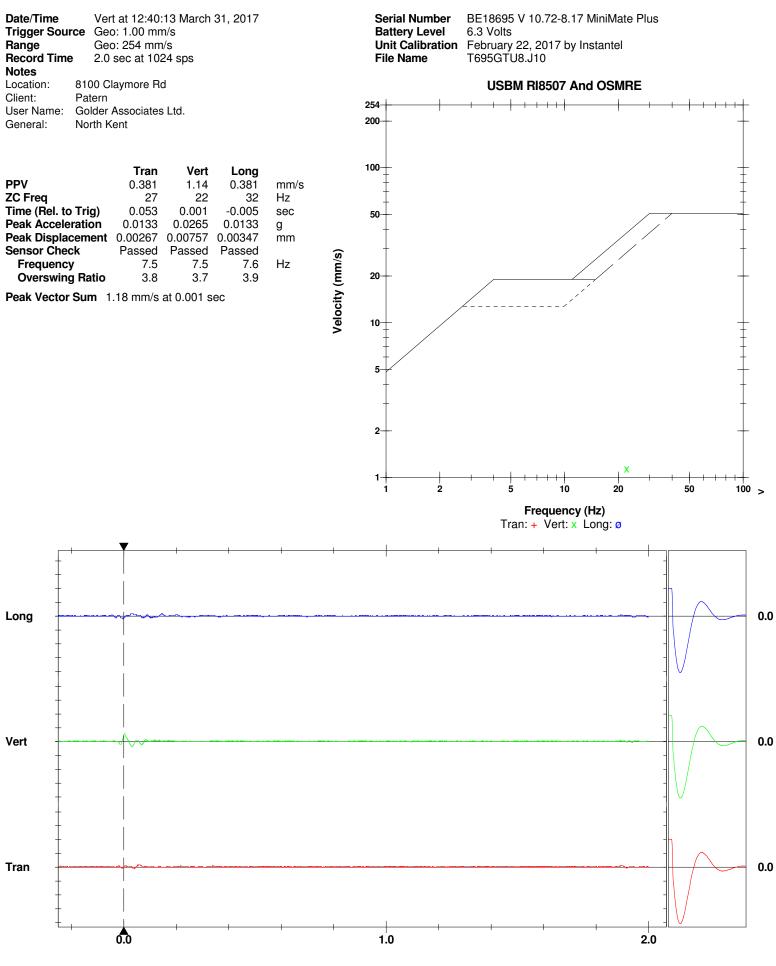
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTU8.550









Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger = > _____ ___



 Date/Time
 Vert at 12:40:13 March 31, 2017

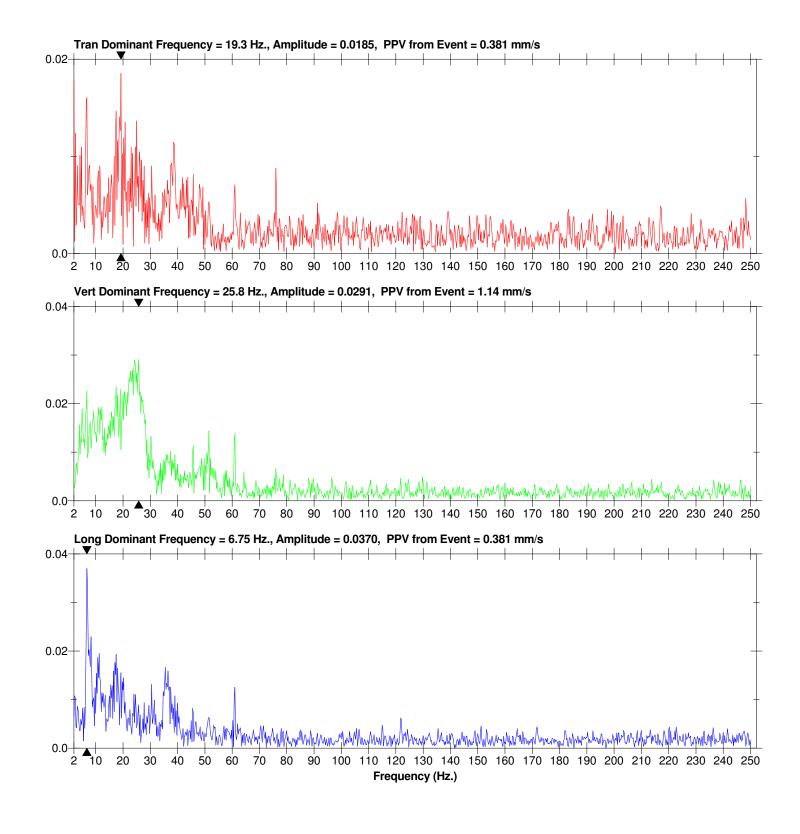
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

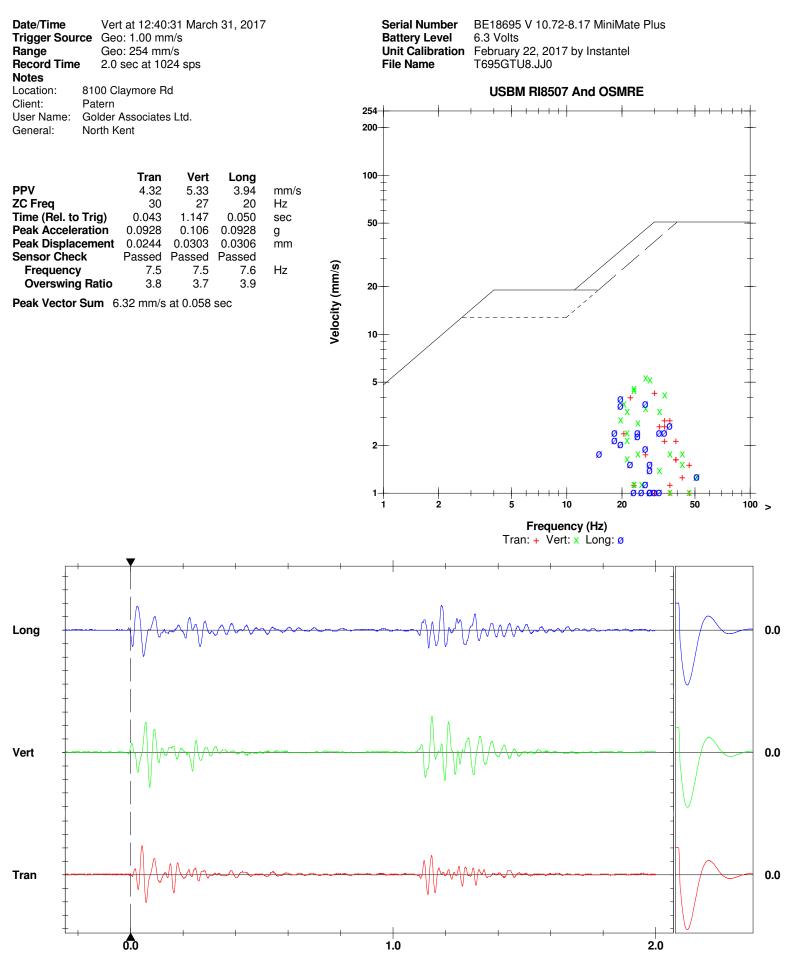
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTU8.J10









Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger = ► ____ ←



 Date/Time
 Vert at 12:40:31 March 31, 2017

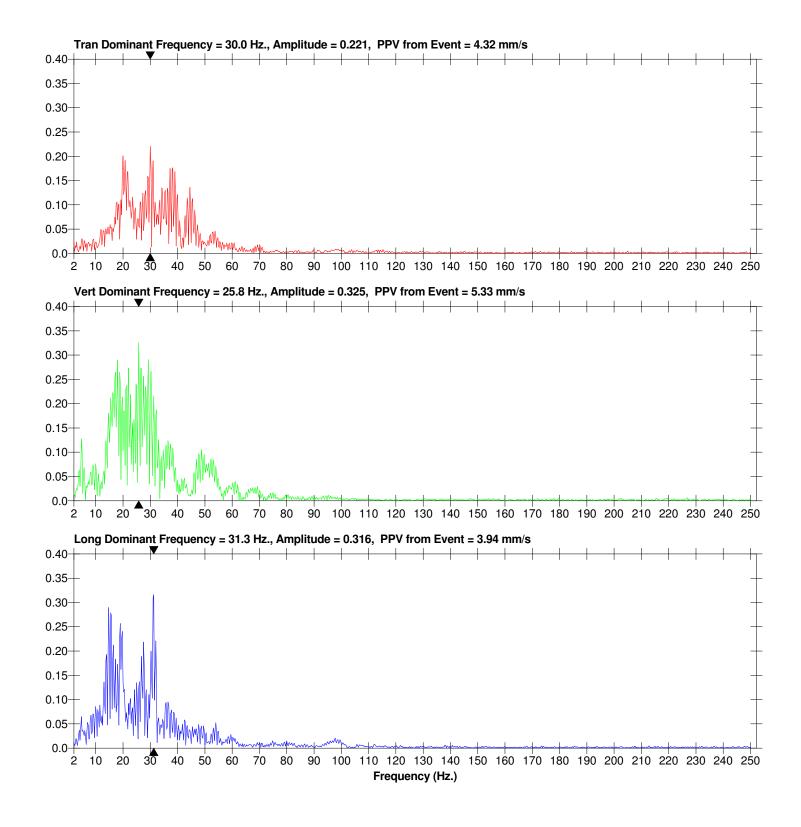
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

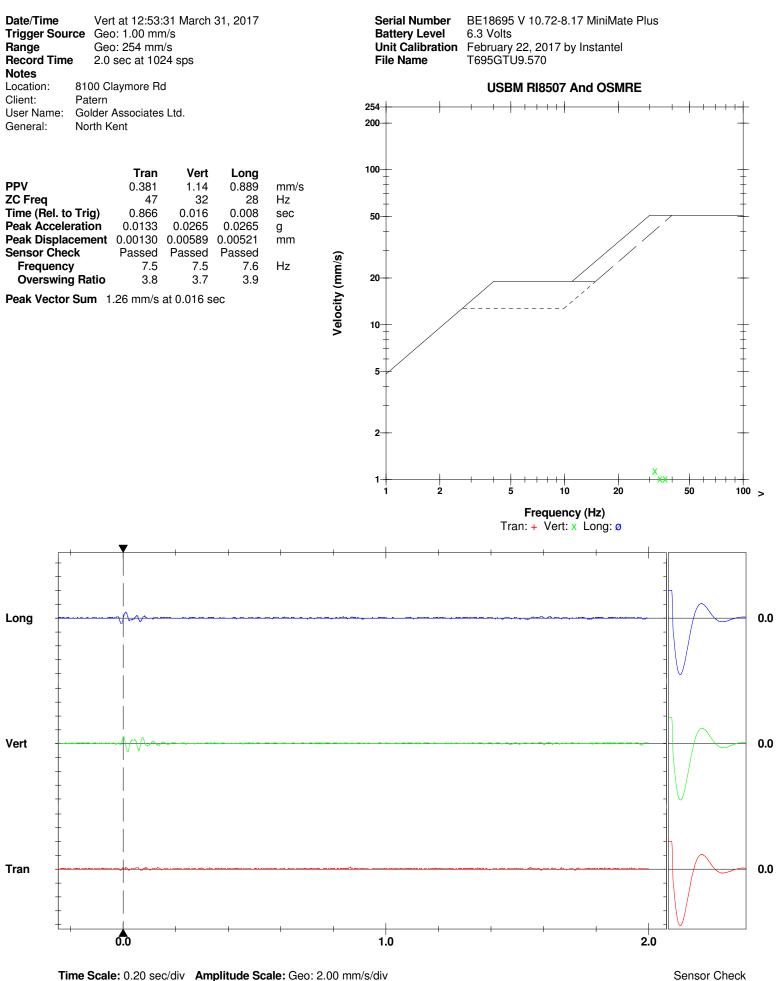
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTU8.JJ0

Notes







Trigger = 🕨 Printed: April 24, 2017 (V 10.60 - 10.60) Sensor Check



 Date/Time
 Vert at 12:53:31 March 31, 2017

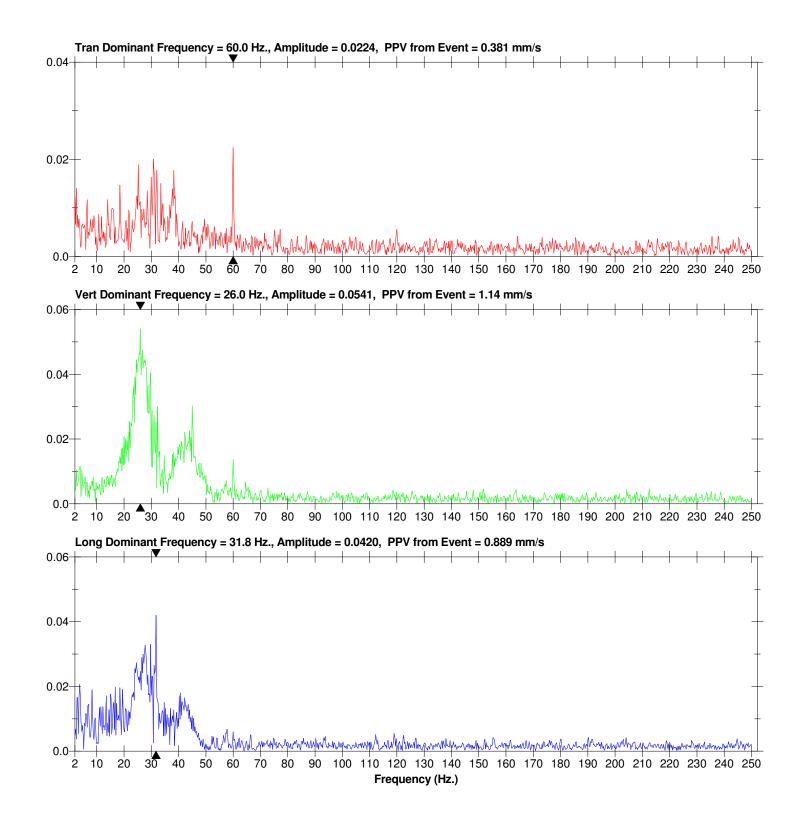
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

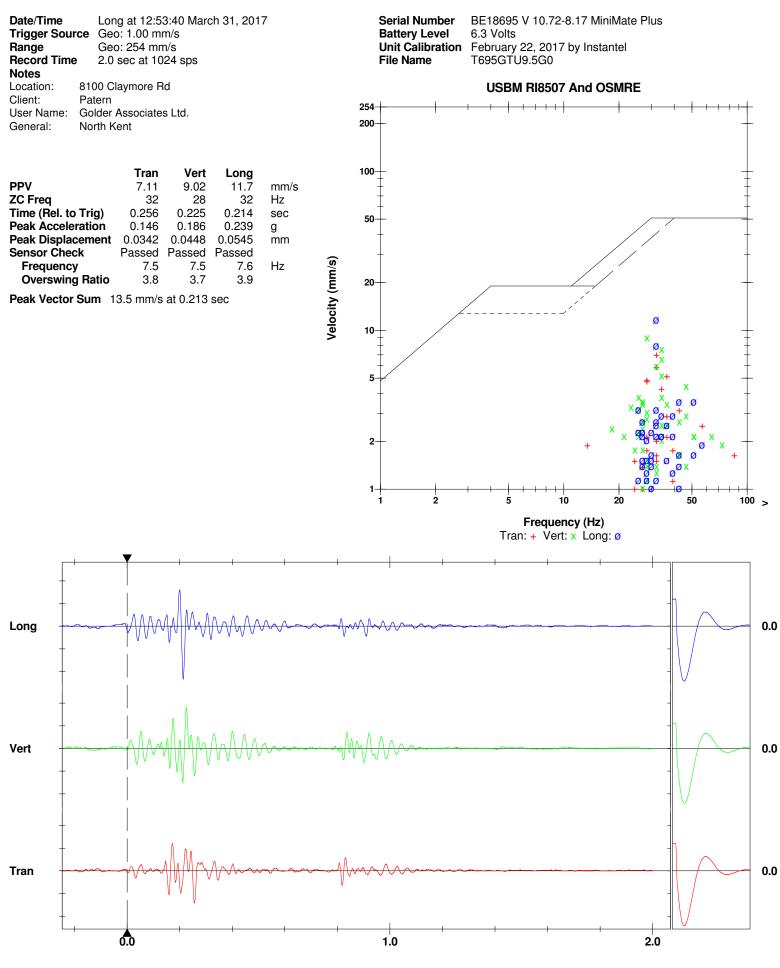
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTU9.570

Notes







Time Scale: 0.20 sec/div Amplitude Scale: Geo: 5.00 mm/s/div Trigger = ► ____



 Date/Time
 Long at 12:53:40 March 31, 2017

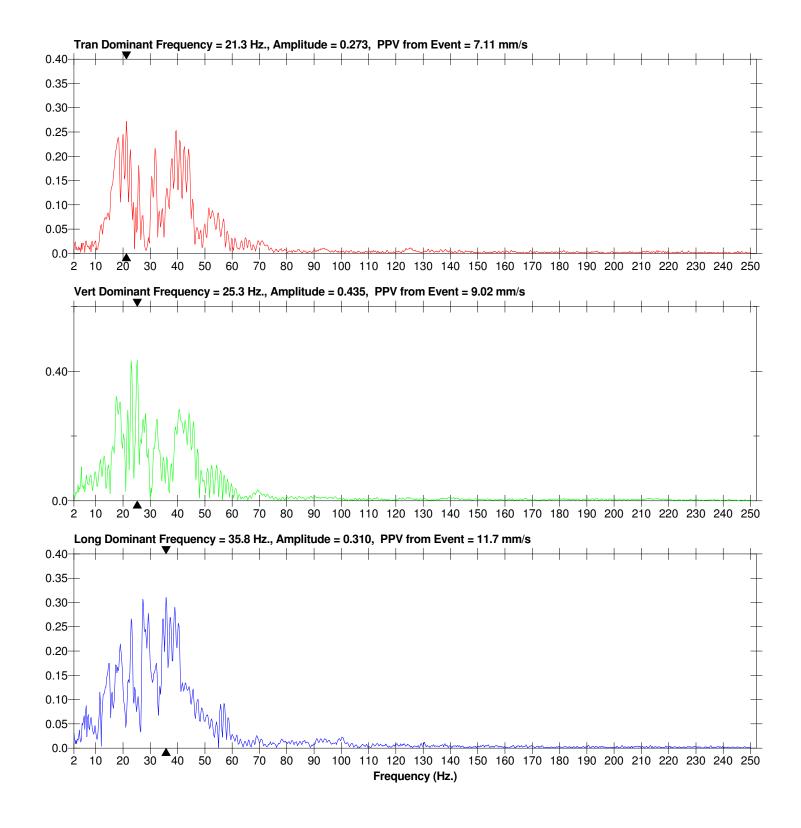
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

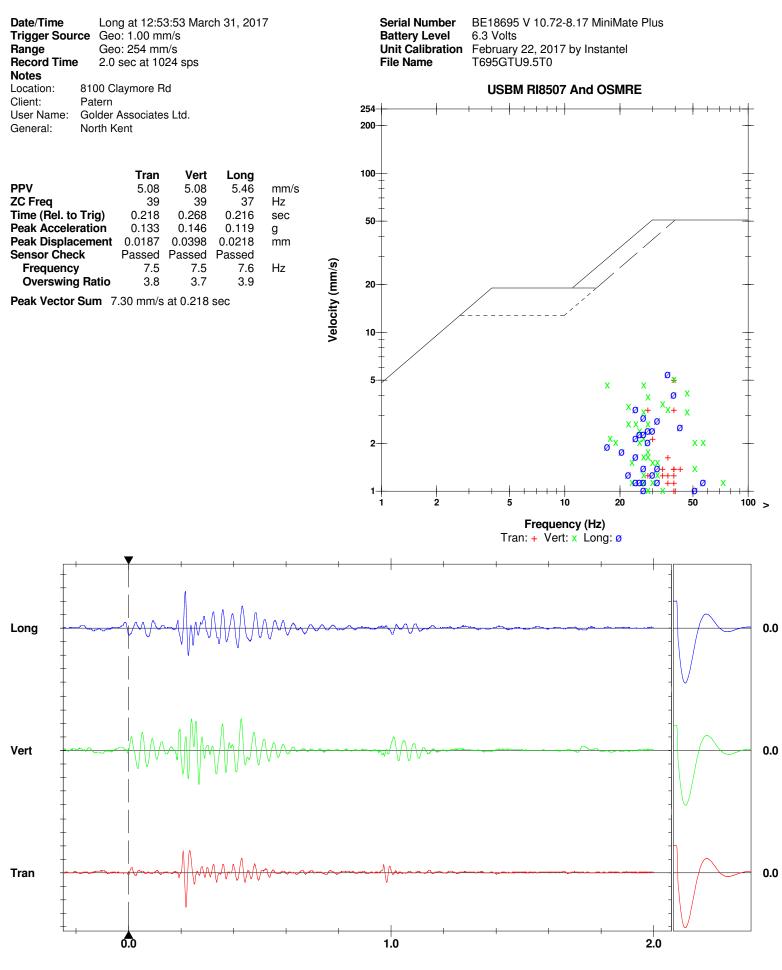
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTU9.5G0

Notes







Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger = ► ____



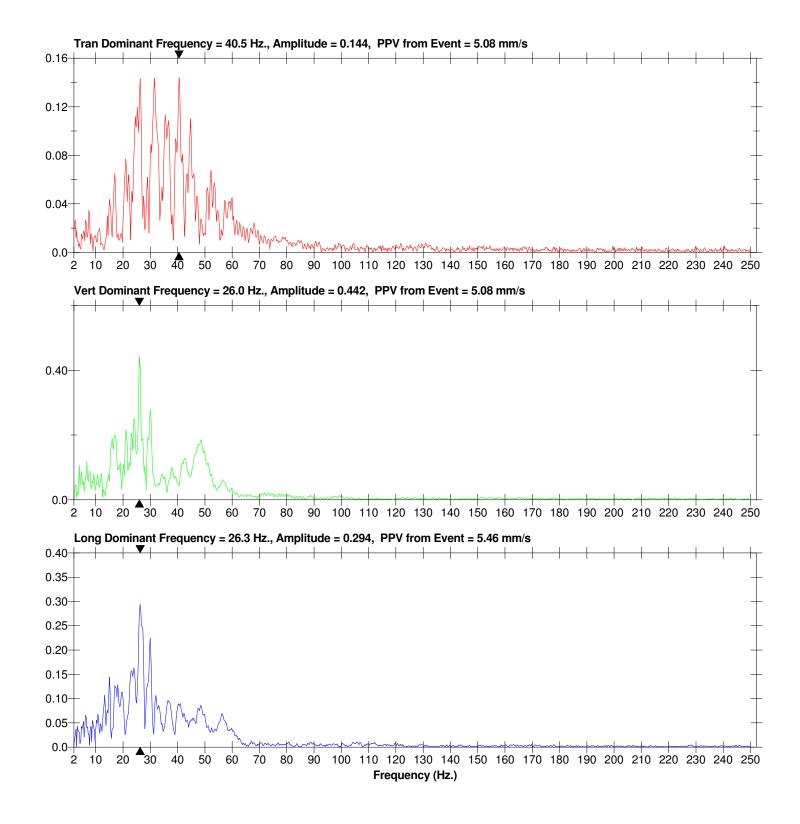
 Date/Time
 Long at 12:53:53 March 31, 2017

 Trigger Source
 Geo: 1.00 mm/s

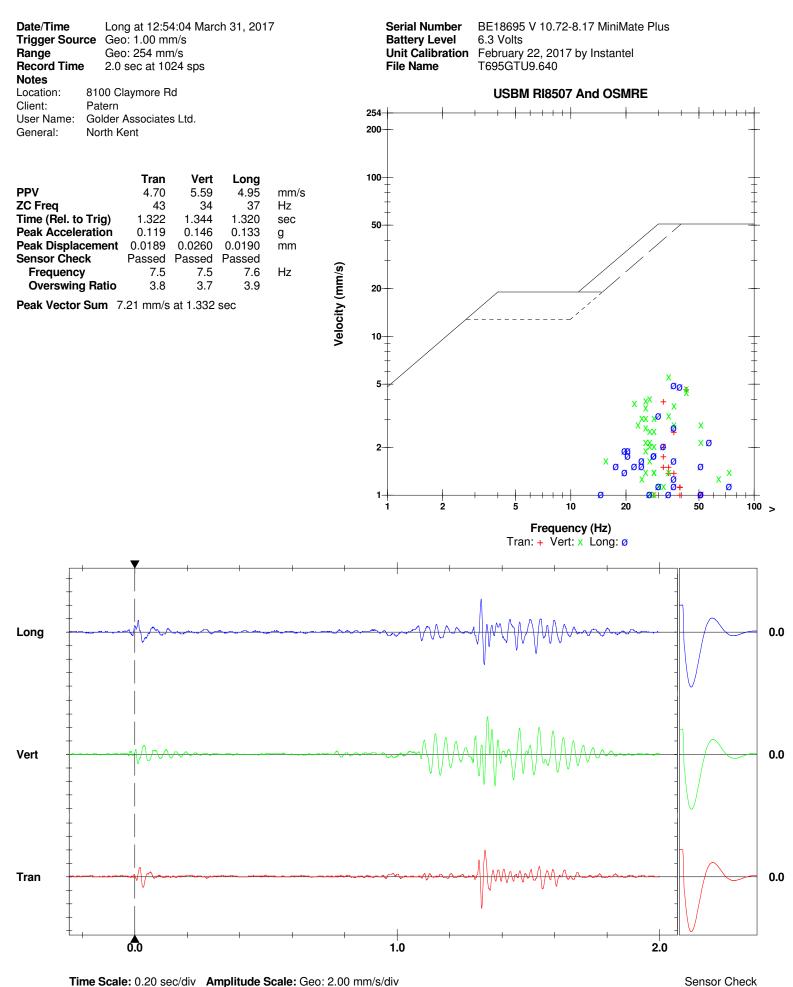
 Range
 Geo: 254 mm/s

 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTU9.5T0







Trigger =



Notes

FFT Report

 Date/Time
 Long at 12:54:04 March 31, 2017

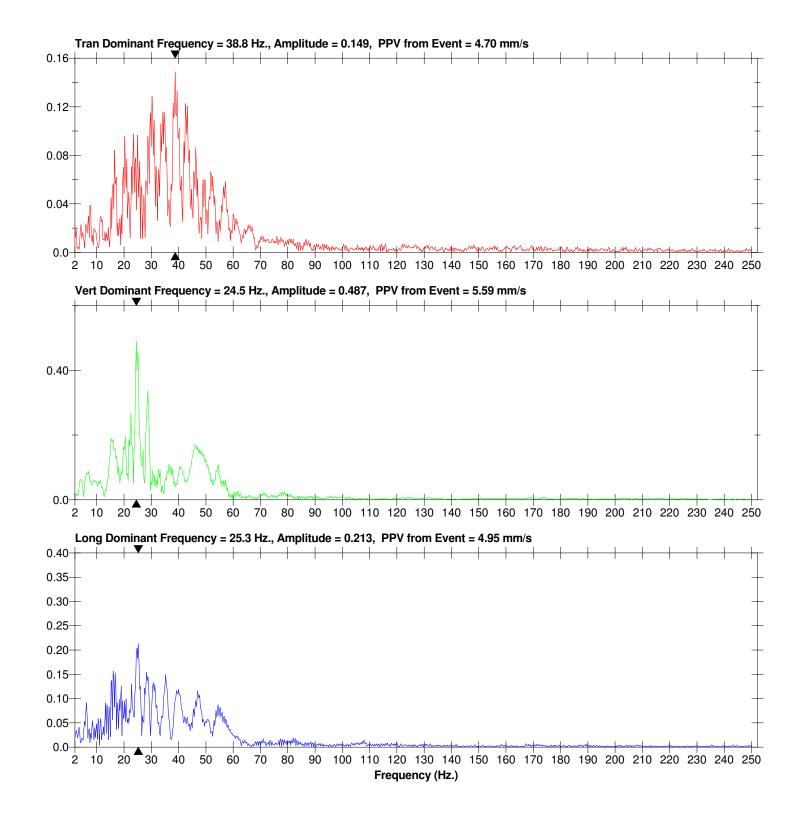
 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

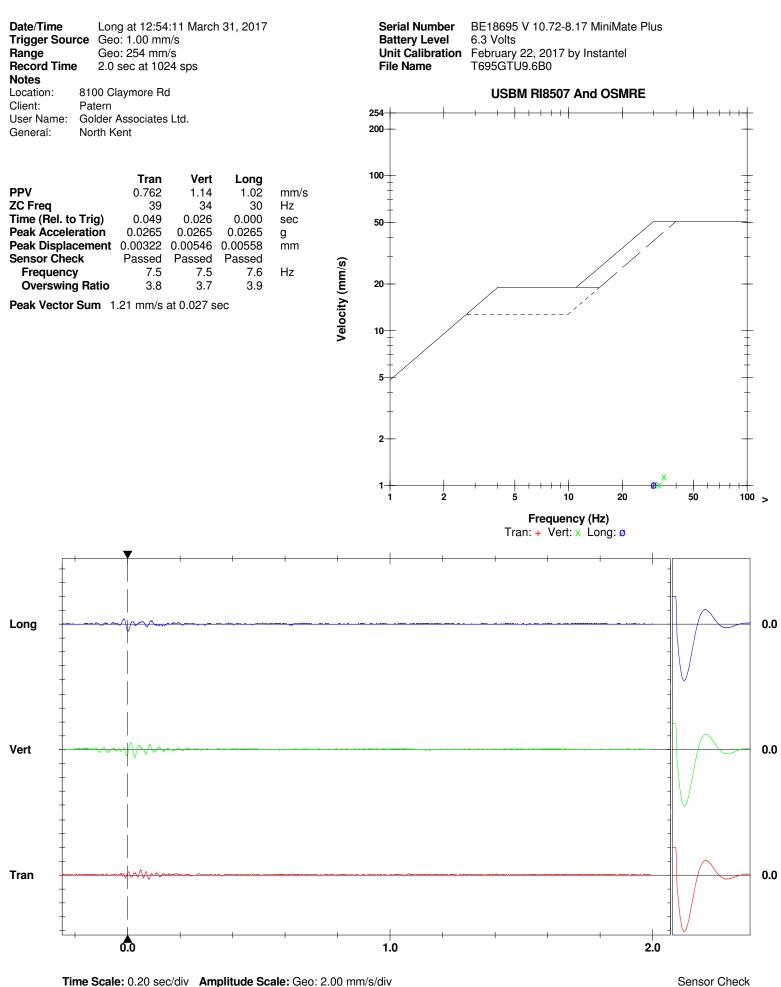
 Record Time
 2.0 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTU9.640

Location: 8100 Claymore Rd Client: Patern User Name: Golder Associates Ltd. General: North Kent







Trigger = ► _____ Printed: April 24, 2017 (V 10.60 - 10.60)



Notes

Location: Client:

General:

User Name:

FFT Report

 Date/Time
 Long at 12:54:11 March 31, 2017

 Trigger Source
 Geo: 1.00 mm/s

 Range
 Geo: 254 mm/s

 Record Time
 2.0 sec at 1024 sps

8100 Claymore Rd

Golder Associates Ltd.

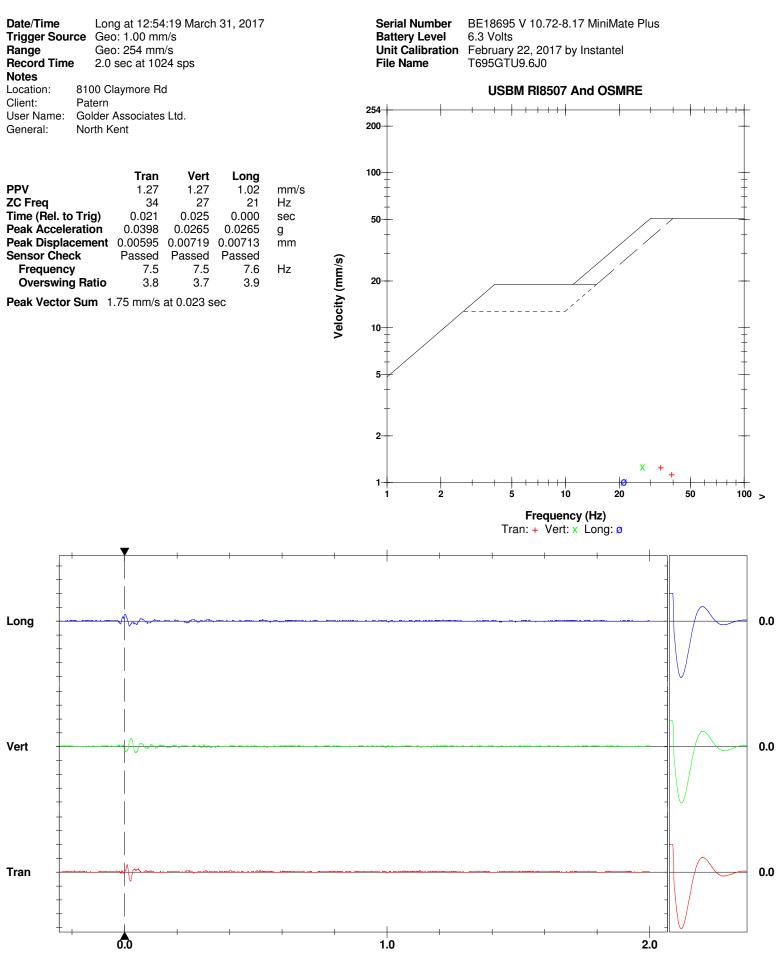
Patern

North Kent

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTU9.6B0

Tran Dominant Frequency = 44.8 Hz., Amplitude = 0.0309, PPV from Event = 0.762 mm/s • 0.04 0.02 0.0 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 2 10 20 30 40 50 60 70 80 90 Vert Dominant Frequency = 28.0 Hz., Amplitude = 0.0582, PPV from Event = 1.14 mm/s 0.06-0.04 0.02 \mathcal{M} 0.0 40 50 60 70 80 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 2 10 20 30 Long Dominant Frequency = 27.8 Hz., Amplitude = 0.0351, PPV from Event = 1.02 mm/s ▼| 0.04+ _ 0.02 0.0 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 2 10 20 30 40 50 70 80 90 60 Frequency (Hz.)





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.00 mm/s/div Trigger = > _____

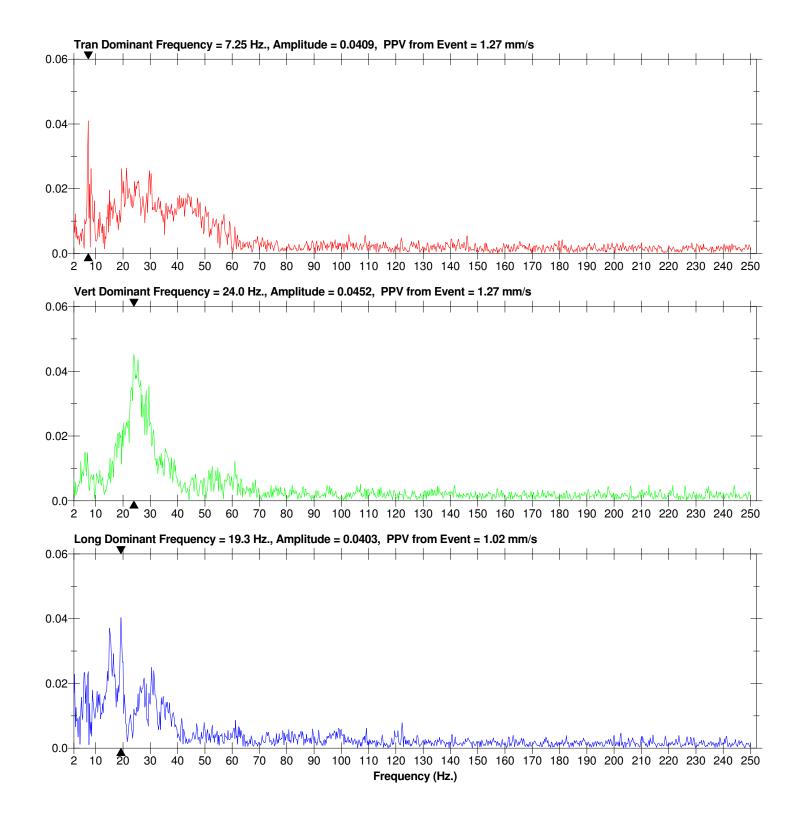


Long at 12:54:19 March 31, 2017 Date/Time Trigger Source Geo: 1.00 mm/s Range Geo: 254 mm/s **Record Time** 2.0 sec at 1024 sps

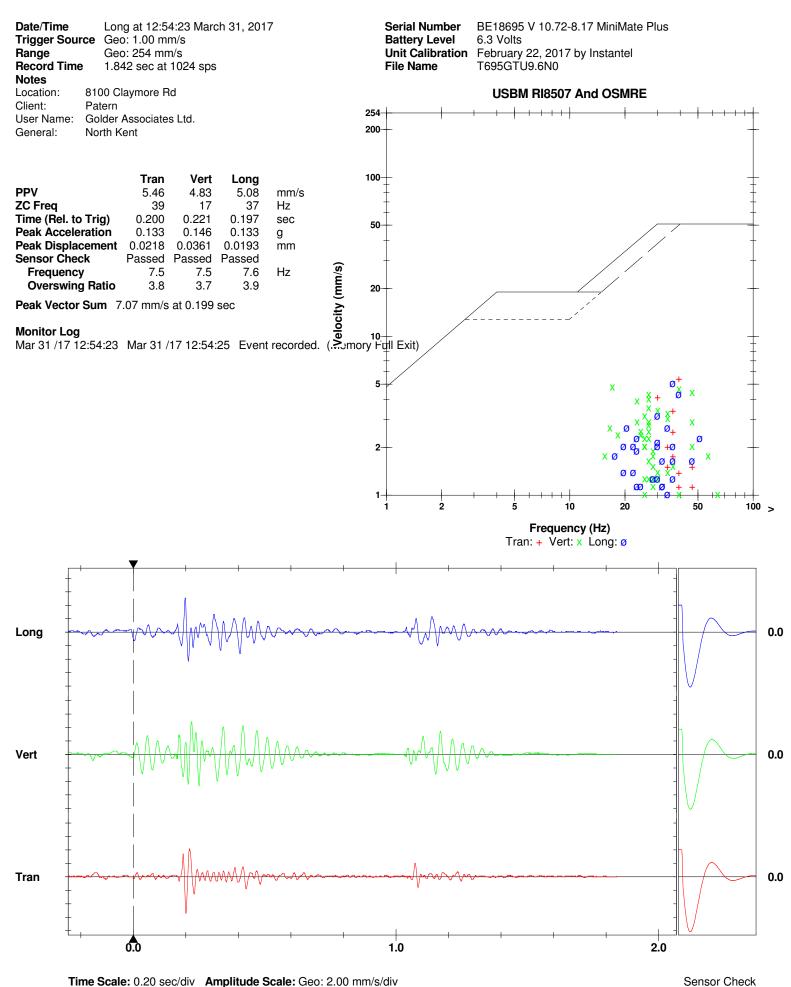
Serial Number BE18695 V 10.72-8.17 MiniMate Plus **Battery Level** 6.3 Volts Unit Calibration February 22, 2017 by Instantel File Name T695GTU9.6J0



8100 Claymore Rd Client: Patern User Name: Golder Associates Ltd. General: North Kent







Trigger = Printed: April 24, 2017 (V 10.60 - 10.60)

Format © 1995-2013 Xmark Corporation



 Date/Time
 Long at 12:54:23 March 31, 2017

 Trigger Source
 Geo: 1.00 mm/s

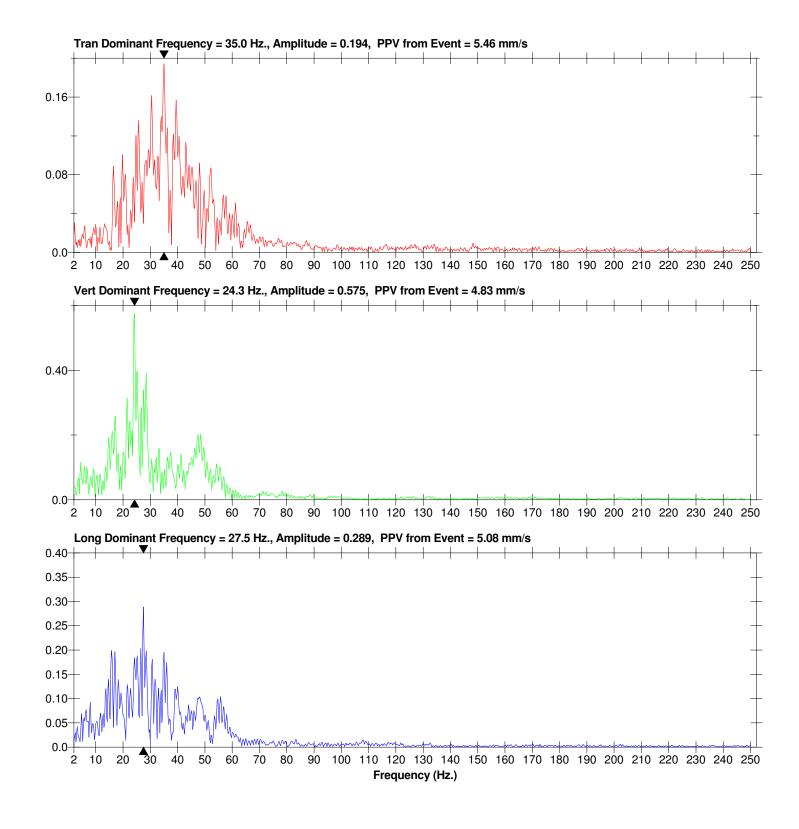
 Range
 Geo: 254 mm/s

 Record Time
 1.842 sec at 1024 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GTU9.6N0



Location: 8100 Claymore Rd Client: Patern User Name: Golder Associates Ltd. General: North Kent





Histogram Start Time 17:08:28 May 3, 2017 Histogram Finish Time 19:07:03 May 3, 2017 Number of Intervals 3557.00 at 2 seconds Geo:254.0 mm/s Range Sample Rate 2048sps

Serial Number BE18695 V 10.72-8.17 MiniMate Plus **Battery Level** 6.4 Volts Unit Calibration February 22, 2017 by Instantel File Name T695GVJO.Y40

Notes

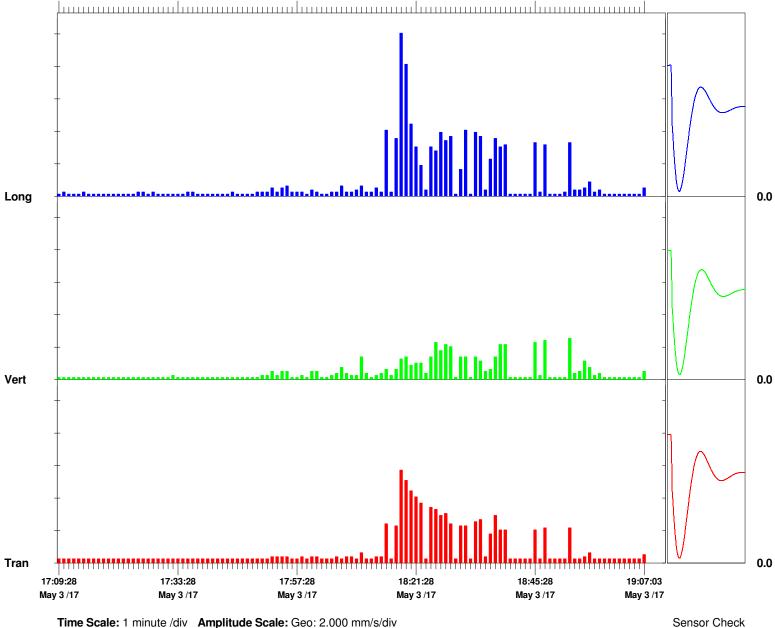
Location:	8100 Claymore Rd 8680 Bush Line
Client:	Pattern
User Name:	Golder Associates Ltd.
General:	10m, 1668031

Extended Notes

Geophone anchored into subgrade, and sandbagged

	Tran	Vert	Long	
PPV	5.715	2.540	10.03	mm/s
ZC Freq	9.5	28	13.7	Hz
Date	May 3 /17	May 3 /17	May 3 /17	
Time	18:18:08	18:51:46	18:18:08	
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.3	7.5	Hz
Overswing Ratio	3.7	4.0	4.1	
	7.6	7.3	7.5	Hz

Peak Vector Sum 10.17 mm/s on May 3, 2017 at 18:18:08





Velocity (mm/s)

 Date/Time
 Vert at 18:10:03 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVJR.SR0

Notes

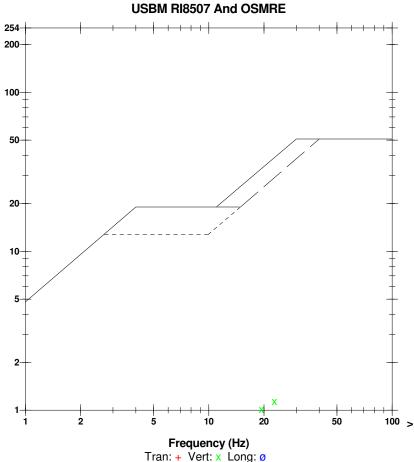
Location: 8100 Claymore Rd 8680 Bush Line Client: Pattern User Name: Golder Associates Ltd. General: 10m, 1668031

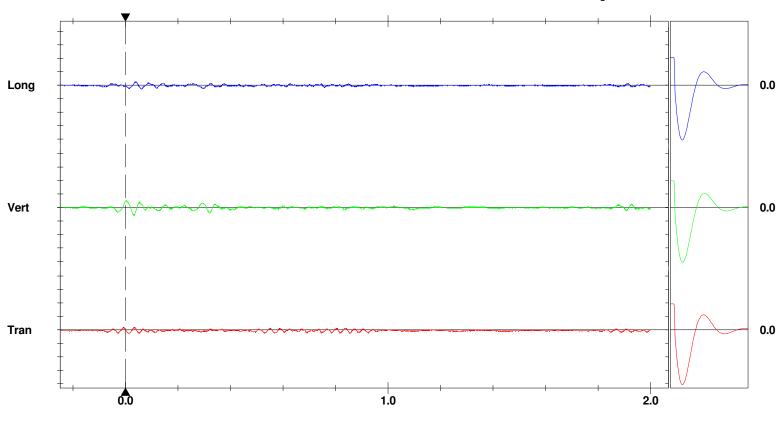
Extended Notes

Combo Mode May 3, 2017 17:08:27 Geophone anchored into subgrade, and sandbagged

	Tran	Vert	Long	
PPV	0.508	1.143	0.635	mm/s
ZC Freq	33	23	28	Hz
Time (Rel. to Trig)	-0.030	0.030	0.063	sec
Peak Acceleration	0.053	0.053	0.053	g
Peak Displacement	0.004	0.009	0.004	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.3	7.5	Hz
Overswing Ratio	3.7	4.0	4.1	

Peak Vector Sum 1.264 mm/s at 0.035 sec





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger = ▶ ____ ◀



Date/Time Vert at 18:10:03 May 3, 2017 Trigger Source Geo: 1.000 mm/s Range Geo: 254.0 mm/s **Record Time** 2.0 sec at 2048 sps

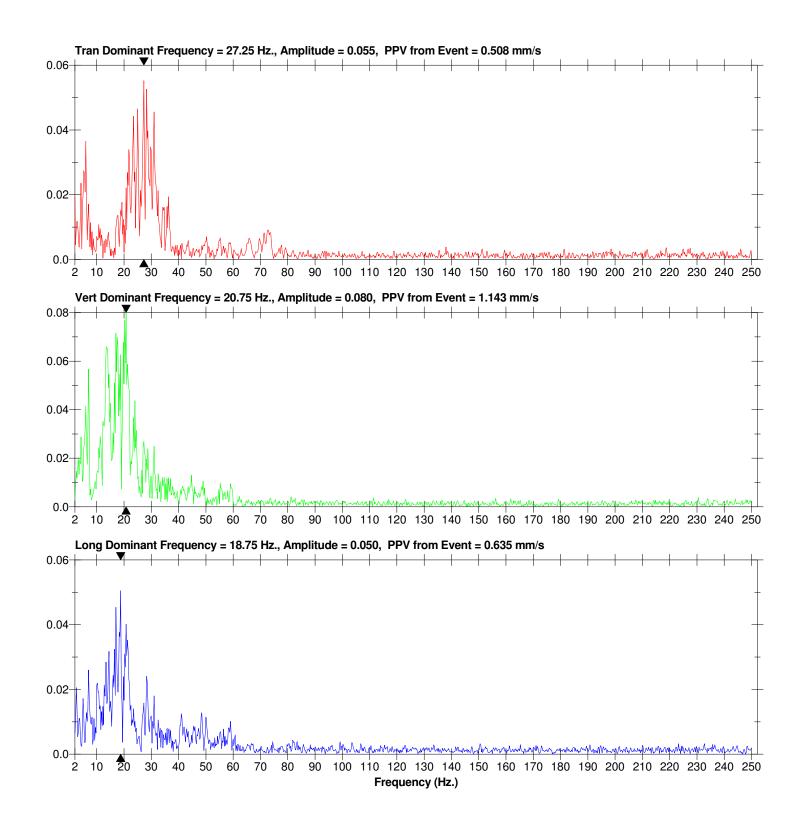
Serial Number BE18695 V 10.72-8.17 MiniMate Plus **Battery Level** 6.4 Volts Unit Calibration February 22, 2017 by Instantel File Name T695GVJR.SR0

Notes

Client:

8680 Bush Line Location: 8100 Claymore Rd Pattern User Name: Golder Associates Ltd. General: 10m, 1668031

Extended Notes Combo Mode May 3, 2017 17:08:27 Geophone anchored into subgrade, and sandbagged





Velocity (mm/s)

Date/Time	Vert at 18:10:05 May 3, 2017
Range	Geo: 1.000 mm/s Geo: 254.0 mm/s
Record Time	2.0 sec at 2048 sps

Serial Number
Battery LevelBE18695 V 10.72-8.17 MiniMate Plus
6.4 VoltsUnit Calibration
File NameFebruary 22, 2017 by Instantel
T695GVJR.ST0

Notes Location: 8100 Claymore Rd-Client: Pattern

User Name: Golder Associates Ltd. General: 10m, 1668031

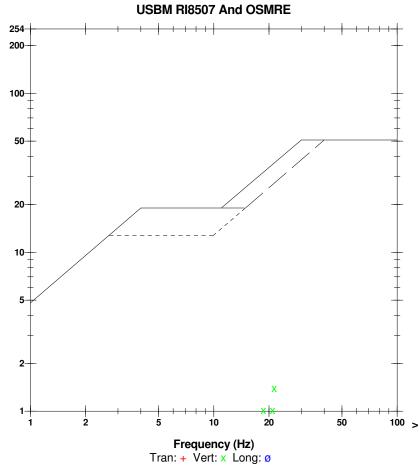
Extended Notes

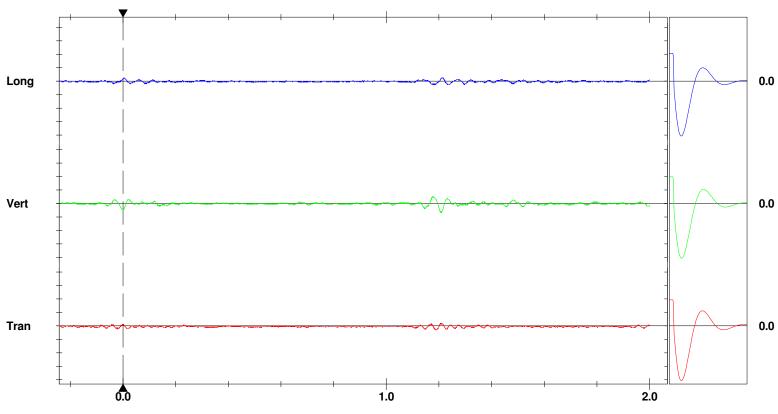
Combo Mode May 3, 2017 17:08:27 Geophone anchored into subgrade, and sandbagged

8680 Bush Line

	Tran	Vert	Long	
PPV	0.635	1.397	0.508	mm/s
ZC Freq	26	21	31	Hz
Time (Rel. to Trig)	1.189	1.207	0.003	sec
Peak Acceleration	0.053	0.053	0.053	g
Peak Displacement	0.004	0.010	0.005	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.3	7.5	Hz
Overswing Ratio	3.7	4.0	4.1	

Peak Vector Sum 1.497 mm/s at 1.208 sec





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger = ► ____ ◀



 Date/Time
 Vert at 18:10:05 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

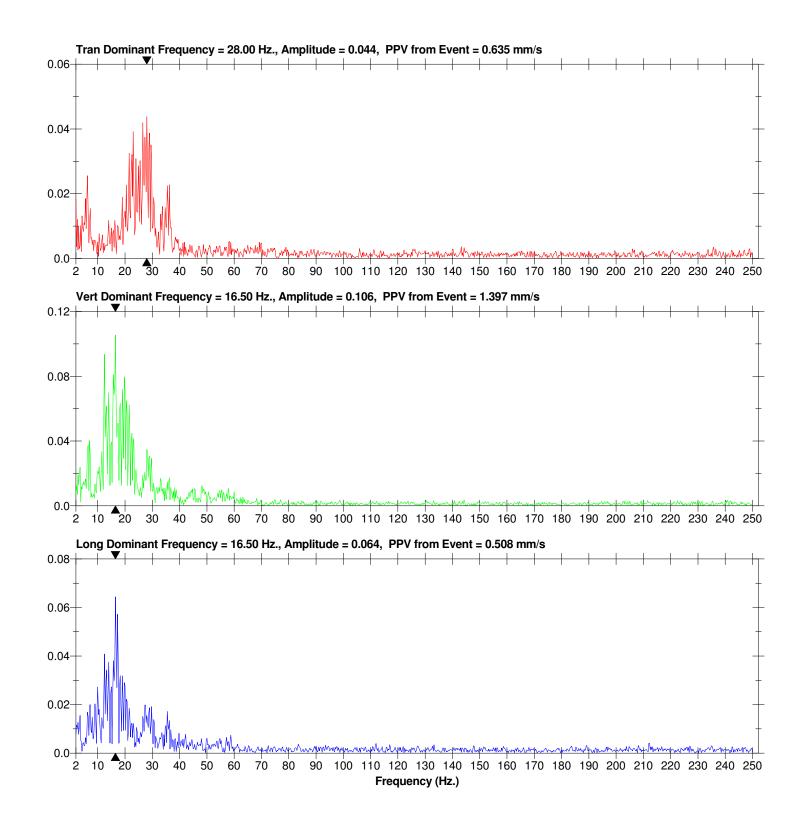
 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVJR.ST0

Notes

Location: 8100 Claymore Rd 8680 Bush Line Client: Pattern User Name: Golder Associates Ltd. General: 10m, 1668031 **Extended Notes** Combo Mode May 3, 2017 17:08:27 Geophone anchored into subgrade, and sandbagged





Velocity (mm/s)

 Date/Time
 Vert at 18:10:09 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Notes

Location: 8100 Claymore Rd 8680 Bush Line Client: Pattern User Name: Golder Associates Ltd. General: 10m, 1668031

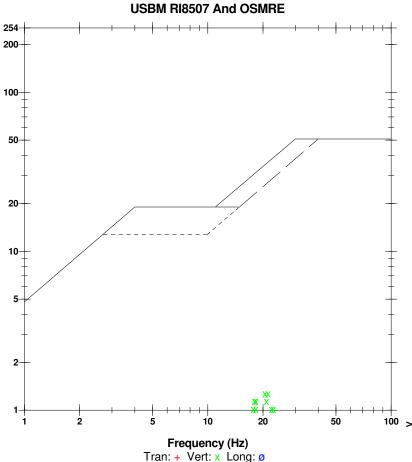
Extended Notes

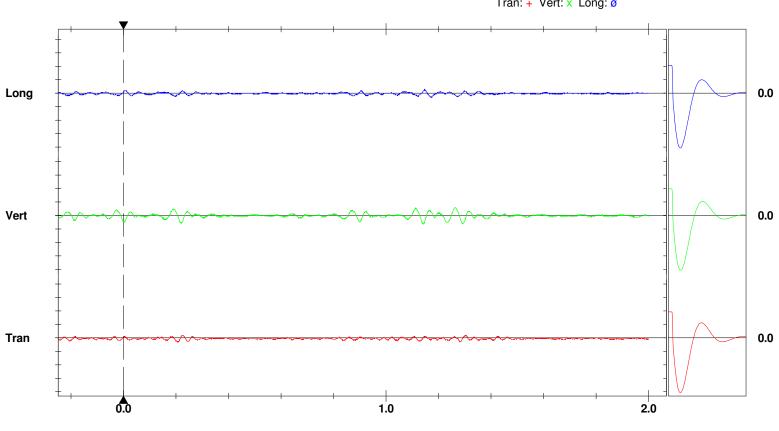
Combo Mode May 3, 2017 17:08:27 Geophone anchored into subgrade, and sandbagged

	Tran	Vert	Long	
PPV	0.635	1.270	0.635	mm/s
ZC Freq	24	21	33	Hz
Time (Rel. to Trig)	0.205	1.140	1.146	sec
Peak Acceleration	0.053	0.053	0.053	g
Peak Displacement	0.004	0.011	0.005	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.3	7.5	Hz
Overswing Ratio	3.7	4.0	4.1	

Peak Vector Sum 1.332 mm/s at 1.143 sec

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVJR.SX0





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger = ▶ ____ ◀



Vert at 18:10:09 May 3, 2017 Date/Time Trigger Source Geo: 1.000 mm/s Range Geo: 254.0 mm/s **Record Time** 2.0 sec at 2048 sps

Serial Number BE18695 V 10.72-8.17 MiniMate Plus **Battery Level** 6.4 Volts Unit Calibration February 22, 2017 by Instantel File Name T695GVJR.SX0

Notes Location:

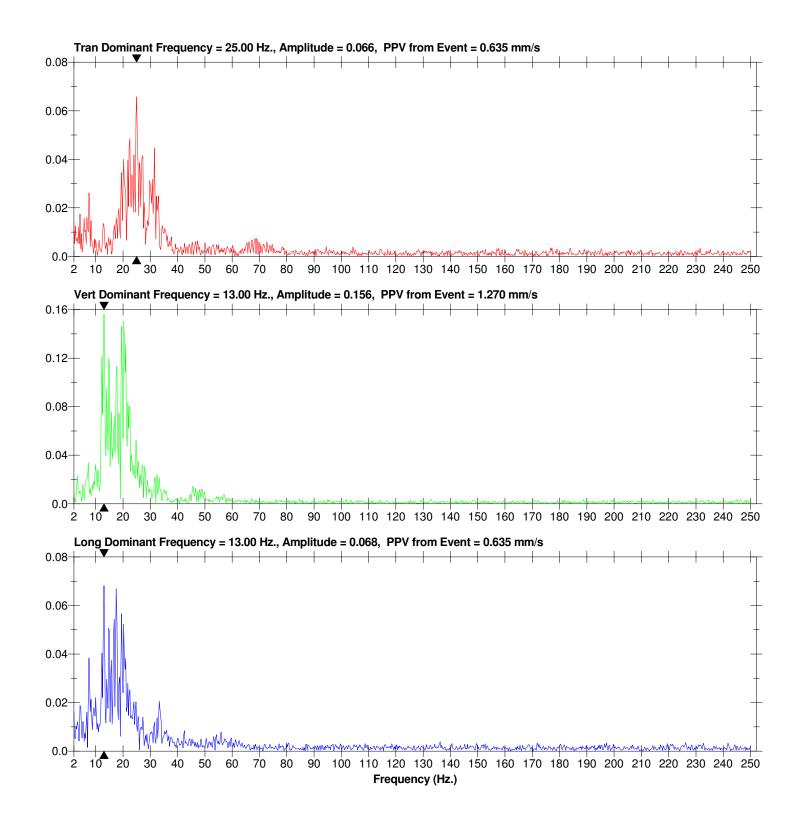
8680 Bush Line D Claymore Rd

Client: User Name: General:

ttern

Golder Associates Ltd. 10m, 1668031

Extended Notes Combo Mode May 3, 2017 17:08:27 Geophone anchored into subgrade, and sandbagged





Velocity (mm/s)

 Date/Time
 Long at 18:14:28 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Notes

Location:	8100 Claymore Rd- 8680 Bush Line
Client:	Pattern
User Name:	Golder Associates Ltd.
General:	10m, 1668031

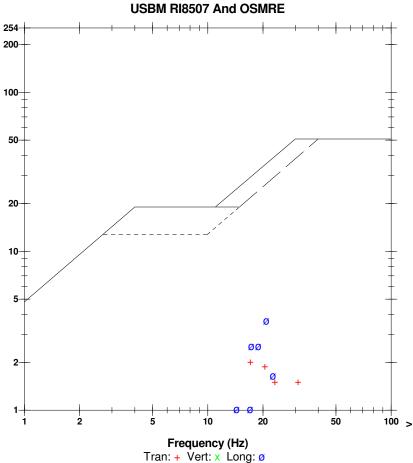
Extended Notes

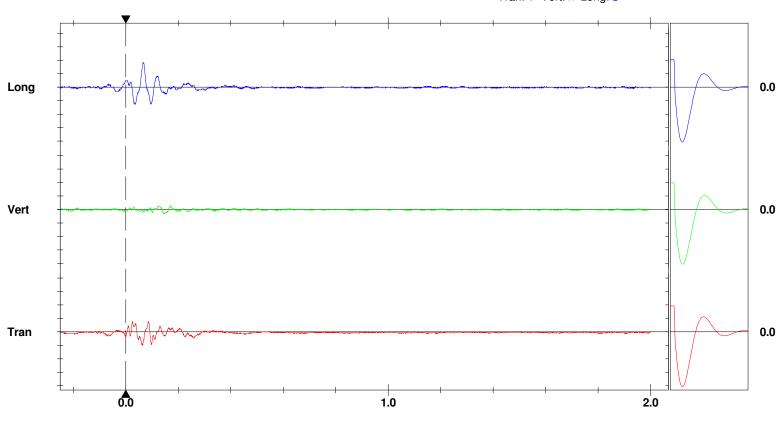
Combo Mode May 3, 2017 17:08:27 Geophone anchored into subgrade, and sandbagged

	Tran	Vert	Long	
PPV	2.032	0.635	3.683	mm/s
ZC Freq	17.1	22	21	Hz
Time (Rel. to Trig)	0.063	0.143	0.065	sec
Peak Acceleration	0.080	0.053	0.080	g
Peak Displacement	0.017	0.005	0.026	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.3	7.5	Hz
Overswing Ratio	3.7	4.0	4.1	

Peak Vector Sum 4.098 mm/s at 0.065 sec

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVJS.040





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger = ▶ ____ ◀



 Date/Time
 Long at 18:14:28 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

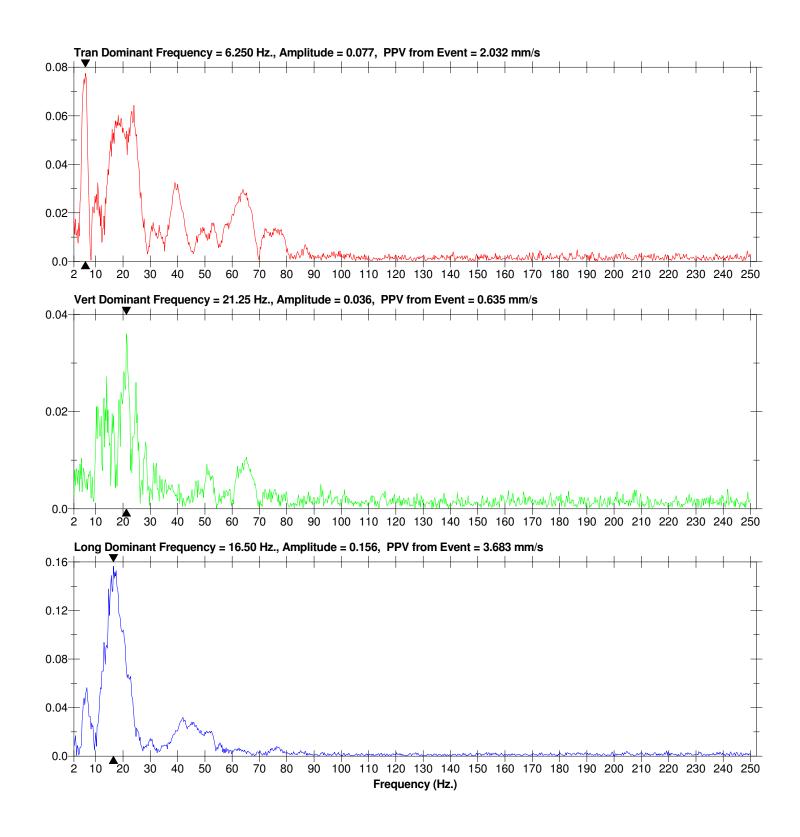
 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVJS.040

Notes

Location: 8100 Claymore Rd 8680 Bush Line Client: Pattern User Name: Golder Associates Ltd. General: 10m, 1668031 **Extended Notes** Combo Mode May 3, 2017 17:08:27 Geophone anchored into subgrade, and sandbagged





Velocity (mm/s)

 Date/Time
 Long at 18:14:47 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Notes

Location:	8100 Claymore Rd	8680 Bush Line
Client:	Pattern	
User Name:	Golder Associates Ltd.	
General:	10m, 1668031	

Extended Notes

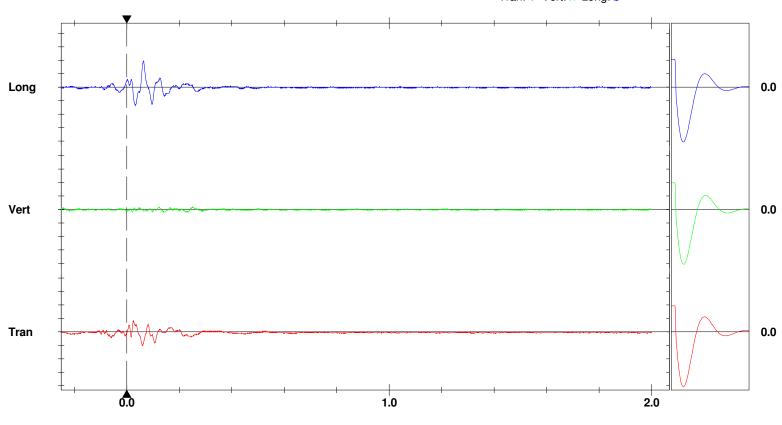
Combo Mode May 3, 2017 17:08:27 Geophone anchored into subgrade, and sandbagged

	Tran	Vert	Long	
PPV	2.032	0.381	3.937	mm/s
ZC Freq	16.5	73	18.3	Hz
Time (Rel. to Trig)	0.059	0.020	0.063	sec
Peak Acceleration	0.053	0.027	0.080	g
Peak Displacement	0.017	0.003	0.026	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.3	7.5	Hz
Overswing Ratio	3.7	4.0	4.1	

Peak Vector Sum 4.322 mm/s at 0.063 sec

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVJS.0N0

USBM RI8507 And OSMRE 254-++++ 200-100-50 20 10 5 ø 2 ø Ø Ø 1 2 5 10 20 50 100 > Frequency (Hz) Tran: + Vert: x Long: Ø



Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger = ▶ ____ ◀



 Date/Time
 Long at 18:14:47 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

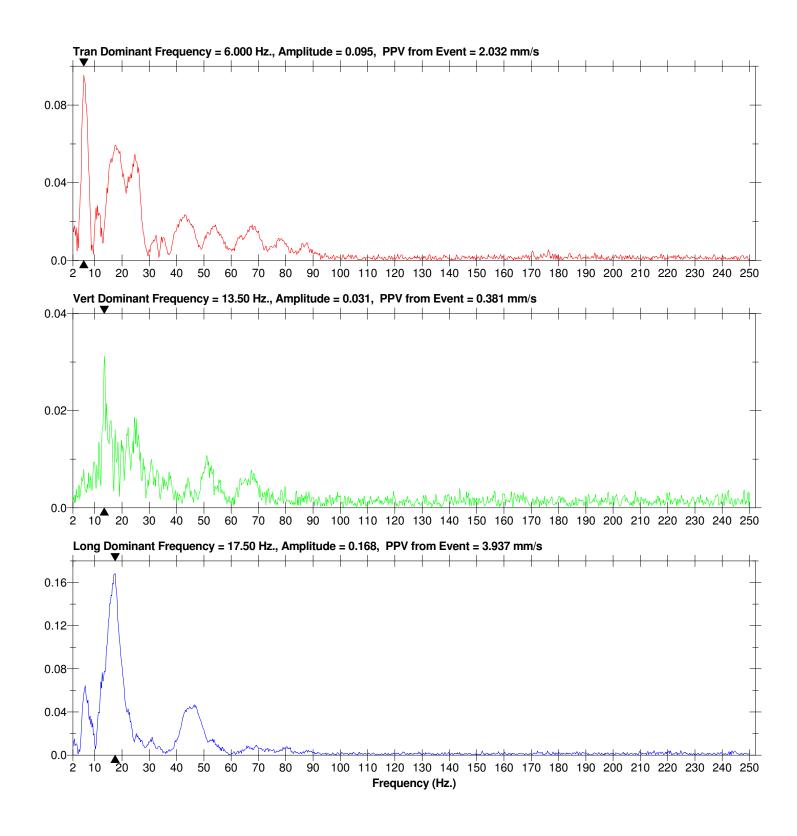
 Record Time
 2.0 sec at 2048 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVJS.0N0

Notes

Location: 8100 C Client: Pattern User Name: Golden General: 10m, 1

8100 Claymore Rd Pattern Golder Associates Ltd. 10m, 1668031 Extended Notes Combo Mode May 3, 2017 17:08:27 Geophone anchored into subgrade, and sandbagged





Velocity (mm/s)

Date/Time	Long at 18:15:19 May 3, 2017
Trigger Source	Geo: 1.000 mm/s
Range	Geo: 254.0 mm/s
Record Time	2.0 sec at 2048 sps

Serial Number
Battery LevelBE18695 V 10.72-8.17 MiniMate Plus
6.4 VoltsUnit Calibration
File NameFebruary 22, 2017 by Instantel
T695GVJS.1J0

Notes

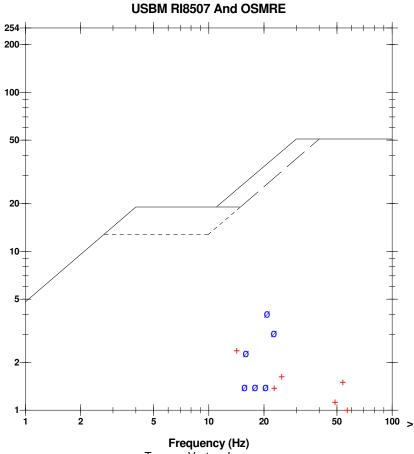
Location:8100 Claymore Rd8680 Bush LineClient:PatternUser Name:Golder Associates Ltd.General:10m, 1668031

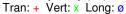
Extended Notes

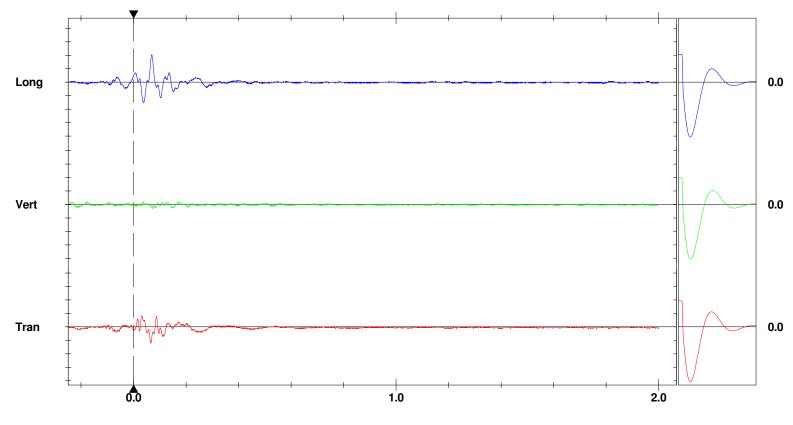
Combo Mode May 3, 2017 17:08:27 Geophone anchored into subgrade, and sandbagged

	Tran	Vert	Long	
PPV	2.413	0.635	4.064	mm/s
ZC Freq	14.2	45	21	Hz
Time (Rel. to Trig)	0.065	0.072	0.068	sec
Peak Acceleration	0.053	0.053	0.080	g
Peak Displacement	0.019	0.003	0.026	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.3	7.5	Hz
Overswing Ratio	3.7	4.0	4.1	

Peak Vector Sum 4.560 mm/s at 0.068 sec







Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger = ► ____ ←



 Date/Time
 Long at 18:15:19 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

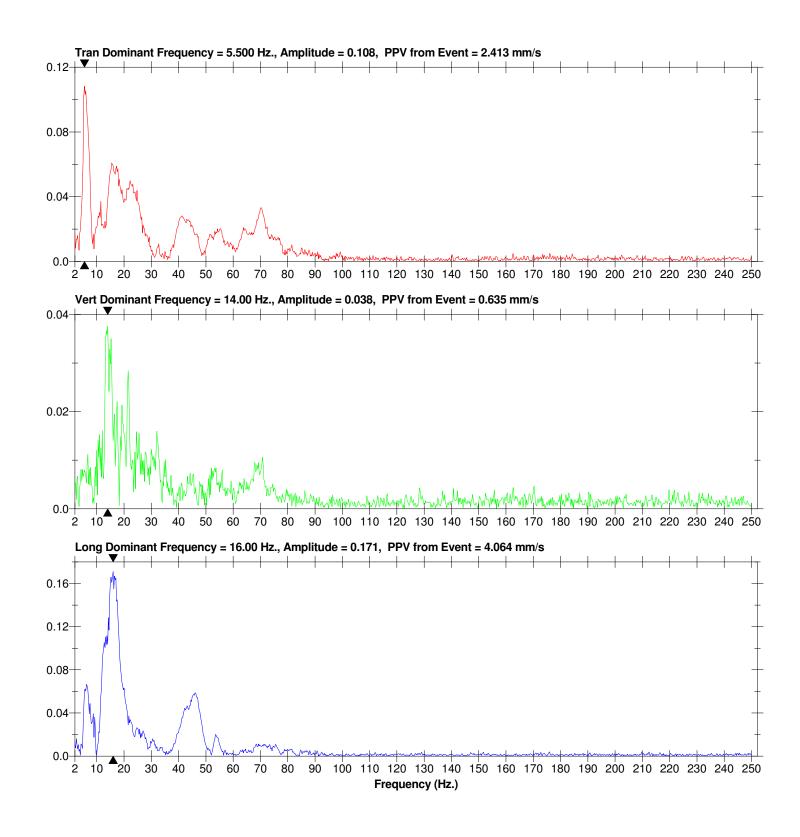
 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVJS.1J0

Notes

Location: 8100 Claymore Rd 8680 Bush Line Client: Pattern User Name: Golder Associates Ltd. General: 10m, 1668031 Extended Notes Combo Mode May 3, 2017 17:08:27 Geophone anchored into subgrade, and sandbagged





Velocity (mm/s)

 Date/Time
 Long at 18:17:02 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Notes

Location:	8100 Claymore Rd	8680 Bush Line
Client:	Pattern	
User Name:	Golder Associates Ltd.	
General:	10m, 1668031	

Extended Notes

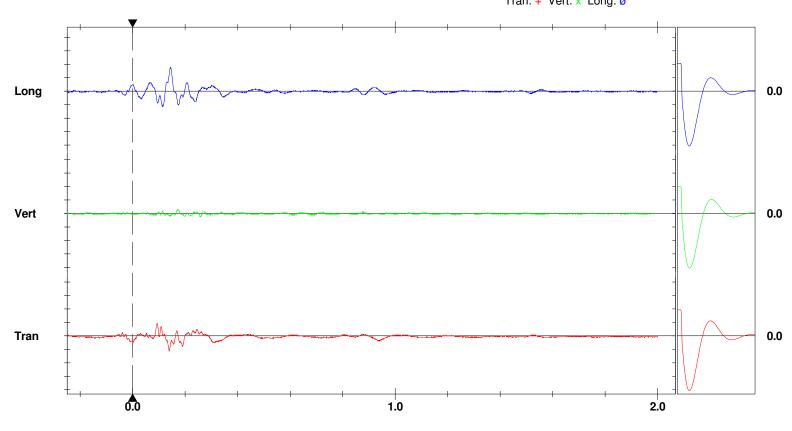
Combo Mode May 3, 2017 17:08:27 Geophone anchored into subgrade, and sandbagged

	Tran	Vert	Long	
PPV	2.286	0.635	3.556	mm/s
ZC Freq	13.0	37	15.5	Hz
Time (Rel. to Trig)	0.139	0.171	0.144	sec
Peak Acceleration	0.053	0.053	0.053	g
Peak Displacement	0.021	0.003	0.030	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.3	7.5	Hz
Overswing Ratio	3.7	4.0	4.1	

Peak Vector Sum 3.829 mm/s at 0.144 sec

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVJS.4E0

USBM RI8507 And OSMRE 254-++++ 200-100-50 20 10 ø +Ø Ø 2 ø ø ø 1 2 5 10 20 50 100 > Frequency (Hz) Tran: + Vert: x Long: ø



Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger = ► ____



 Date/Time
 Long at 18:17:02 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

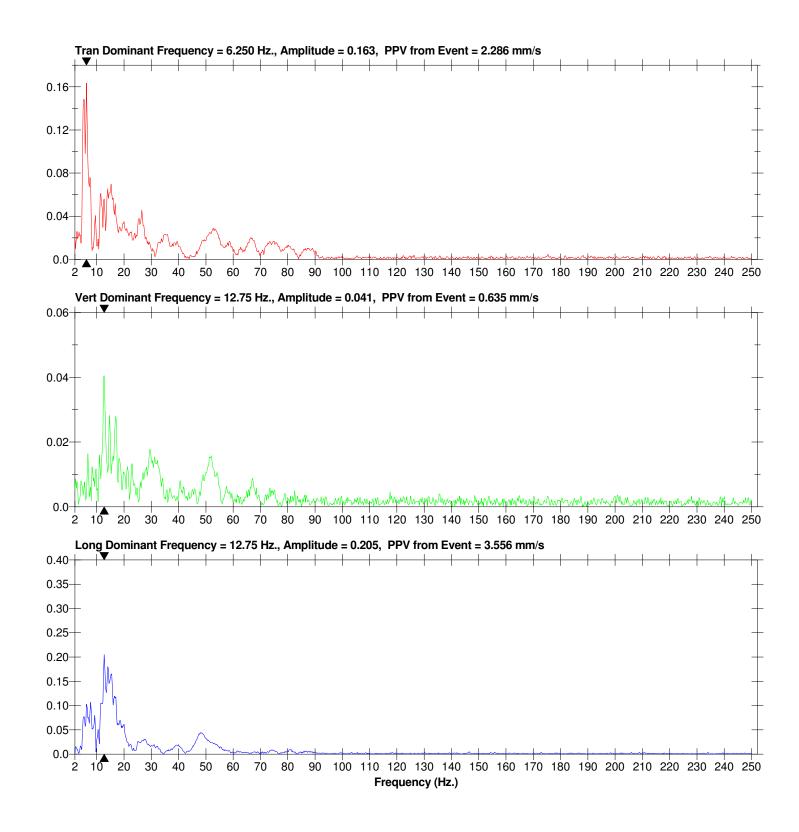
 Record Time
 2.0 sec at 2048 sps

Notes

Location: 8100 Claymore Rd 8680 Bush Line Client: Pattern User Name: Golder Associates Ltd. General: 10m, 1668031 Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.4 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVJS.4E0

Extended Notes

Combo Mode May 3, 2017 17:08:27 Geophone anchored into subgrade, and sandbagged





Histogram Start Time14:33:32 May 4, 2017Histogram Finish Time15:32:18 May 4, 2017Number of Intervals1763.00 at 2 secondsRangeGeo:254.0 mm/sSample Rate2048sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVLC.FW0

Notes

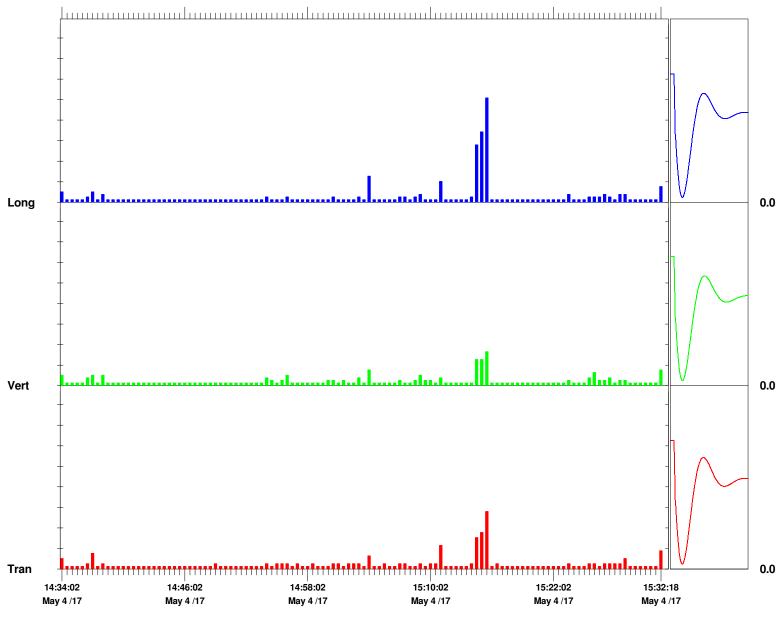
Location:	-8100 Claymore Rd 8680 Bush Line
Client:	Pattern
User Name:	Golder Associates Ltd.
General:	10m, 1668031

Extended Notes

Geophone anchored into subgrade, and sandbagged

	Tran	Vert	Long	
PPV	2.794	1.651	5.080	mm/s
ZC Freq	27	32	29	Hz
Date	May 4 /17	May 4 /17	May 4 /17	
Time	15:15:20	15:15:20	15:15:12	
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.3	7.4	Hz
Overswing Ratio	3.7	4.0	4.2	
Sensor Check	Passed 7.6	Passed 7.3	Passed 7.4	Hz

Peak Vector Sum 5.187 mm/s on May 4, 2017 at 15:15:12



Time Scale: 30 seconds /div Amplitude Scale: Geo: 1.000 mm/s/div



Velocity (mm/s)

 Date/Time
 Long at 15:03:34 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Notes

Location:	8100 Claymore Rd	8680 Bush Line
Client:	Pattern	
User Name:	Golder Associates Ltd.	
General:	10m, 1668031	

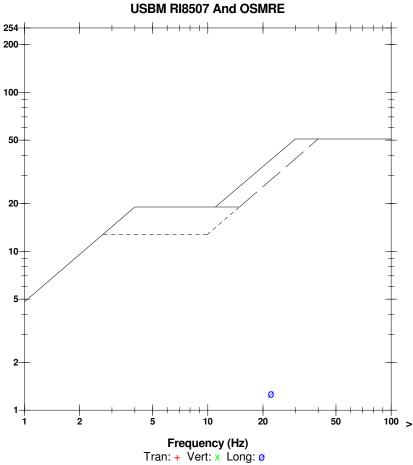
Extended Notes

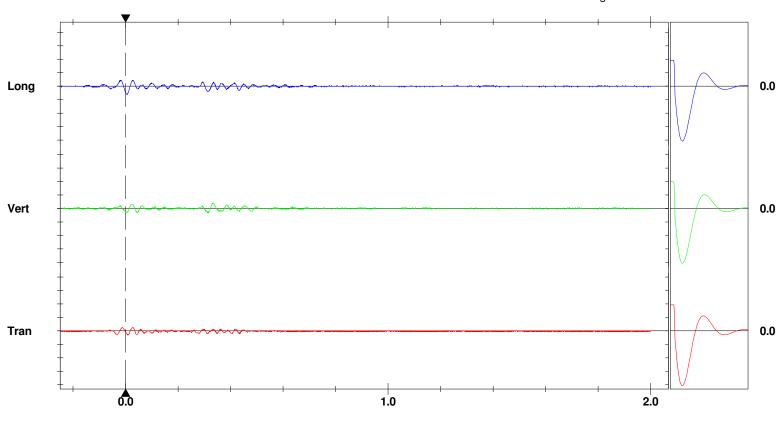
Combo Mode May 4, 2017 14:33:32 Geophone anchored into subgrade, and sandbagged

	Tran	Vert	Long	
PPV	0.635	0.762	1.270	mm/s
ZC Freq	30	26	22	Hz
Time (Rel. to Trig)	0.008	0.330	0.005	sec
Peak Acceleration	0.027	0.027	0.053	g
Peak Displacement	0.004	0.005	0.009	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.3	7.4	Hz
Overswing Ratio	3.7	4.0	4.2	

Peak Vector Sum 1.508 mm/s at 0.005 sec

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVLD.TY0





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger =



 Date/Time
 Long at 15:03:34 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

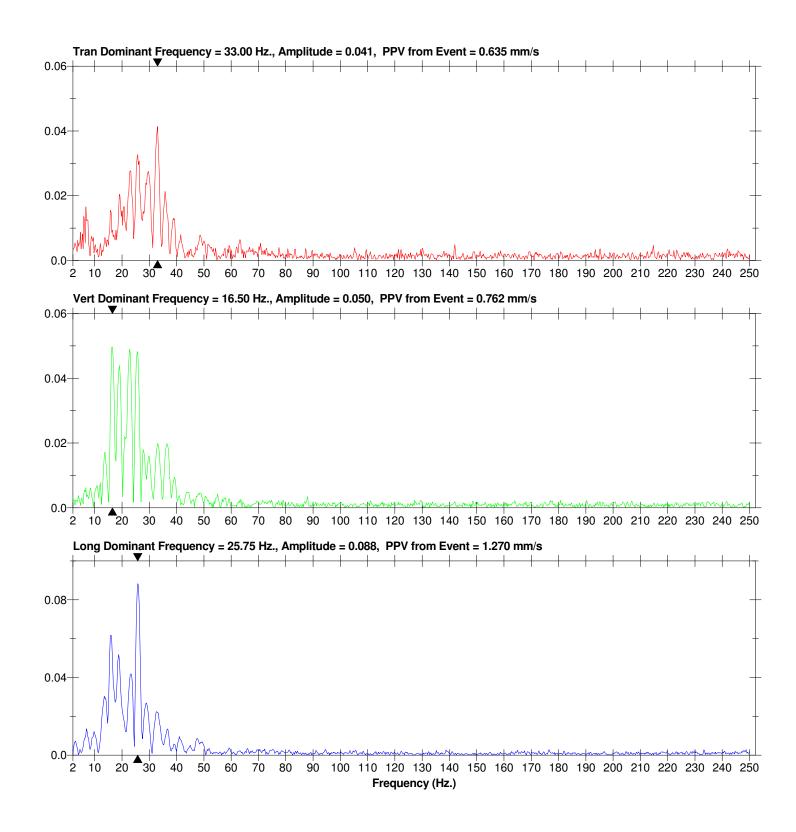
 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVLD.TY0

Notes

Location: 8100 Claymore Rd 8680 Bush Line Client: Pattern User Name: Golder Associates Ltd. General: 10m, 1668031 **Extended Notes** Combo Mode May 4, 2017 14:33:32 Geophone anchored into subgrade, and sandbagged





Velocity (mm/s)

 Date/Time
 Tran at 15:10:58 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Notes

Location:	-8100 Claymore Rd	8680 Bush Line
Client:	Pattern	
User Name:	Golder Associates Ltd.	
General:	10m, 1668031	

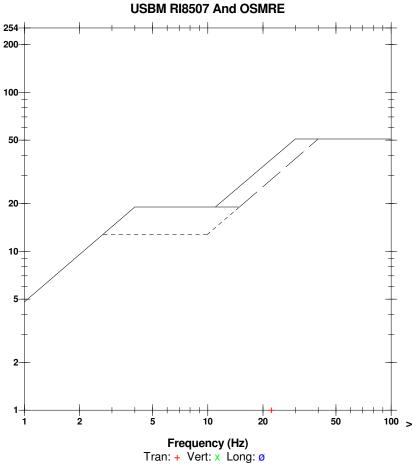
Extended Notes

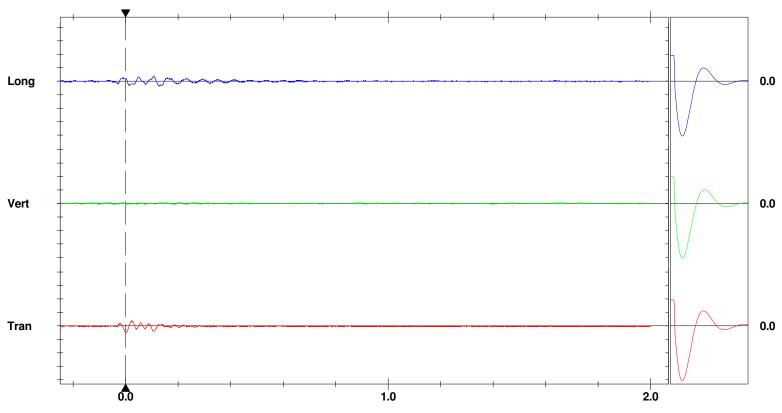
Combo Mode May 4, 2017 14:33:32 Geophone anchored into subgrade, and sandbagged

	Tran	Vert	Long	
PPV	1.016	0.127	0.762	mm/s
ZC Freq	22	>200	21	Hz
Time (Rel. to Trig)	0.000	-0.244	0.107	sec
Peak Acceleration	0.027	0.053	0.027	g
Peak Displacement	0.007	0.000	0.007	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.3	7.4	Hz
Overswing Ratio	3.7	4.0	4.2	

Peak Vector Sum 1.092 mm/s at 0.000 sec

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVLE.6A0





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger = ▶ ____ ◀



Date/Time Tran at 15:10:58 May 4, 2017 Trigger Source Geo: 1.000 mm/s Range Geo: 254.0 mm/s **Record Time** 2.0 sec at 2048 sps

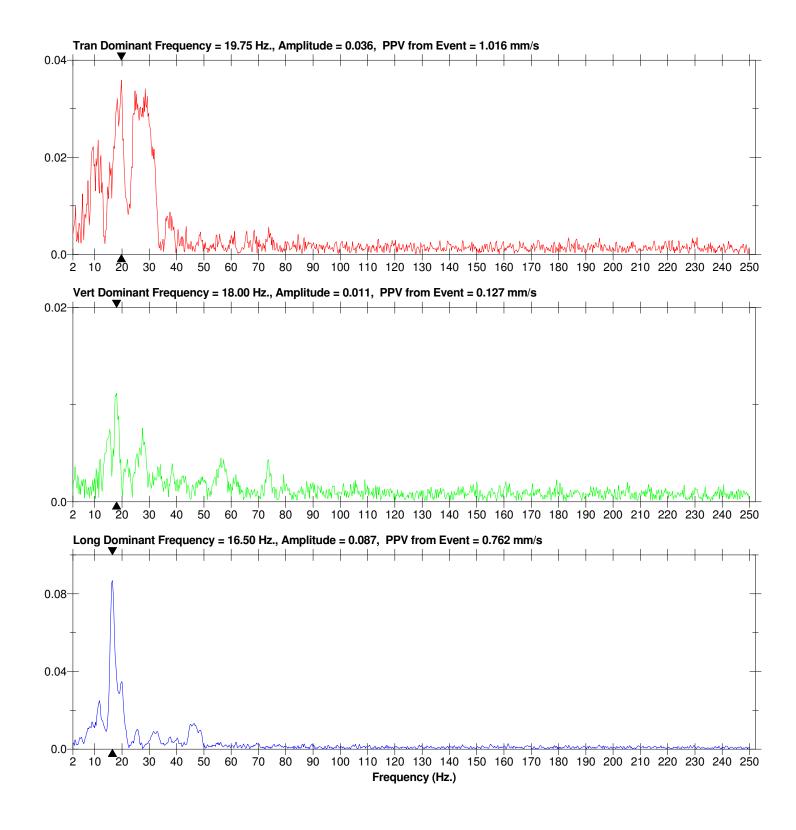
Notes Client:

8680 Bush Line 8100 Claymore Rd Location: Pattern User Name: Golder Associates Ltd. General: 10m, 1668031

Serial Number BE18695 V 10.72-8.17 MiniMate Plus **Battery Level** 6.3 Volts Unit Calibration February 22, 2017 by Instantel File Name T695GVLE.6A0

Extended Notes

Combo Mode May 4, 2017 14:33:32 Geophone anchored into subgrade, and sandbagged





Velocity (mm/s)

 Date/Time
 Long at 15:11:00 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Notes

110100		
Location:	-8100 Claymore Rd	8680 Bush Line
Client:	Pattern	
User Name:	Golder Associates Ltd.	
General:	10m, 1668031	

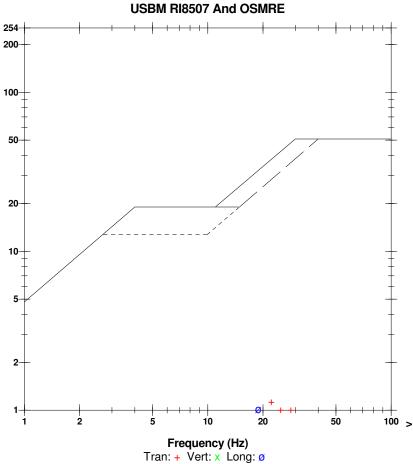
Extended Notes

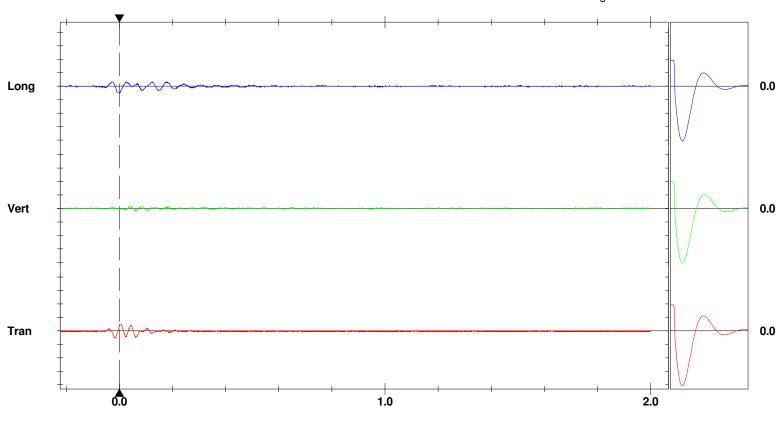
Combo Mode May 4, 2017 14:33:32 Geophone anchored into subgrade, and sandbagged

	Tran	Vert	Long	
PPV	1.143	0.381	1.016	mm/s
ZC Freq	22	47	19.0	Hz
Time (Rel. to Trig)	-0.016	0.041	0.000	sec
Peak Acceleration	0.053	0.053	0.053	g
Peak Displacement	0.008	0.002	0.009	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.3	7.4	Hz
Overswing Ratio	3.7	4.0	4.2	

Peak Vector Sum 1.350 mm/s at 0.002 sec

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVLE.6C0





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger = ► ____



 Date/Time
 Long at 15:11:00 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

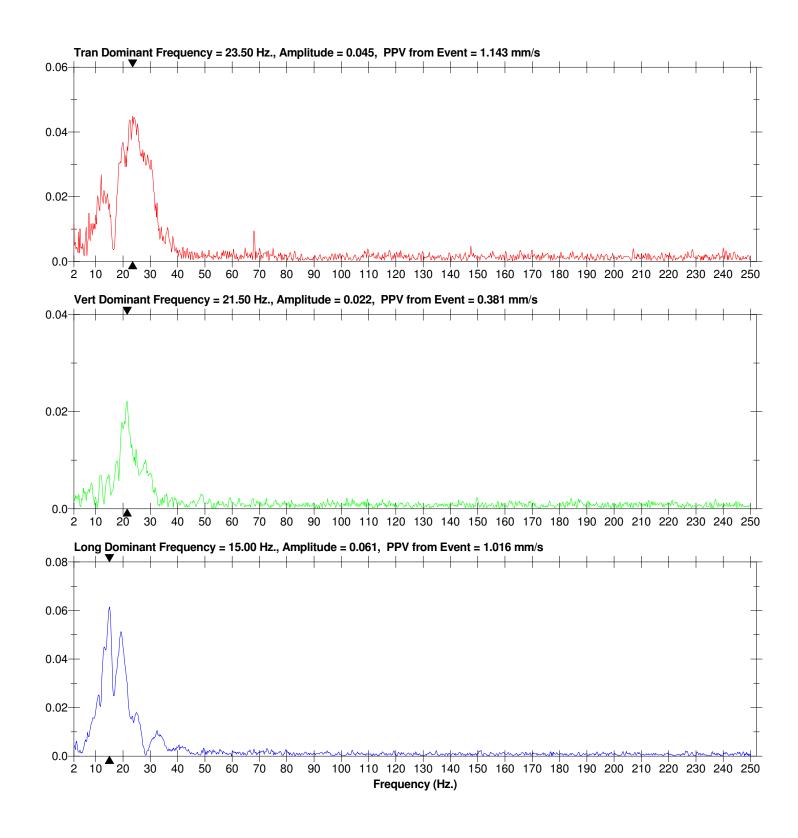
 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVLE.6C0

Notes

Location: 8100 Claymore Rd 8680 Bush Line Client: Pattern User Name: Golder Associates Ltd. General: 10m, 1668031 **Extended Notes** Combo Mode May 4, 2017 14:33:32 Geophone anchored into subgrade, and sandbagged





Velocity (mm/s)

 Date/Time
 Long at 15:14:11 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Notes

Location:	8100 Claymore Rd	8680 Bush Line
Client:	Pattern	0000 2000 2000
User Name:	Golder Associates Ltd.	
General:	10m, 1668031	

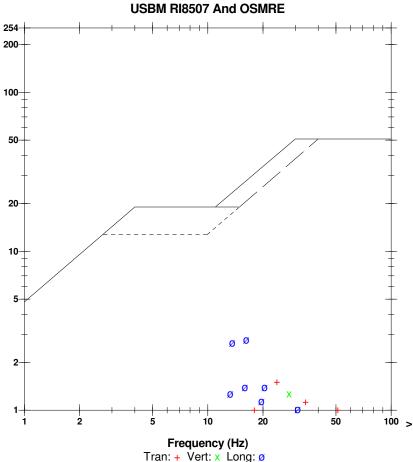
Extended Notes

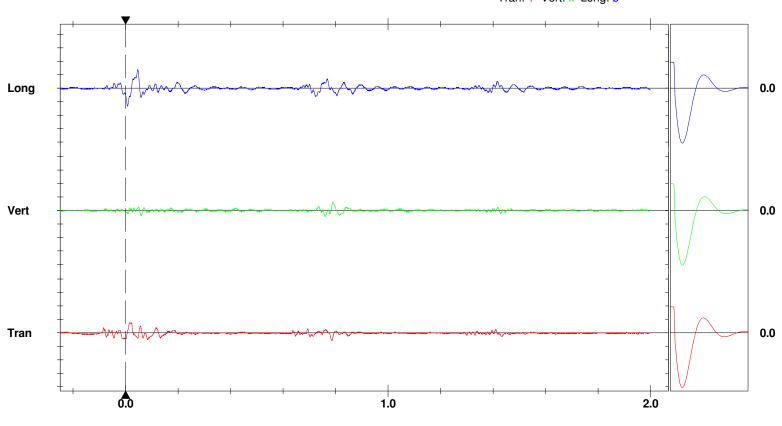
Combo Mode May 4, 2017 14:33:32 Geophone anchored into subgrade, and sandbagged

	Tran	Vert	Long	
PPV	1.524	1.270	2.794	mm/s
ZC Freq	24	28	16.3	Hz
Time (Rel. to Trig)	0.015	0.790	0.046	sec
Peak Acceleration	0.053	0.053	0.053	g
Peak Displacement	0.011	0.007	0.024	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.3	7.4	Hz
Overswing Ratio	3.7	4.0	4.2	

Peak Vector Sum 2.907 mm/s at 0.046 sec

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVLE.BN0





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger =



Date/Time Long at 15:14:11 May 4, 2017 Trigger Source Geo: 1.000 mm/s Range Geo: 254.0 mm/s **Record Time** 2.0 sec at 2048 sps

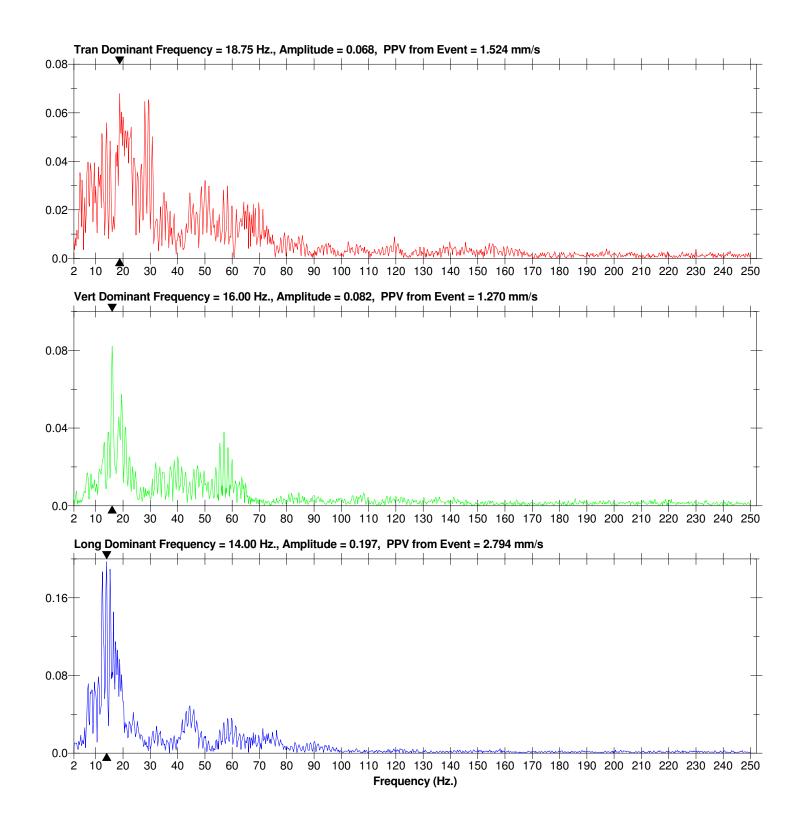
Notes

Location: 8100 Claymore Rd 8680 Bush Line Client: Pattern User Name: Golder Associates Ltd. General: 10m, 1668031

Serial Number BE18695 V 10.72-8.17 MiniMate Plus **Battery Level** 6.3 Volts Unit Calibration February 22, 2017 by Instantel File Name T695GVLE.BN0

Extended Notes

Combo Mode May 4, 2017 14:33:32 Geophone anchored into subgrade, and sandbagged





Velocity (mm/s)

 Date/Time
 Long at 15:14:39 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Notes

Location: 8100 Claymore Rd- 8680 Bush Line Client: Pattern User Name: Golder Associates Ltd. General: 10m, 1668031

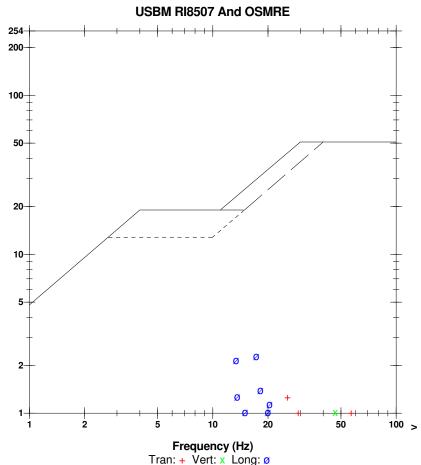
Extended Notes

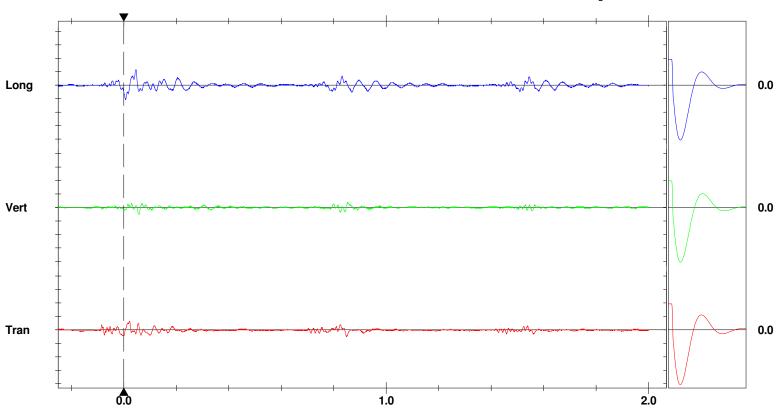
Combo Mode May 4, 2017 14:33:32 Geophone anchored into subgrade, and sandbagged

	Tran	Vert	Long	
PPV	1.270	1.016	2.286	mm/s
ZC Freq	26	47	17.4	Hz
Time (Rel. to Trig)	0.021	0.056	0.045	sec
Peak Acceleration	0.053	0.053	0.053	g
Peak Displacement	0.008	0.004	0.019	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.3	7.4	Hz
Overswing Ratio	3.7	4.0	4.2	

Peak Vector Sum 2.376 mm/s at 0.045 sec

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVLE.CF0





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger = ▶ ____ ◀



Date/Time Long at 15:14:39 May 4, 2017 Trigger Source Geo: 1.000 mm/s Range Geo: 254.0 mm/s **Record Time** 2.0 sec at 2048 sps

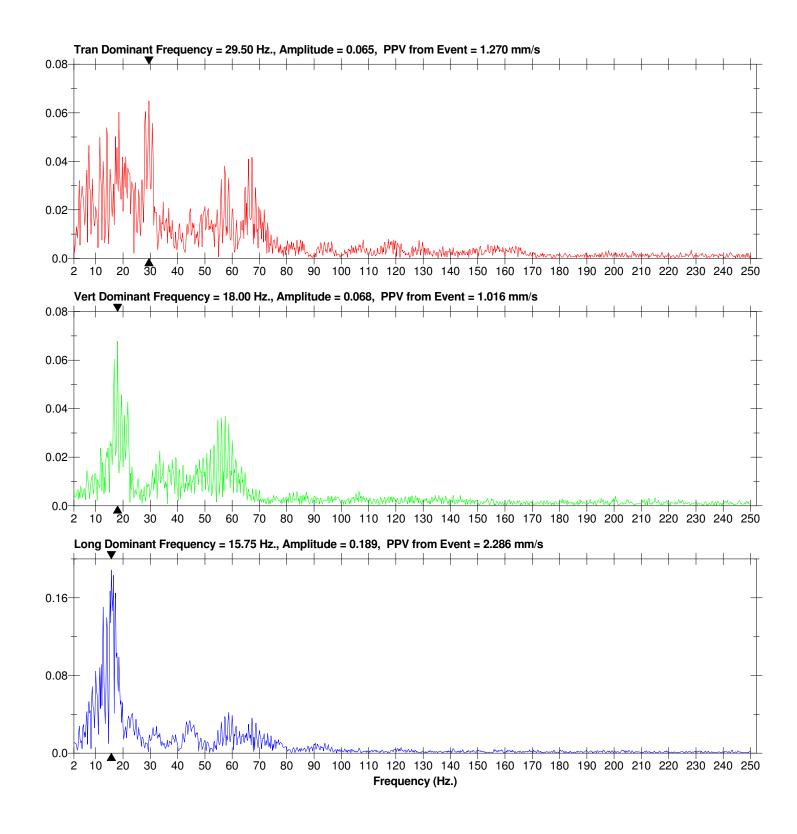
Notes

Location: 8100 Claymore Rd 8680 Bush Line Pattern Client: User Name: Golder Associates Ltd. General: 10m, 1668031

Serial Number BE18695 V 10.72-8.17 MiniMate Plus **Battery Level** 6.3 Volts Unit Calibration February 22, 2017 by Instantel File Name T695GVLE.CF0

Extended Notes

Combo Mode May 4, 2017 14:33:32 Geophone anchored into subgrade, and sandbagged





Velocity (mm/s)

Date/Time	Long at 15:15:00 May 4, 2017
Trigger Source	Geo: 1.000 mm/s
Range	Geo: 254.0 mm/s
Record Time	2.0 sec at 2048 sps

Serial Number
Battery LevelBE18695 V 10.72-8.17 MiniMate Plus
6.3 VoltsUnit Calibration
File NameFebruary 22, 2017 by Instantel
T695GVLE.D00

Notes

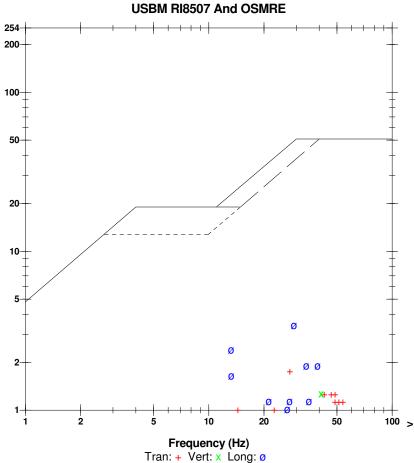
Location:8100 Claymore Rd8680 Bush LineClient:PatternUser Name:Golder Associates Ltd.General:10m, 1668031

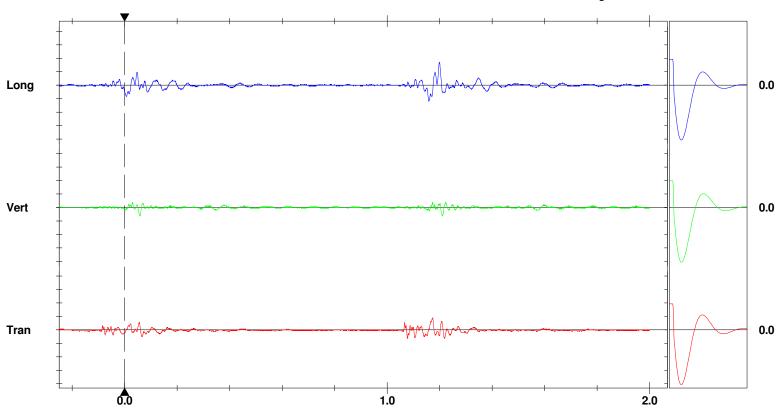
Extended Notes

Combo Mode May 4, 2017 14:33:32 Geophone anchored into subgrade, and sandbagged

	Tran	Vert	Long	
PPV	1.778	1.270	3.429	mm/s
ZC Freq	28	41	29	Hz
Time (Rel. to Trig)	1.171	0.057	1.199	sec
Peak Acceleration	0.080	0.053	0.080	g
Peak Displacement	0.010	0.005	0.022	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.3	7.4	Hz
Overswing Ratio	3.7	4.0	4.2	

Peak Vector Sum 3.563 mm/s at 1.199 sec





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger =



 Date/Time
 Long at 15:15:00 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

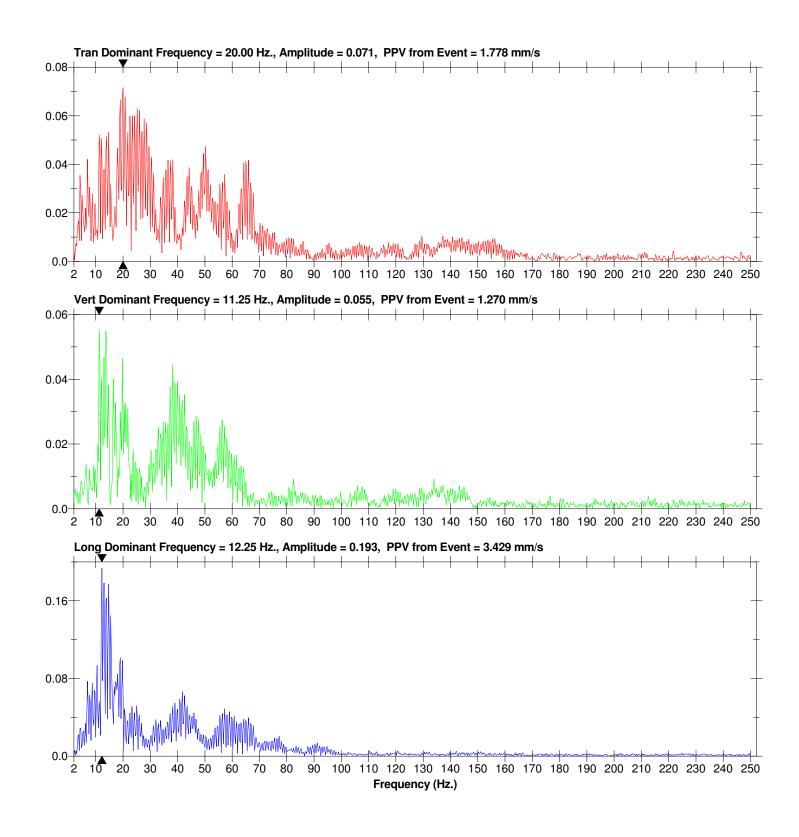
 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVLE.D00

Notes

Location: 8100 Claymore Rd 8680 Bush Line Client: Pattern User Name: Golder Associates Ltd. General: 10m, 1668031 **Extended Notes** Combo Mode May 4, 2017 14:33:32 Geophone anchored into subgrade, and sandbagged





Velocity (mm/s)

 Date/Time
 Tran at 15:15:02 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Notes

Location:	8100 Claymore Rd	8680 Bush Line
Client:	Pattern	
User Name:	Golder Associates Ltd.	
General:	10m, 1668031	

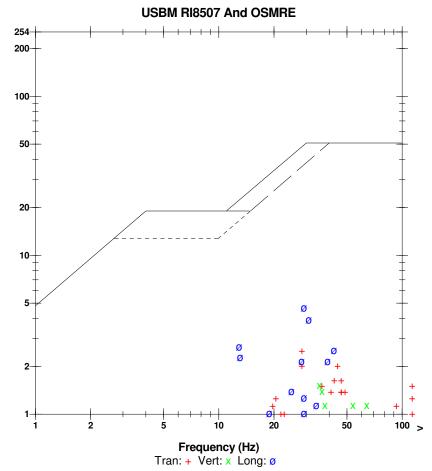
Extended Notes

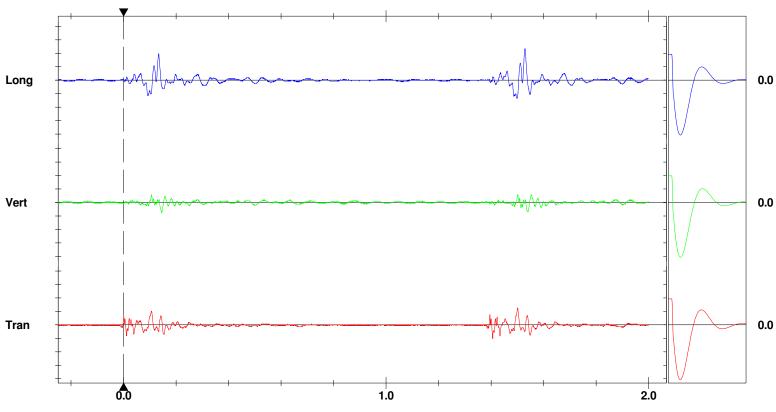
Combo Mode May 4, 2017 14:33:32 Geophone anchored into subgrade, and sandbagged

	Tran	Vert	Long	
PPV	2.540	1.524	4.699	mm/s
ZC Freq	28	35	29	Hz
Time (Rel. to Trig)	1.501	0.145	1.529	sec
Peak Acceleration	0.133	0.080	0.106	g
Peak Displacement	0.013	0.007	0.028	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.3	7.4	Hz
Overswing Ratio	3.7	4.0	4.2	

Peak Vector Sum 4.808 mm/s at 1.529 sec

Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVLE.D20





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger = ▶ ____ ◀



 Date/Time
 Tran at 15:15:02 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

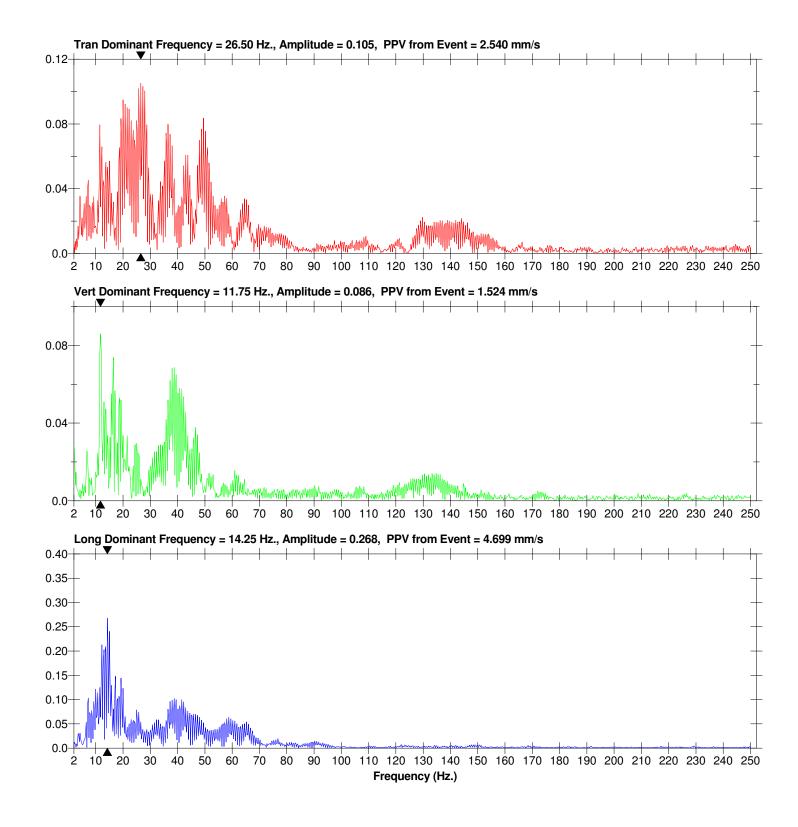
 Record Time
 2.0 sec at 2048 sps

Notes

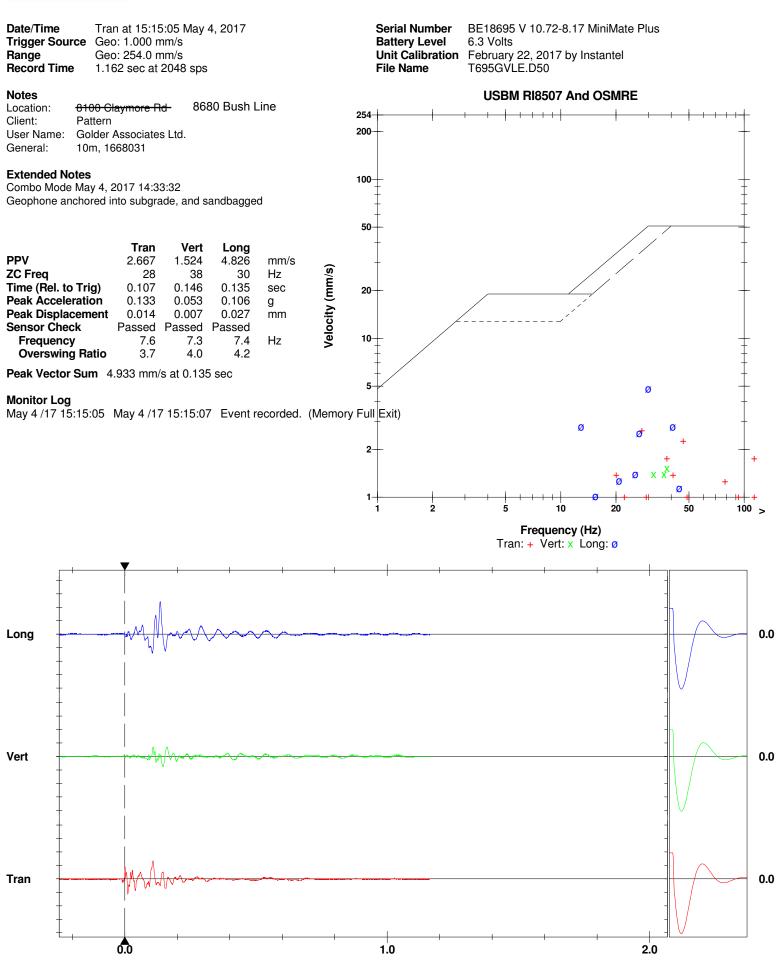
Location: 8100 Claymore Rd 8680 Bush Line Client: Pattern User Name: Golder Associates Ltd. General: 10m, 1668031 Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVLE.D20

Extended Notes

Combo Mode May 4, 2017 14:33:32 Geophone anchored into subgrade, and sandbagged







Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger =



 Date/Time
 Tran at 15:15:05 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

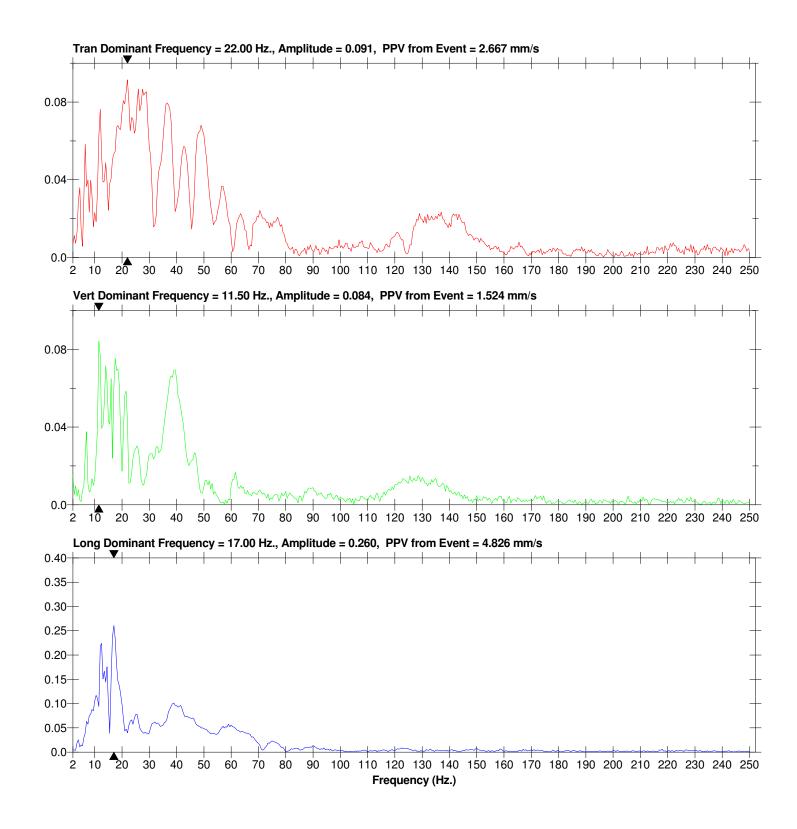
 Record Time
 1.162 sec at 2048 sps

Notes

Location: 8100 Claymore Rd 8680 Bush Line Client: Pattern User Name: Golder Associates Ltd. General: 10m, 1668031 Serial NumberBE18695 V 10.72-8.17 MiniMate PlusBattery Level6.3 VoltsUnit CalibrationFebruary 22, 2017 by InstantelFile NameT695GVLE.D50

Extended Notes

Combo Mode May 4, 2017 14:33:32 Geophone anchored into subgrade, and sandbagged





Histogram Start Time Histogram Finish Time 18:00:00 May 3, 2017 Number of Intervals Range Sample Rate

17:09:43 May 3, 2017 1508.00 at 2 seconds Geo:254.0 mm/s 2048sps

Serial Number BE8719 V 10.72-8.17 MiniMate Plus 6.3 Volts **Battery Level** Unit Calibration March 13, 2017 by Instantel File Name J719GVJP.070

Notes

Location:	T5, North Kent
Client:	Pattern
User Name:	Golder Associates Ltd.
General:	30m, 1668031

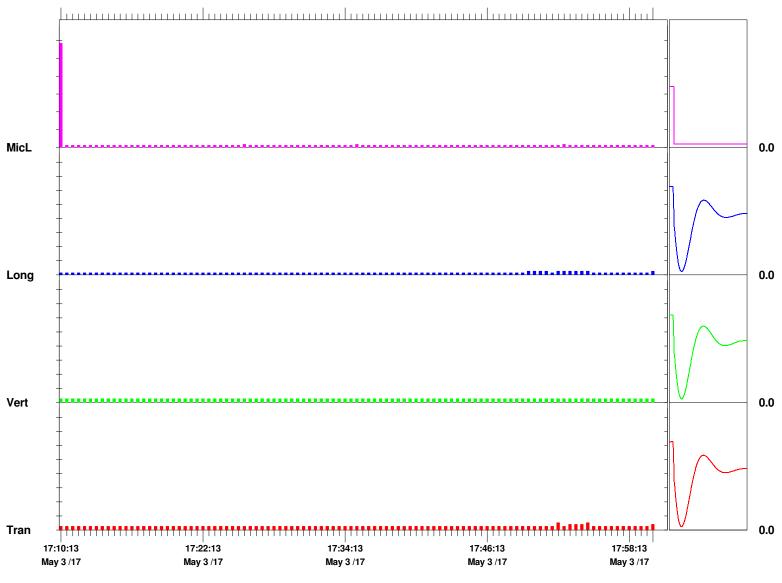
Extended Notes

Geophone anchored into subgrade and sandbagged

Microphone Linear Weighting 29.25 pa.(L) on May 3, 2017 at 17:09:51 PSPL **ZC Freq** 4.4 Hz Channel Test Check (Freq = 0.0 Hz Amp = 0 mv)

	Tran	Vert	Long	
PPV	0.508	0.254	0.254	mm/s
ZC Freq	15.1	>200	>200	Hz
Date	May 3 /17	May 3 /17	May 3 /17	
Time	17:52:11	17:09:45	17:49:41	
Sensor Check	Passed	Passed	Passed	
Frequency	7.3	7.4	7.2	Hz
Overswing Ratio	4.1	3.7	4.2	

Peak Vector Sum 0.539 mm/s on May 3, 2017 at 17:52:11



Time Scale: 30 seconds /div Amplitude Scale: Geo: 1.000 mm/s/div Mic: 5.000 pa.(L)/div



Histogram Start Time Histogram Finish Time 15:33:26 May 4, 2017 Number of Intervals Range Sample Rate

14:35:24 May 4, 2017 1740.00 at 2 seconds Geo:254.0 mm/s 2048sps

Serial Number BE8719 V 10.72-8.17 MiniMate Plus **Battery Level** 6.2 Volts Unit Calibration March 13, 2017 by Instantel File Name J719GVLC.J00

Notes

Location:	T5, North Kent
Client:	Pattern
User Name:	Golder Associates Ltd.
General:	30m, 1668031

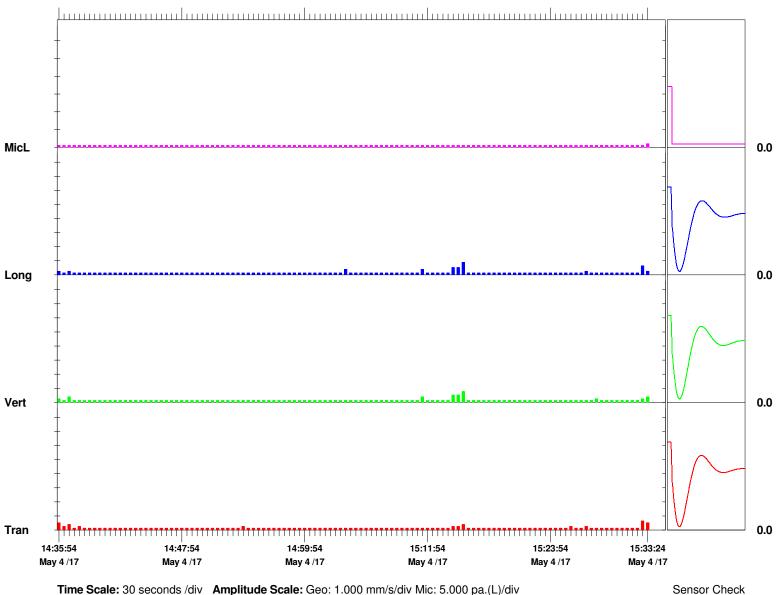
Extended Notes

Geophone anchored into subgrade and sandbagged

Microphone	Linear Weighting
PSPL	1.000 pa.(L) on May 4, 2017 at 15:33:12
ZC Freq	41 Hz
Channel Test	Check (Freq = 0.0 Hz Amp = 0 mv)

Tran	Vert	Long	
0.635	0.762	0.889	mm/s
102	39	37	Hz
May 4 /17	May 4 /17	May 4 /17	
15:32:34	15:15:04	15:15:04	
Passed	Passed	Passed	
7.3	7.4	7.1	Hz
4.2	3.8	4.3	
	0.635 102 May 4 /17 15:32:34 Passed 7.3	0.635 0.762 102 39 May 4 /17 May 4 /17 15:32:34 15:15:04 Passed Passed 7.3 7.4	0.635 0.762 0.889 102 39 37 May 4 /17 May 4 /17 May 4 /17 15:32:34 15:15:04 15:15:04 Passed Passed Passed 7.3 7.4 7.1

Peak Vector Sum 0.976 mm/s on May 4, 2017 at 15:15:04



Printed: May 12, 2017 (V 10.72 - 10.72.1)



Histogram Start Time Histogram Finish Time Number of Intervals Range Sample Rate

17:09:27 May 3, 2017 18:48:27 May 3, 2017 2970.00 at 2 seconds Geo:254.0 mm/s 2048sps

Serial Number **Battery Level** Unit Calibration File Name

BE8720 V 10.72-8.17 MiniMate Plus 5.7 Volts (Battery Very Low) March 13, 2017 by Instantel J720GVJO.ZR0

Notes

Location:	Т5
Client:	Pattern
User Name:	Golder Associates Ltd.
General:	3m,1668031

Extended Notes

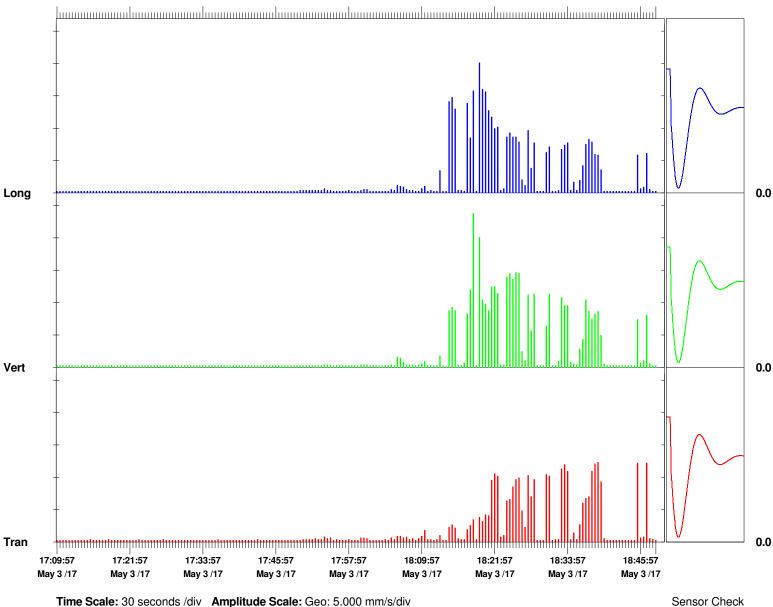
Geophone secured into subgrade under sandbag.

	Tran	Vert	Long	
PPV	12.32	23.75	20.07	mm/s
ZC Freq	31	16.3	24	Hz
Date	May 3 /17	May 3 /17	May 3 /17	
Time	18:38:33	18:18:03	18:19:03	
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.6	7.4	Hz
Overswing Ratio	3.4	3.6	3.9	

Peak Vector Sum 23.90 mm/s on May 3, 2017 at 18:18:03

Monitor Log

May 3 /17 17:09:26 May 3 /17 18:48:27 Event recorded. (Battery Low Exit)



Printed: May 12, 2017 (V 10.72 - 10.72.1)

Format © 1995-2015 Xmark Corporation

Sensor Check



Velocity (mm/s)

 Date/Time
 Vert at 18:05:56 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Notes

Location: T5 Client: Pattern User Name: Golder Associates Ltd. General: 3m,1668031

Extended Notes

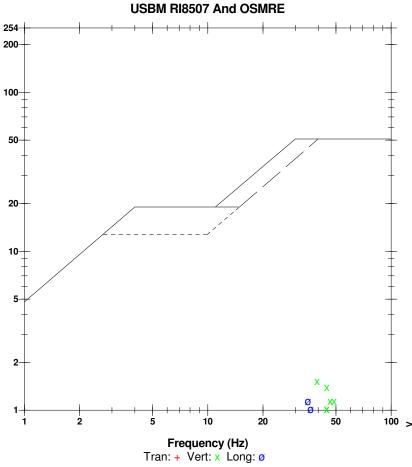
Combo Mode May 3, 2017 17:09:26 Geophone secured into subgrade under sandbag.

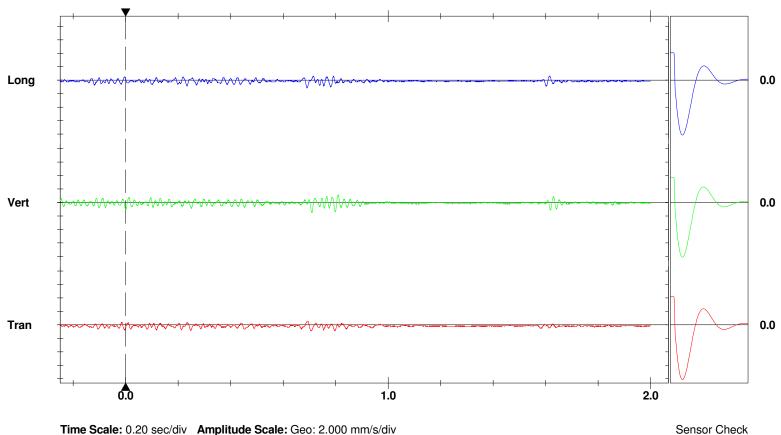
	Tran	Vert	Long	
PPV	0.889	1.524	1.143	mm/s
ZC Freq	38	39	35	Hz
Time (Rel. to Trig)	0.248	0.708	0.690	sec
Peak Acceleration	0.053	0.053	0.053	g
Peak Displacement	0.005	0.006	0.005	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.6	7.4	Hz
Overswing Ratio	3.4	3.6	3.9	

Peak Vector Sum 1.723 mm/s at 0.709 sec

Serial Number Battery Level Unit Calibration File Name

BE8720 V 10.72-8.17 MiniMate Plus 5.9 Volts (Battery Low) March 13, 2017 by Instantel J720GVJR.LW0







 Date/Time
 Vert at 18:05:56 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

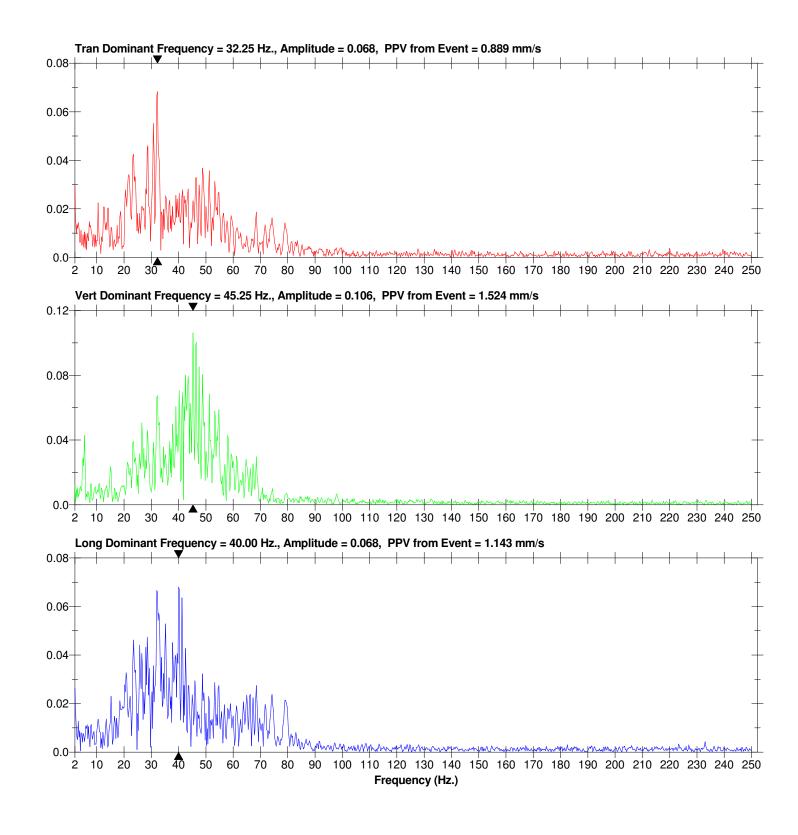
 Record Time
 2.0 sec at 2048 sps

Notes

Location: T5 Client: Pattern User Name: Golder Associates Ltd. General: 3m,1668031 Serial NumberBE8720 V 10.72-8.17 MiniMate PlusBattery Level5.9 Volts (Battery Low)Unit CalibrationMarch 13, 2017 by InstantelFile NameJ720GVJR.LW0

Extended Notes

Combo Mode May 3, 2017 17:09:26 Geophone secured into subgrade under sandbag.





Velocity (mm/s)

Tran at 18:10:02 May 3, 2017 Date/Time Trigger Source Geo: 1.000 mm/s Geo: 254.0 mm/s Range Record Time 2.0 sec at 2048 sps

Notes

Location: Τ5 Pattern Client: User Name: Golder Associates Ltd. 3m,1668031 General:

Extended Notes

Combo Mode May 3, 2017 17:09:26 Geophone secured into subgrade under sandbag.

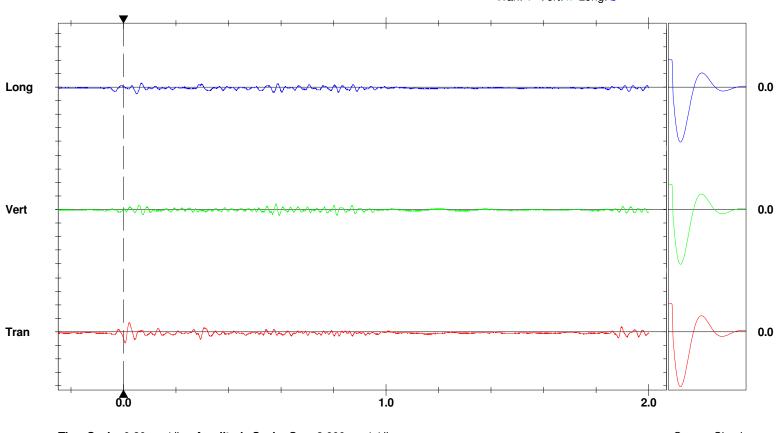
	Tran	Vert	Long	
PPV	1.651	0.889	1.016	mm/s
ZC Freq	22	45	23	Hz
Time (Rel. to Trig)	0.006	0.581	0.051	sec
Peak Acceleration	0.053	0.053	0.053	g
Peak Displacement	0.011	0.004	0.007	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.6	7.4	Hz
Overswing Ratio	3.4	3.6	3.9	

Peak Vector Sum 1.661 mm/s at 0.006 sec

Serial Number **Battery Level** File Name

BE8720 V 10.72-8.17 MiniMate Plus 5.9 Volts (Battery Low) Unit Calibration March 13, 2017 by Instantel J720GVJR.SQ0

USBM RI8507 And OSMRE 254-+ + + + 200-100-50 20 10-2 1 2 10 50 100 > 5 20 Frequency (Hz) Tran: + Vert: x Long: Ø



Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger = ► 4



 Date/Time
 Tran at 18:10:02 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

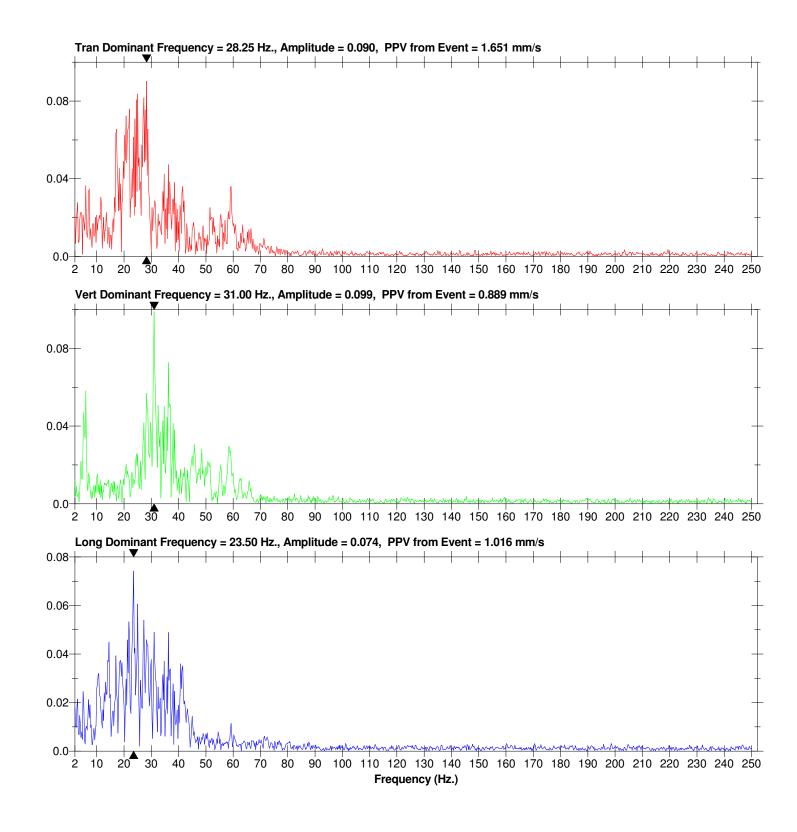
Notes

Location:	Т5
Client:	Pattern
User Name:	Golder Associates Ltd.
General:	3m,1668031

Serial NumberBE8720 V 10.72-8.17 MiniMate PlusBattery Level5.9 Volts (Battery Low)Unit CalibrationMarch 13, 2017 by InstantelFile NameJ720GVJR.SQ0

Extended Notes

Combo Mode May 3, 2017 17:09:26 Geophone secured into subgrade under sandbag.





Velocity (mm/s)

 Date/Time
 Tran at 18:10:05 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Notes

Location: T5 Client: Pattern User Name: Golder Associates Ltd. General: 3m,1668031

Extended Notes

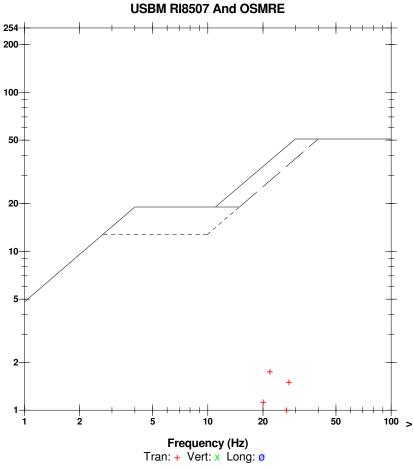
Combo Mode May 3, 2017 17:09:26 Geophone secured into subgrade under sandbag.

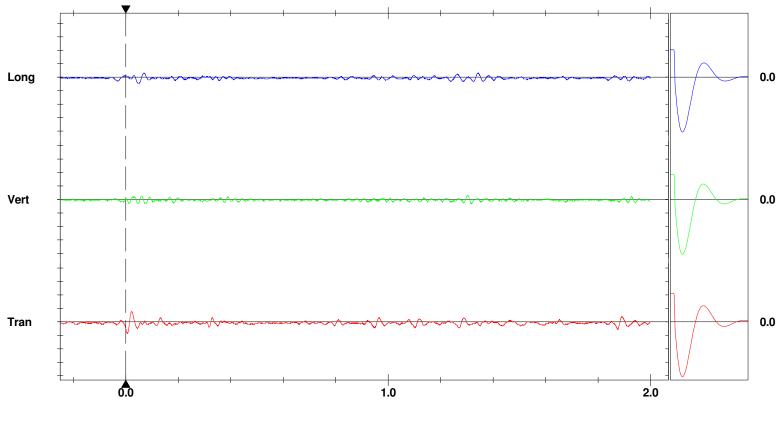
	Tran	Vert	Long	
PPV	1.778	0.635	0.889	mm/s
ZC Freq	22	39	23	Hz
Time (Rel. to Trig)	0.006	0.012	0.045	sec
Peak Acceleration	0.053	0.027	0.053	g
Peak Displacement	0.011	0.004	0.007	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.6	7.4	Hz
Overswing Ratio	3.4	3.6	3.9	

Peak Vector Sum 1.801 mm/s at 0.006 sec

Serial Number Battery Level Unit Calibration File Name

BE8720 V 10.72-8.17 MiniMate Plus 5.9 Volts (Battery Low) March 13, 2017 by Instantel J720GVJR.ST0





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger = ► ____ ←



 Date/Time
 Tran at 18:10:05 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

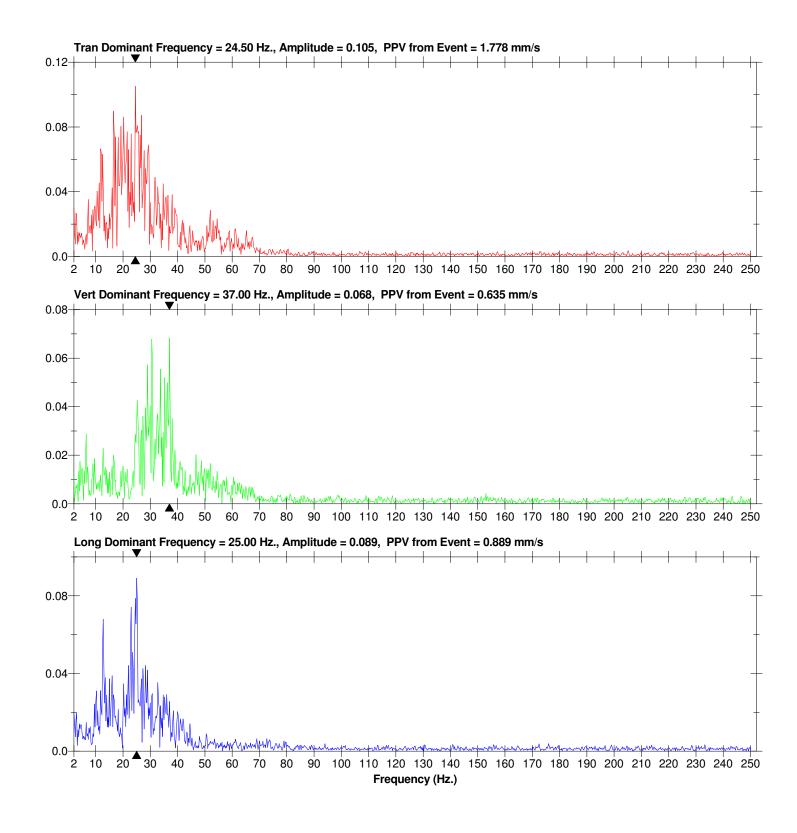
Notes

Location:	T5
Client:	Pattern
User Name:	Golder Associates Ltd.
General:	3m,1668031

Serial NumberBE8720 V 10.72-8.17 MiniMate PlusBattery Level5.9 Volts (Battery Low)Unit CalibrationMarch 13, 2017 by InstantelFile NameJ720GVJR.ST0

Extended Notes

Combo Mode May 3, 2017 17:09:26 Geophone secured into subgrade under sandbag.





Velocity (mm/s)

 Date/Time
 Tran at 18:10:07 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Notes

Location: T5 Client: Pattern User Name: Golder Associates Ltd. General: 3m,1668031

Extended Notes

Combo Mode May 3, 2017 17:09:26 Geophone secured into subgrade under sandbag.

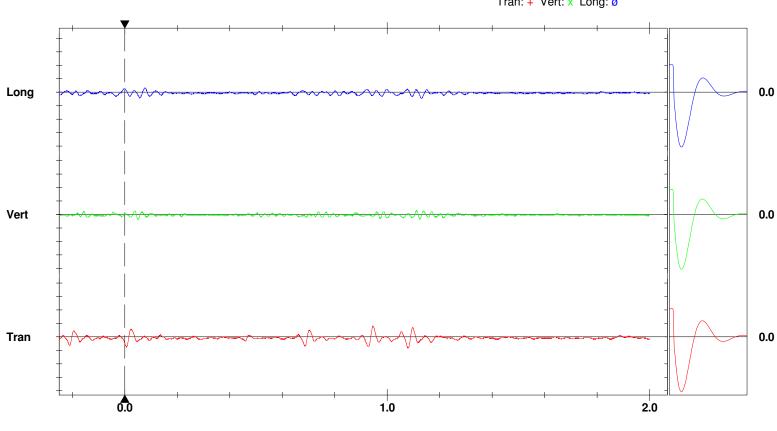
	Tran	Vert	Long	
PPV	1.651	0.762	0.889	mm/s
ZC Freq	26	41	26	Hz
Time (Rel. to Trig)	0.945	0.050	0.057	sec
Peak Acceleration	0.053	0.027	0.053	g
Peak Displacement	0.012	0.004	0.006	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.6	7.4	Hz
Overswing Ratio	3.4	3.6	3.9	

Peak Vector Sum 1.746 mm/s at 0.945 sec

Serial Number Battery Level Unit Calibration File Name

BE8720 V 10.72-8.17 MiniMate Plus 5.9 Volts (Battery Low) March 13, 2017 by Instantel J720GVJR.SV0

USBM RI8507 And OSMRE 254-+ + + + 200-100-50 20 10-2 1 2 10 50 100 > 5 20 Frequency (Hz) Tran: + Vert: x Long: Ø



Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger = ▶ ____ ◀



 Date/Time
 Tran at 18:10:07 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

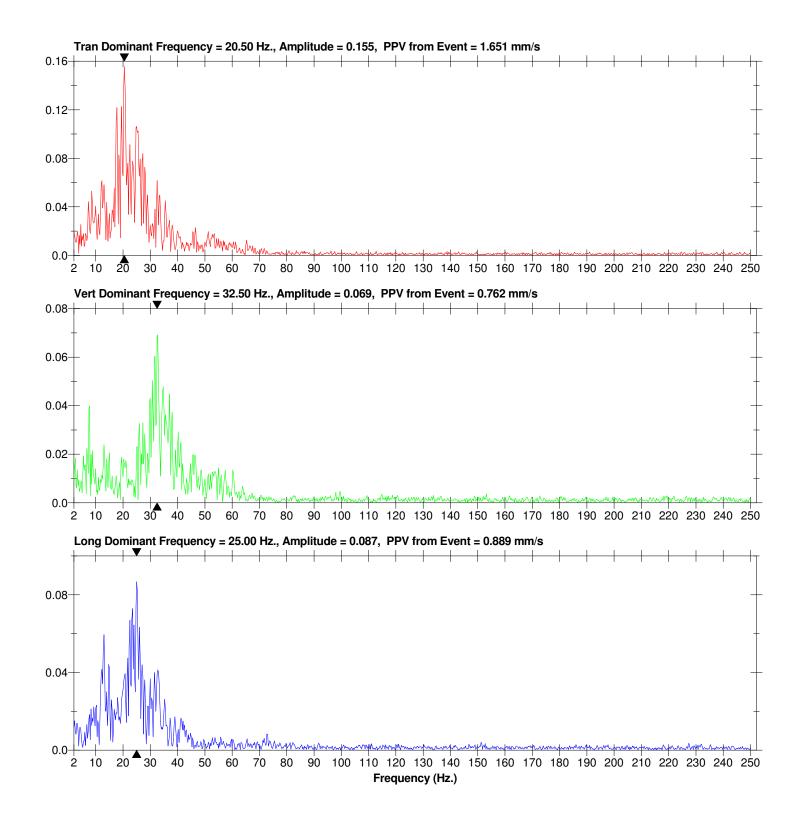
 Record Time
 2.0 sec at 2048 sps

Notes

Location: T5 Client: Pattern User Name: Golder Associates Ltd. General: 3m,1668031 Serial NumberBE8720 V 10.72-8.17 MiniMate PlusBattery Level5.9 Volts (Battery Low)Unit CalibrationMarch 13, 2017 by InstantelFile NameJ720GVJR.SV0

Extended Notes

Combo Mode May 3, 2017 17:09:26 Geophone secured into subgrade under sandbag.





Velocity (mm/s)

 Date/Time
 Long at 18:12:36 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Notes

Location: T5 Client: Pattern User Name: Golder Associates Ltd. General: 3m,1668031

Extended Notes

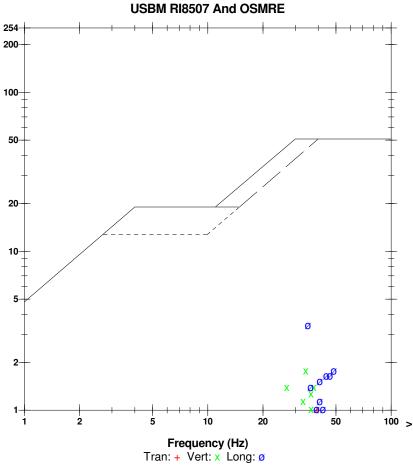
Combo Mode May 3, 2017 17:09:26 Geophone secured into subgrade under sandbag.

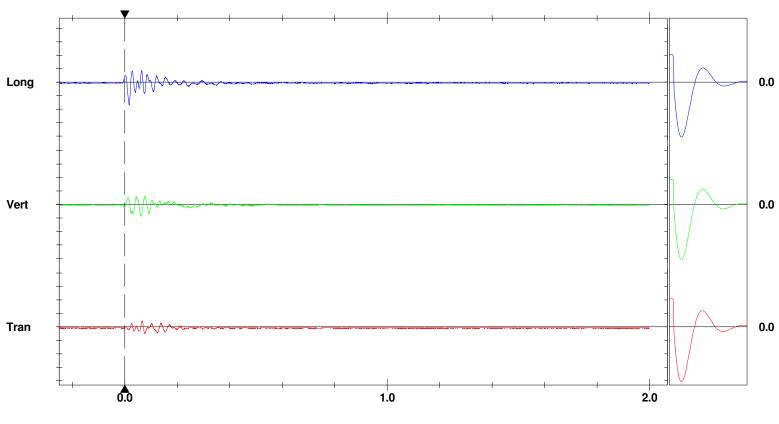
	Tran	Vert	Long	
PPV	1.016	1.778	3.429	mm/s
ZC Freq	39	34	35	Hz
Time (Rel. to Trig)	0.075	0.061	0.017	sec
Peak Acceleration	0.053	0.053	0.080	g
Peak Displacement	0.006	0.010	0.014	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.6	7.4	Hz
Overswing Ratio	3.4	3.6	3.9	

Peak Vector Sum 3.497 mm/s at 0.017 sec

Serial Number Battery Level Unit Calibration File Name

BE8720 V 10.72-8.17 MiniMate Plus 5.9 Volts (Battery Low) March 13, 2017 by Instantel J720GVJR.X00





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger = ► ____



 Date/Time
 Long at 18:12:36 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

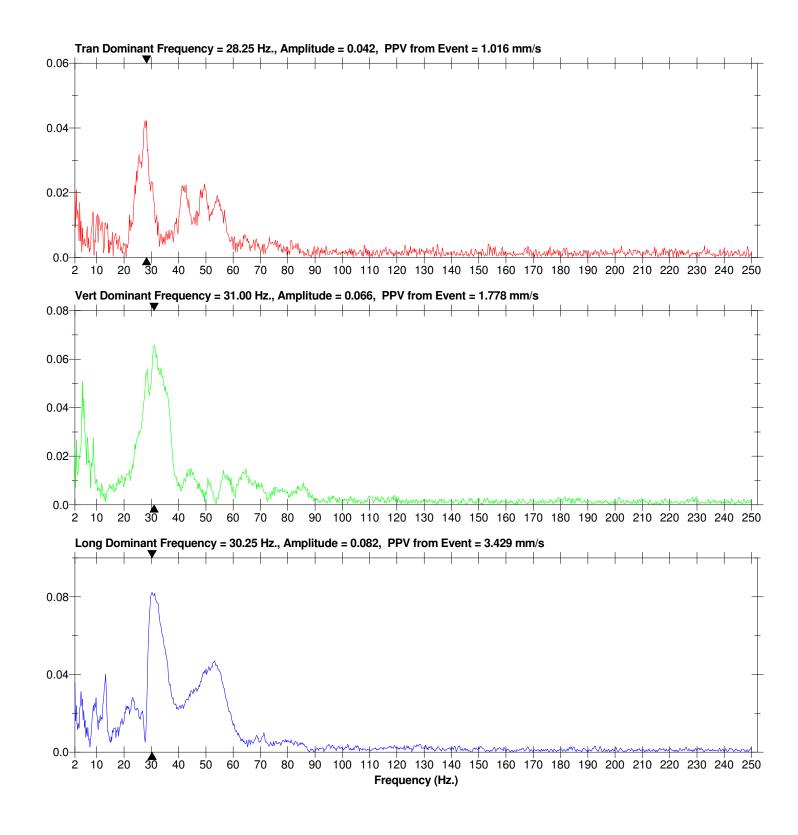
 Record Time
 2.0 sec at 2048 sps

Notes

Location: T5 Client: Pattern User Name: Golder Associates Ltd. General: 3m,1668031 Serial NumberBE8720 V 10.72-8.17 MiniMate PlusBattery Level5.9 Volts (Battery Low)Unit CalibrationMarch 13, 2017 by InstantelFile NameJ720GVJR.X00

Extended Notes

Combo Mode May 3, 2017 17:09:26 Geophone secured into subgrade under sandbag.





Velocity (mm/s)

 Date/Time
 Vert at 18:14:26 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Notes

Location: T5 Client: Pattern User Name: Golder Associates Ltd. General: 3m,1668031

Extended Notes

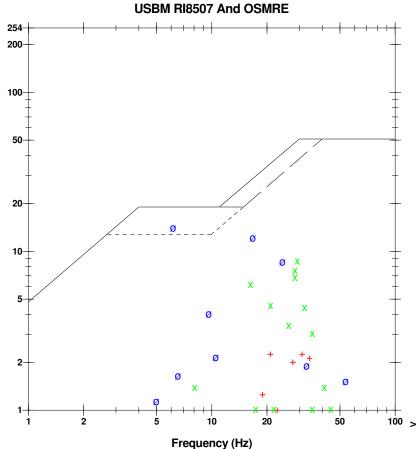
Combo Mode May 3, 2017 17:09:26 Geophone secured into subgrade under sandbag.

	Tran	Vert	Long	
PPV	2.286	8.763	14.10	mm/s
ZC Freq	21	29	6.2	Hz
Time (Rel. to Trig)	0.407	0.366	0.357	sec
Peak Acceleration	0.080	0.239	0.186	g
Peak Displacement	0.018	0.059	0.131	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.6	7.4	Hz
Overswing Ratio	3.4	3.6	3.9	

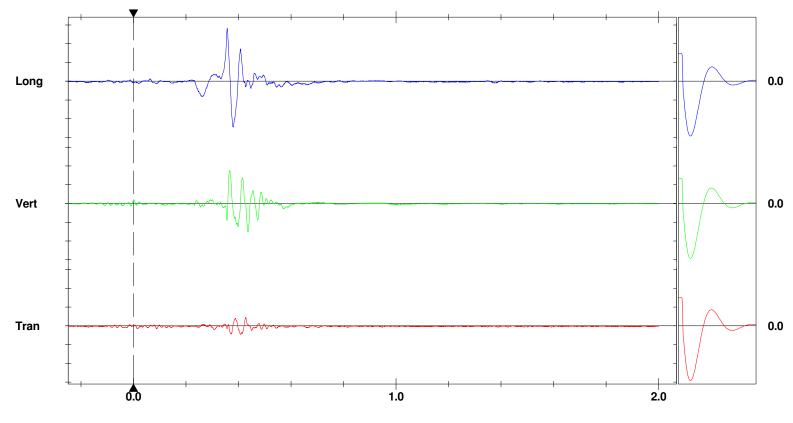
Peak Vector Sum 14.80 mm/s at 0.357 sec



BE8720 V 10.72-8.17 MiniMate Plus 5.8 Volts (Battery Low) March 13, 2017 by Instantel J720GVJS.020



Tran: + Vert: x Long: ø



Time Scale: 0.20 sec/div Amplitude Scale: Geo: 5.000 mm/s/div Trigger =



 Date/Time
 Vert at 18:14:26 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

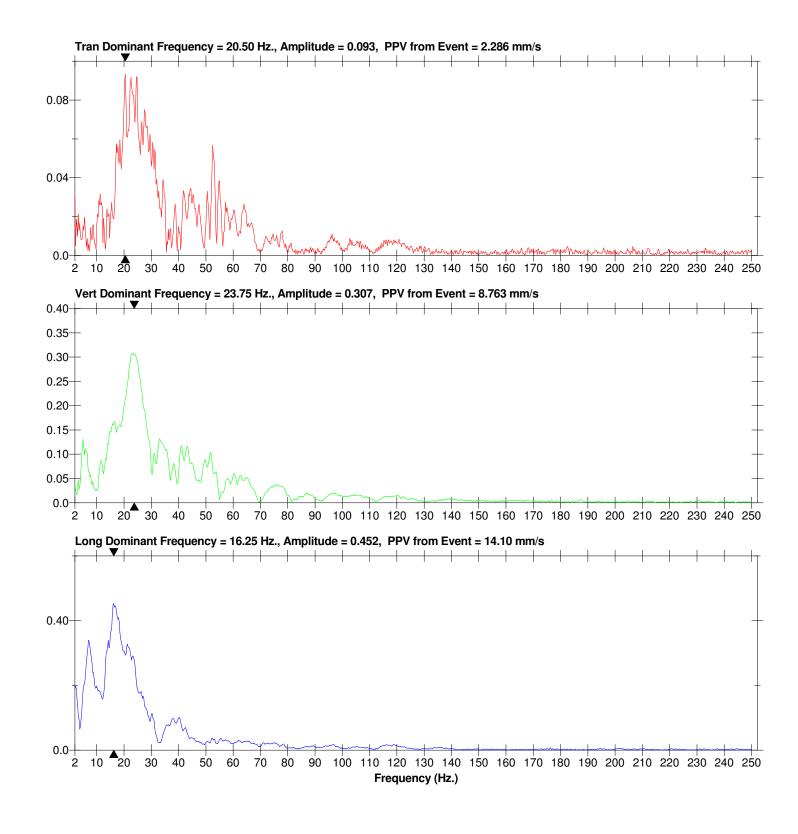
Notes

Location:	Т5
Client:	Pattern
User Name:	Golder Associates Ltd.
General:	3m,1668031

Serial NumberBE8720 V 10.72-8.17 MiniMate PlusBattery Level5.8 Volts (Battery Low)Unit CalibrationMarch 13, 2017 by InstantelFile NameJ720GVJS.020

Extended Notes

Combo Mode May 3, 2017 17:09:26 Geophone secured into subgrade under sandbag.





Velocity (mm/s)

 Date/Time
 Vert at 18:14:30 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Notes

Location: T5 Client: Pattern User Name: Golder Associates Ltd. General: 3m,1668031

Extended Notes

Combo Mode May 3, 2017 17:09:26 Geophone secured into subgrade under sandbag.

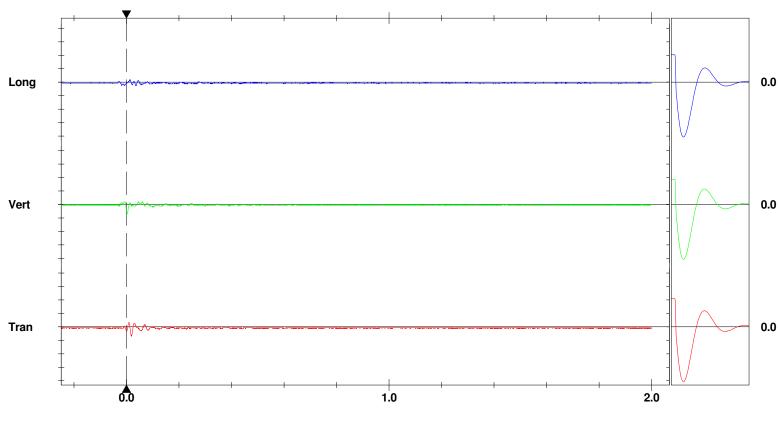
	Tran	Vert	Long	
PPV	1.397	1.397	0.508	mm/s
ZC Freq	47	43	24	Hz
Time (Rel. to Trig)	0.019	0.001	-0.019	sec
Peak Acceleration	0.053	0.053	0.053	g
Peak Displacement	0.005	0.005	0.003	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.6	7.4	Hz
Overswing Ratio	3.4	3.6	3.9	

Peak Vector Sum 1.535 mm/s at 0.001 sec

Serial Number Battery Level Unit Calibration File Name

BE8720 V 10.72-8.17 MiniMate Plus 5.8 Volts (Battery Low) March 13, 2017 by Instantel J720GVJS.060

USBM RI8507 And OSMRE 254-++++200-100-50 20 10-2 **X**+ 1 2 5 10 20 50 100 > Frequency (Hz) Tran: + Vert: x Long: Ø



Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger =



 Date/Time
 Vert at 18:14:30 May 3, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

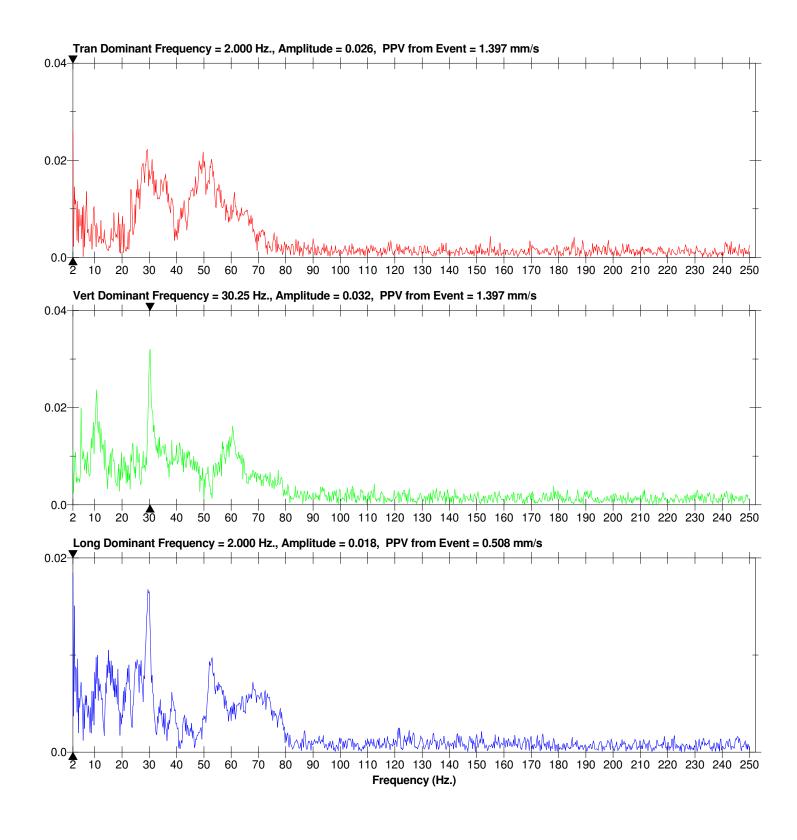
 Record Time
 2.0 sec at 2048 sps

Notes

Location: T5 Client: Pattern User Name: Golder Associates Ltd. General: 3m,1668031 Serial NumberBE8720 V 10.72-8.17 MiniMate PlusBattery Level5.8 Volts (Battery Low)Unit CalibrationMarch 13, 2017 by InstantelFile NameJ720GVJS.060

Extended Notes

Combo Mode May 3, 2017 17:09:26 Geophone secured into subgrade under sandbag.





Histogram Start Time Histogram Finish Time 15:32:45 May 4, 2017 Number of Intervals Range Sample Rate

14:34:31 May 4, 2017 1747.00 at 2 seconds Geo:254.0 mm/s 2048sps

Serial Number **Battery Level** Unit Calibration File Name

BE8720 V 10.72-8.17 MiniMate Plus 5.9 Volts (Battery Low) March 13, 2017 by Instantel J720GVLC.HJ0

Notes

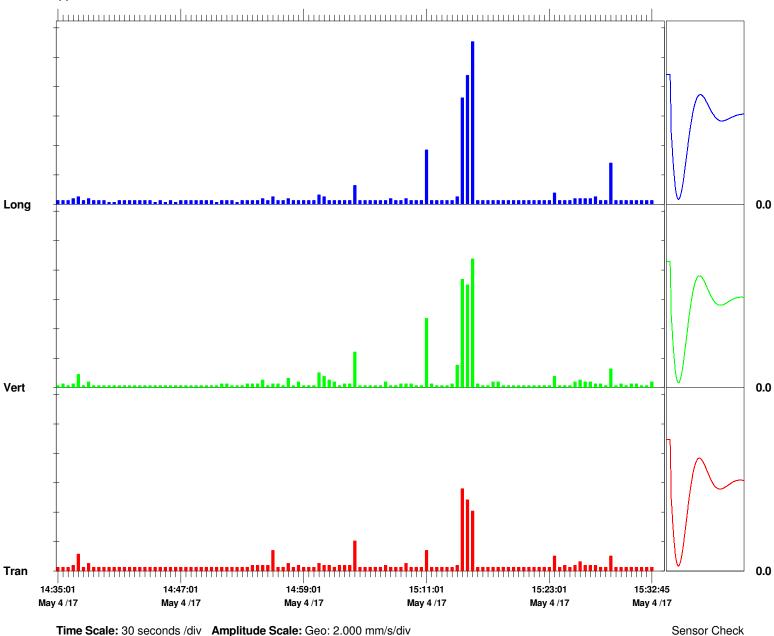
Location:	Т5
Client:	Pattern
User Name:	Golder Associates Ltd.
General:	3m,1668031

Extended Notes

Geophone secured into subgrade under sandbag.

	Tran	Vert	Long	
PPV	5.588	8.763	11.05	mm/s
ZC Freq	37	N/A	15.3	Hz
Date	May 4 /17	May 4 /17	May 4 /17	
Time	15:14:11	15:15:11	15:15:05	
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.6	7.3	Hz
Overswing Ratio	3.5	3.6	4.0	

Peak Vector Sum 11.28 mm/s on May 4, 2017 at 15:15:05 N/A: Not Applicable





Velocity (mm/s)

Date/Time Tran at 14:36:31 May 4, 2017 Trigger Source Geo: 1.000 mm/s Range Geo: 254.0 mm/s Record Time 2.0 sec at 2048 sps

Notes

Location: Τ5 Pattern Client: User Name: Golder Associates Ltd. 3m,1668031 General:

Extended Notes

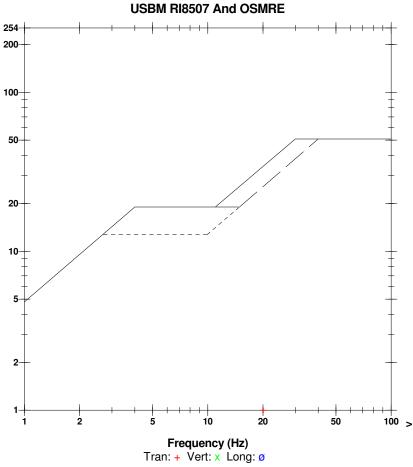
Combo Mode May 4, 2017 14:34:31 Geophone secured into subgrade under sandbag.

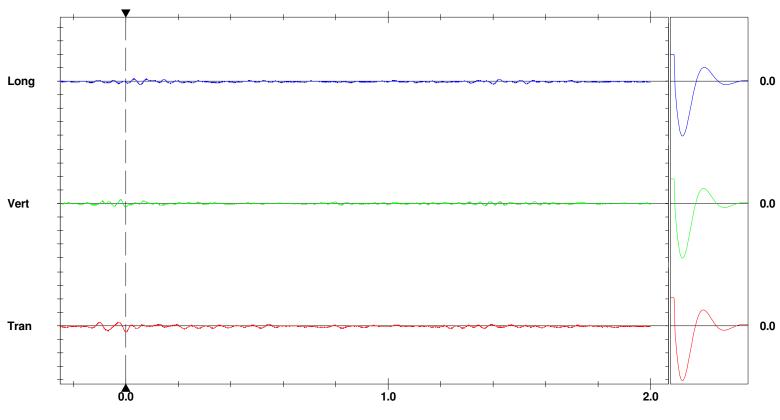
	Tran	Vert	Long	
PPV	1.016	0.635	0.508	mm/s
ZC Freq	20	30	26	Hz
Time (Rel. to Trig)	0.000	-0.019	0.051	sec
Peak Acceleration	0.027	0.027	0.027	g
Peak Displacement	0.009	0.004	0.004	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.6	7.3	Hz
Overswing Ratio	3.5	3.6	4.0	

Peak Vector Sum 1.143 mm/s at 0.000 sec

Serial Number BE8720 V 10.72-8.17 MiniMate Plus **Battery Level** 5.9 Volts File Name

Unit Calibration March 13, 2017 by Instantel J720GVLC.KV0





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger = ► 4



 Date/Time
 Tran at 14:36:31 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

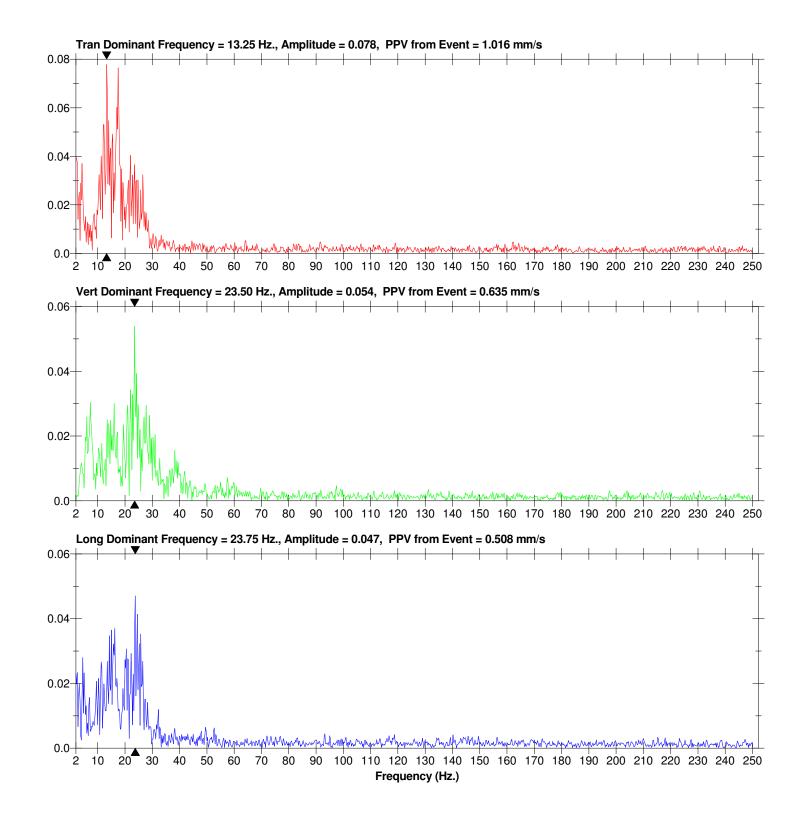
 Record Time
 2.0 sec at 2048 sps

Notes

Location: T5 Client: Pattern User Name: Golder Associates Ltd. General: 3m,1668031 Serial NumberBE8720 V 10.72-8.17 MiniMate PlusBattery Level5.9 VoltsUnit CalibrationMarch 13, 2017 by InstantelFile NameJ720GVLC.KV0

Extended Notes

Combo Mode May 4, 2017 14:34:31 Geophone secured into subgrade under sandbag.





Velocity (mm/s)

Date/Time Tran at 14:36:33 May 4, 2017 Trigger Source Geo: 1.000 mm/s Range Geo: 254.0 mm/s Record Time 2.0 sec at 2048 sps

Notes

Location: Τ5 Pattern Client: User Name: Golder Associates Ltd. 3m,1668031 General:

Extended Notes

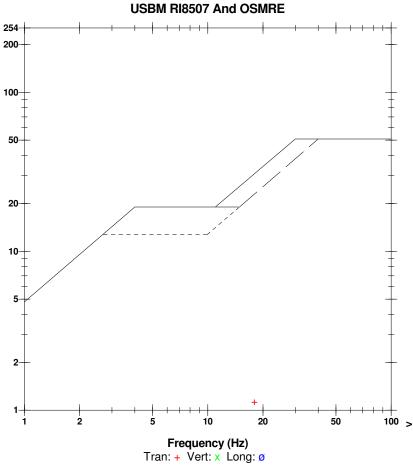
Combo Mode May 4, 2017 14:34:31 Geophone secured into subgrade under sandbag.

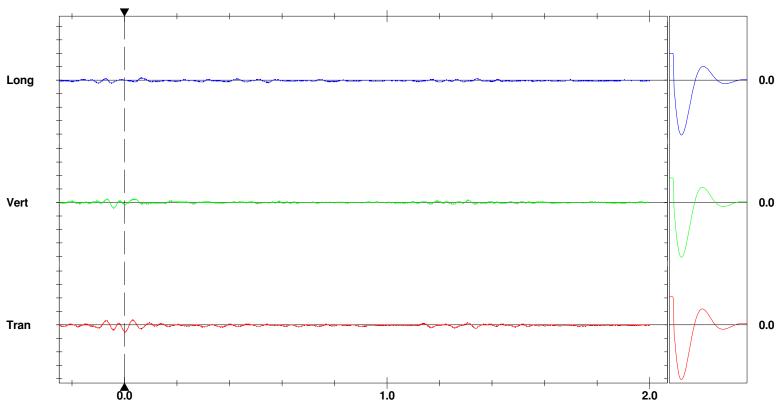
	Tran	Vert	Long	
PPV	1.143	0.889	0.508	mm/s
ZC Freq	18.0	20	21	Hz
Time (Rel. to Trig)	0.001	-0.042	-0.100	sec
Peak Acceleration	0.027	0.053	0.053	g
Peak Displacement	0.010	0.007	0.004	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.6	7.3	Hz
Overswing Ratio	3.5	3.6	4.0	

Peak Vector Sum 1.205 mm/s at 0.002 sec

Serial Number **Battery Level** 5.9 Volts File Name

BE8720 V 10.72-8.17 MiniMate Plus Unit Calibration March 13, 2017 by Instantel J720GVLC.KX0





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger = ► 4



 Date/Time
 Tran at 14:36:33 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

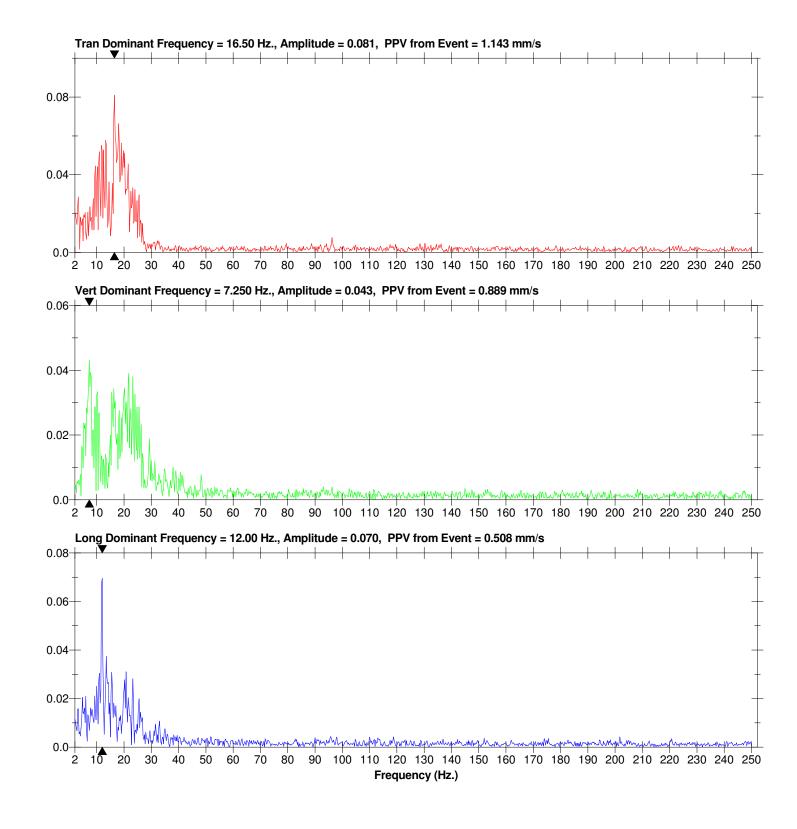
 Record Time
 2.0 sec at 2048 sps

Notes

Location: T5 Client: Pattern User Name: Golder Associates Ltd. General: 3m,1668031 Serial NumberBE8720 V 10.72-8.17 MiniMate PlusBattery Level5.9 VoltsUnit CalibrationMarch 13, 2017 by InstantelFile NameJ720GVLC.KX0

Extended Notes

Combo Mode May 4, 2017 14:34:31 Geophone secured into subgrade under sandbag.





Velocity (mm/s)

 Date/Time
 Tran at 14:55:48 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Notes

Location: T5 Client: Pattern User Name: Golder Associates Ltd. General: 3m,1668031

Extended Notes

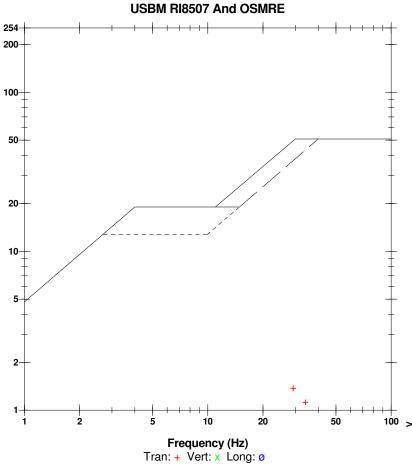
Combo Mode May 4, 2017 14:34:31 Geophone secured into subgrade under sandbag.

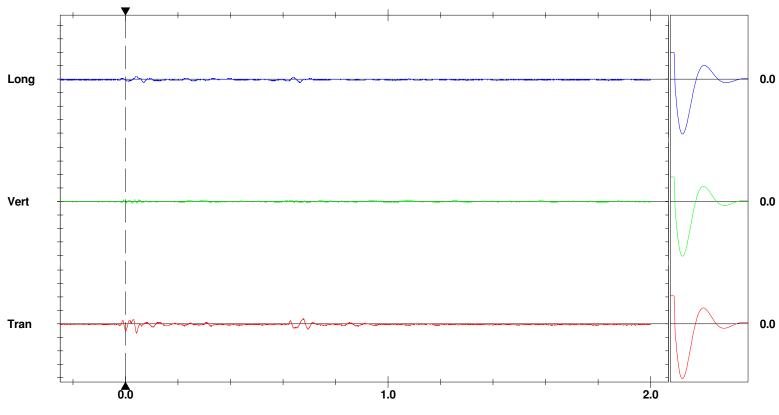
	Tran	Vert	Long	
PPV	1.397	0.254	0.508	mm/s
ZC Freq	29	>200	34	Hz
Time (Rel. to Trig)	0.042	-0.008	0.067	sec
Peak Acceleration	0.053	0.053	0.053	g
Peak Displacement	0.007	0.000	0.003	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.6	7.3	Hz
Overswing Ratio	3.5	3.6	4.0	

Peak Vector Sum 1.470 mm/s at 0.042 sec

Serial Number Battery Level Unit Calibration File Name

BE8720 V 10.72-8.17 MiniMate Plus 5.9 Volts (Battery Low) March 13, 2017 by Instantel J720GVLD.H00





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger = ► ____



 Date/Time
 Tran at 14:55:48 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

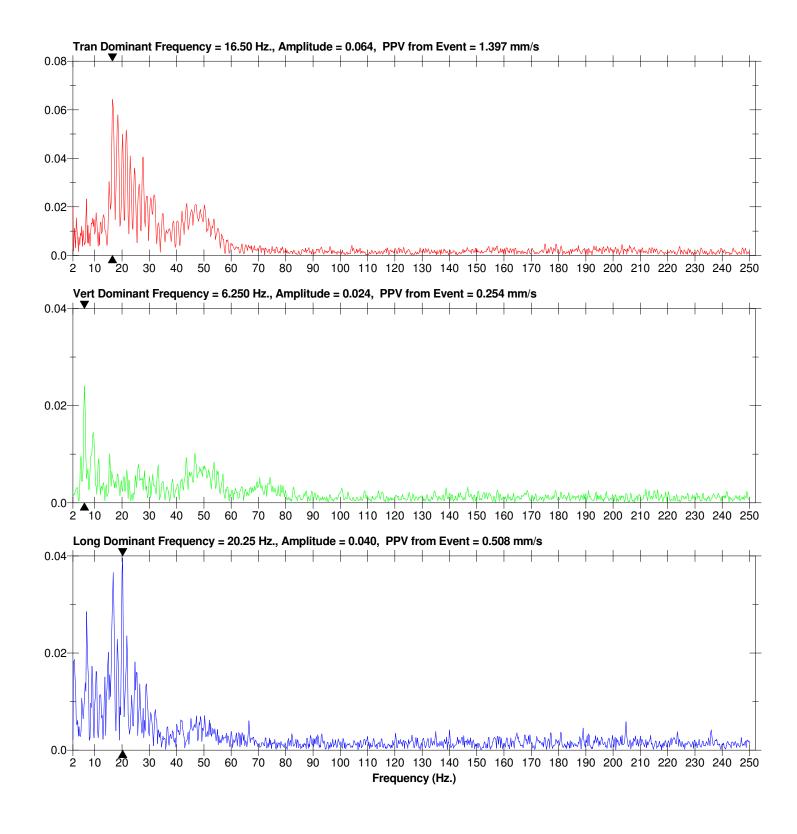
 Record Time
 2.0 sec at 2048 sps

Notes

Location: T5 Client: Pattern User Name: Golder Associates Ltd. General: 3m,1668031 Serial NumberBE8720 V 10.72-8.17 MiniMate PlusBattery Level5.9 Volts (Battery Low)Unit CalibrationMarch 13, 2017 by InstantelFile NameJ720GVLD.H00

Extended Notes

Combo Mode May 4, 2017 14:34:31 Geophone secured into subgrade under sandbag.





Velocity (mm/s)

Date/Time Vert at 15:00:28 May 4, 2017 Trigger Source Geo: 1.000 mm/s Geo: 254.0 mm/s Range Record Time 2.0 sec at 2048 sps

Notes

Location: Τ5 Pattern Client: User Name: Golder Associates Ltd. 3m,1668031 General:

Extended Notes

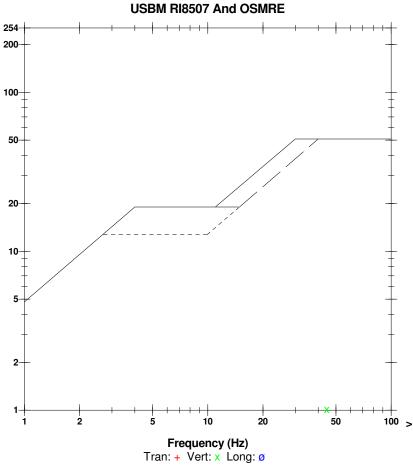
Combo Mode May 4, 2017 14:34:31 Geophone secured into subgrade under sandbag.

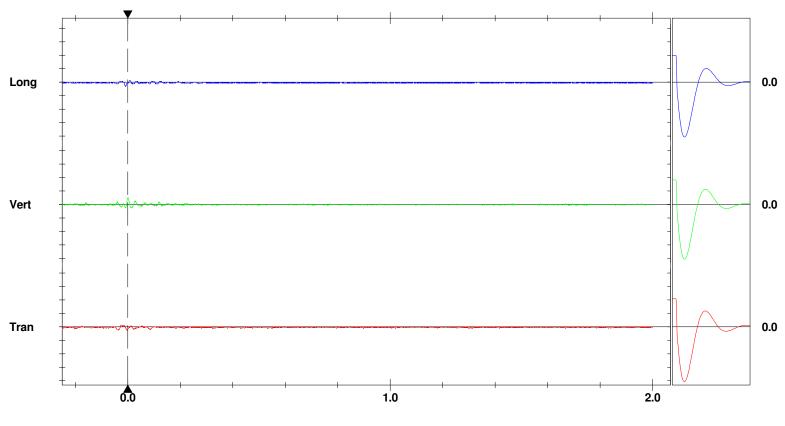
	Tran	Vert	Long	
PPV	0.508	1.016	0.635	mm/s
ZC Freq	35	45	51	Hz
Time (Rel. to Trig)	-0.036	0.000	-0.010	sec
Peak Acceleration	0.053	0.053	0.027	g
Peak Displacement	0.002	0.004	0.002	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.6	7.3	Hz
Overswing Ratio	3.5	3.6	4.0	

Peak Vector Sum 1.092 mm/s at 0.000 sec

Serial Number **Battery Level** File Name

BE8720 V 10.72-8.17 MiniMate Plus 5.9 Volts (Battery Low) Unit Calibration March 13, 2017 by Instantel J720GVLD.OS0





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger = ► 4



 Date/Time
 Vert at 15:00:28 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

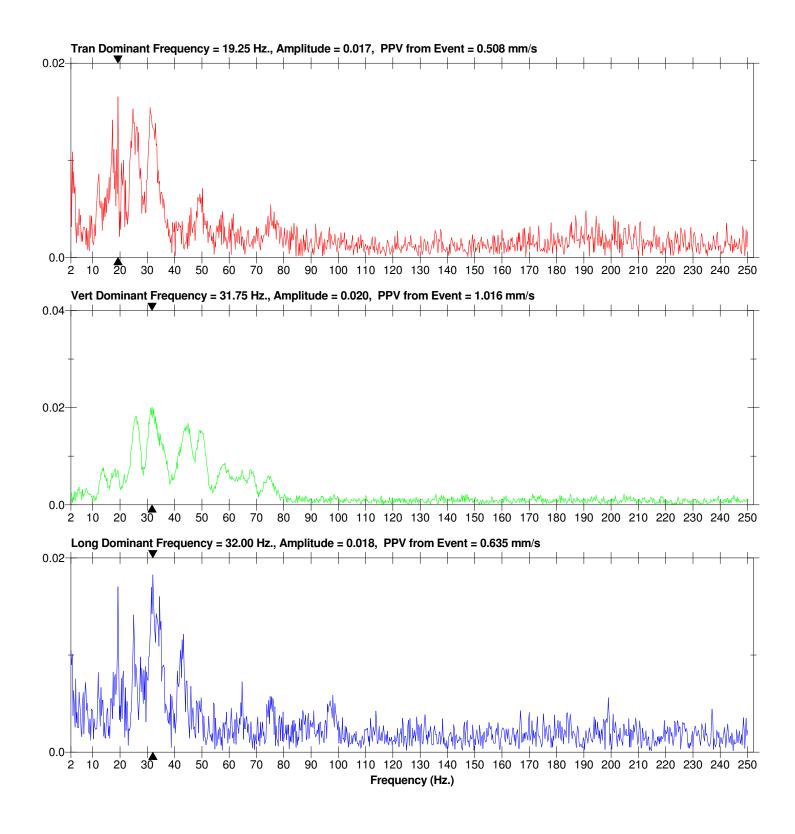
Notes

Location:	T5
Client:	Pattern
User Name:	Golder Associates Ltd.
General:	3m,1668031

Serial NumberBE8720 V 10.72-8.17 MiniMate PlusBattery Level5.9 Volts (Battery Low)Unit CalibrationMarch 13, 2017 by InstantelFile NameJ720GVLD.OS0

Extended Notes

Combo Mode May 4, 2017 14:34:31 Geophone secured into subgrade under sandbag.





Velocity (mm/s)

 Date/Time
 Tran at 15:03:32 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Notes

Location: T5 Client: Pattern User Name: Golder Associates Ltd. General: 3m,1668031

Extended Notes

Combo Mode May 4, 2017 14:34:31 Geophone secured into subgrade under sandbag.

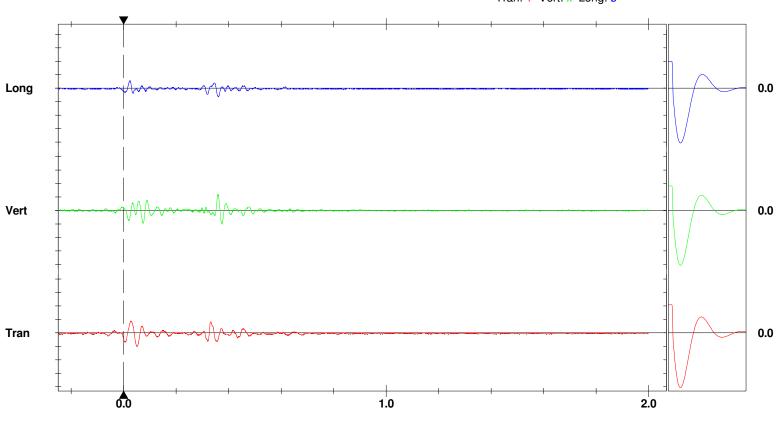
	Tran	Vert	Long	
PPV	2.032	2.413	1.270	mm/s
ZC Freq	23	34	32	Hz
Time (Rel. to Trig)	0.049	0.359	0.359	sec
Peak Acceleration	0.053	0.053	0.080	g
Peak Displacement	0.014	0.011	0.007	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.6	7.3	Hz
Overswing Ratio	3.5	3.6	4.0	

Peak Vector Sum 2.910 mm/s at 0.359 sec

Serial Number Battery Level Unit Calibration File Name

BE8720 V 10.72-8.17 MiniMate Plus 5.9 Volts (Battery Low) March 13, 2017 by Instantel J720GVLD.TW0

USBM RI8507 And OSMRE 254-+ + + + 200-100-50 20 10 2 1 2 10 20 50 100 > 5 Frequency (Hz) Tran: + Vert: x Long: Ø



Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger =



 Date/Time
 Tran at 15:03:32 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

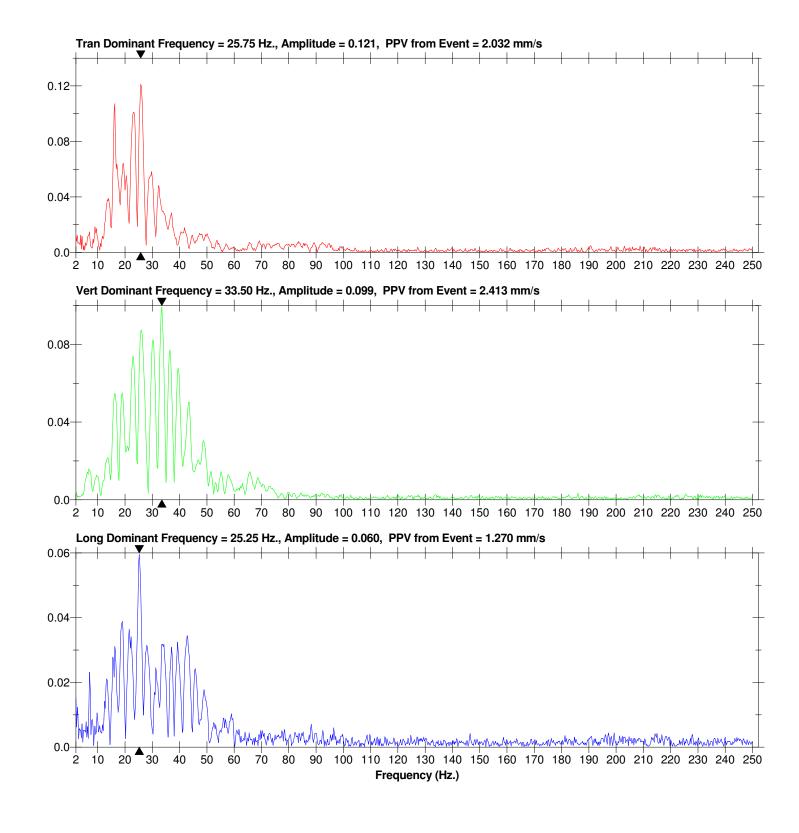
Notes

Location:	Т5
Client:	Pattern
User Name:	Golder Associates Ltd.
General:	3m,1668031

Serial NumberBE8720 V 10.72-8.17 MiniMate PlusBattery Level5.9 Volts (Battery Low)Unit CalibrationMarch 13, 2017 by InstantelFile NameJ720GVLD.TW0

Extended Notes

Combo Mode May 4, 2017 14:34:31 Geophone secured into subgrade under sandbag.





Velocity (mm/s)

 Date/Time
 Long at 15:10:55 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Notes

Location: T5 Client: Pattern User Name: Golder Associates Ltd. General: 3m,1668031

Extended Notes

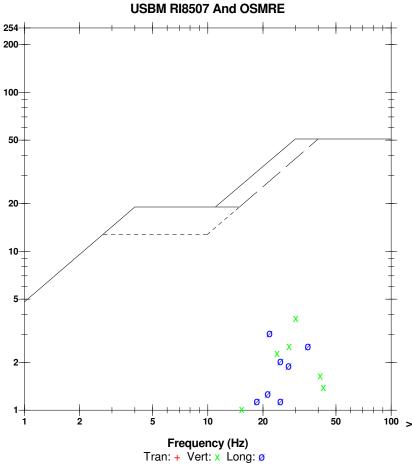
Combo Mode May 4, 2017 14:34:31 Geophone secured into subgrade under sandbag.

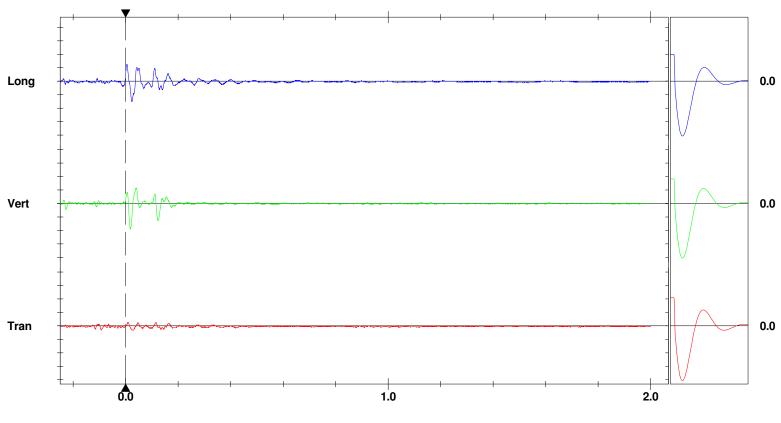
	Tran	Vert	Long	
PPV	0.635	3.810	3.048	mm/s
ZC Freq	47	30	22	Hz
Time (Rel. to Trig)	-0.094	0.018	0.023	sec
Peak Acceleration	0.053	0.080	0.080	g
Peak Displacement	0.005	0.020	0.020	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.6	7.3	Hz
Overswing Ratio	3.5	3.6	4.0	

Peak Vector Sum 4.224 mm/s at 0.021 sec

Serial Number B Battery Level S Unit Calibration F File Name

BE8720 V 10.72-8.17 MiniMate Plus 5.9 Volts (Battery Low) March 13, 2017 by Instantel J720GVLE.670





Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger = ▶ ____ ◀



 Date/Time
 Long at 15:10:55 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

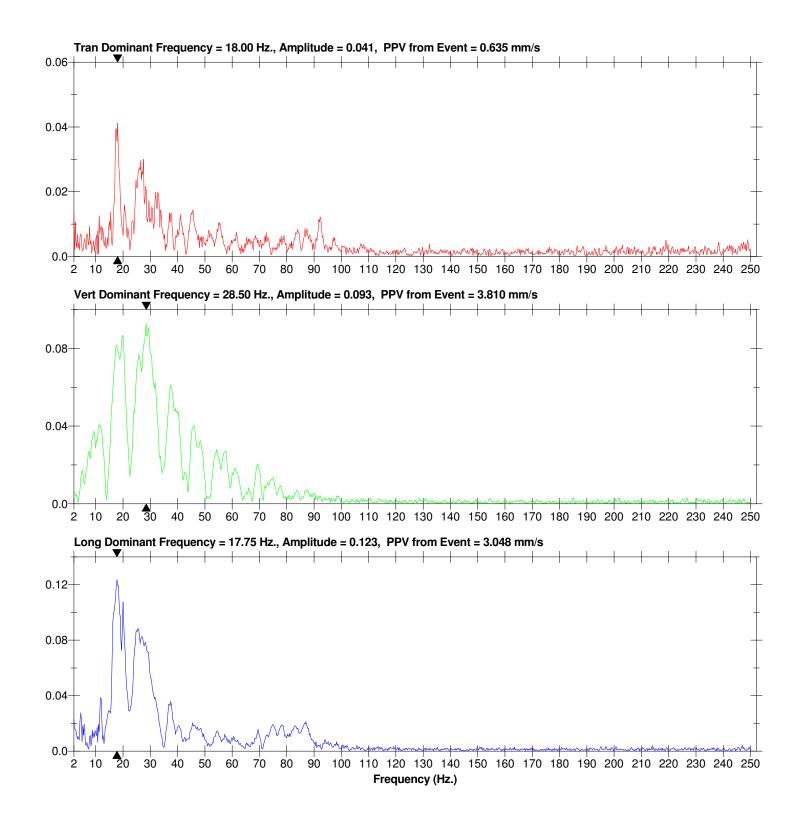
 Record Time
 2.0 sec at 2048 sps

Notes

Location: T5 Client: Pattern User Name: Golder Associates Ltd. General: 3m,1668031 Serial NumberBE8720 V 10.72-8.17 MiniMate PlusBattery Level5.9 Volts (Battery Low)Unit CalibrationMarch 13, 2017 by InstantelFile NameJ720GVLE.670

Extended Notes

Combo Mode May 4, 2017 14:34:31 Geophone secured into subgrade under sandbag.





Velocity (mm/s)

 Date/Time
 Long at 15:10:58 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

 Record Time
 2.0 sec at 2048 sps

Notes

Location: T5 Client: Pattern User Name: Golder Associates Ltd. General: 3m,1668031

Extended Notes

Combo Mode May 4, 2017 14:34:31 Geophone secured into subgrade under sandbag.

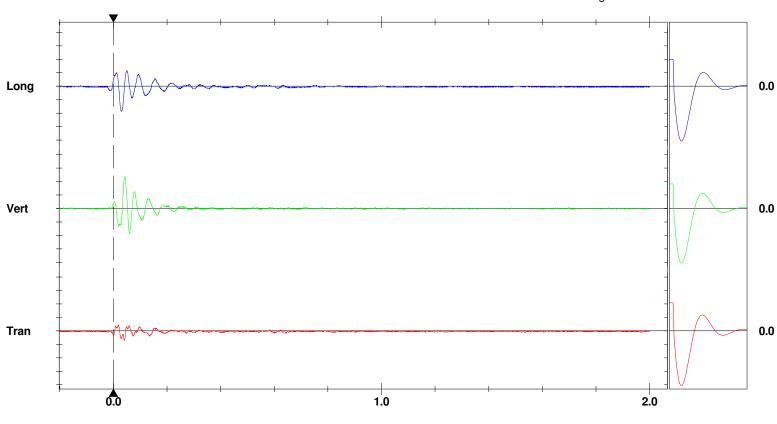
	Tran	Vert	Long	
PPV	1.397	4.699	3.683	mm/s
ZC Freq	23	26	24	Hz
Time (Rel. to Trig)	0.040	0.042	0.028	sec
Peak Acceleration	0.053	0.106	0.080	g
Peak Displacement	0.009	0.028	0.026	mm
Sensor Check	Passed	Passed	Passed	
Frequency	7.6	7.6	7.3	Hz
Overswing Ratio	3.5	3.6	4.0	

Peak Vector Sum 4.909 mm/s at 0.042 sec

Serial Number Battery Level Unit Calibration File Name

BE8720 V 10.72-8.17 MiniMate Plus 5.9 Volts (Battery Low) March 13, 2017 by Instantel J720GVLE.6A0

USBM RI8507 And OSMRE 254-+ + + + 200-100-50 20 10-5 Х ø Х x 2 ø øø ø X + Ø 1 2 5 10 20 50 100 > Frequency (Hz) Tran: + Vert: x Long: Ø



Time Scale: 0.20 sec/div Amplitude Scale: Geo: 2.000 mm/s/div Trigger =



 Date/Time
 Long at 15:10:58 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

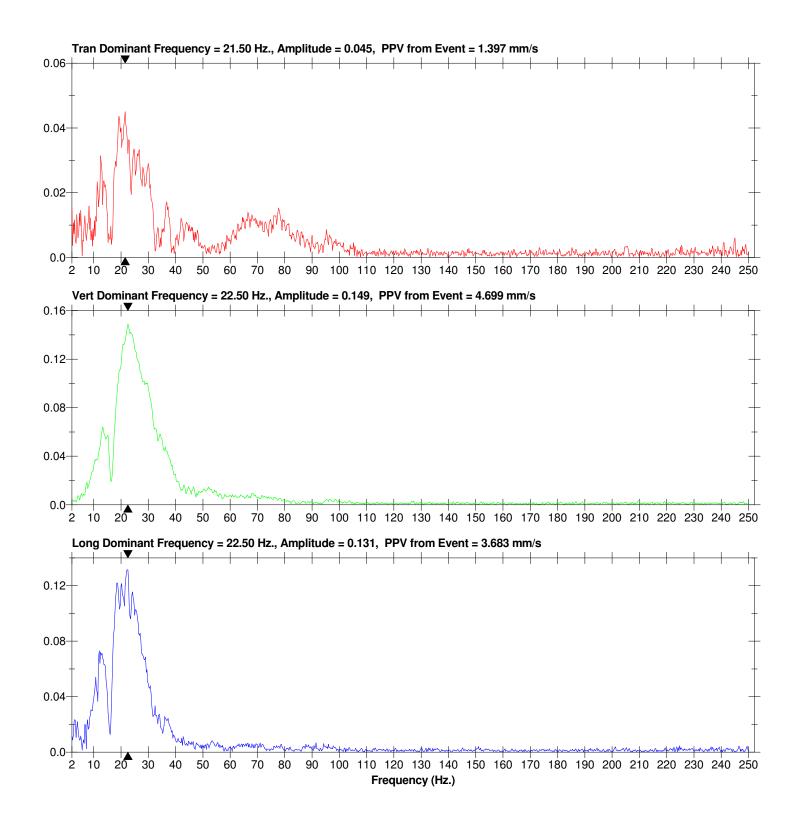
 Record Time
 2.0 sec at 2048 sps

Notes

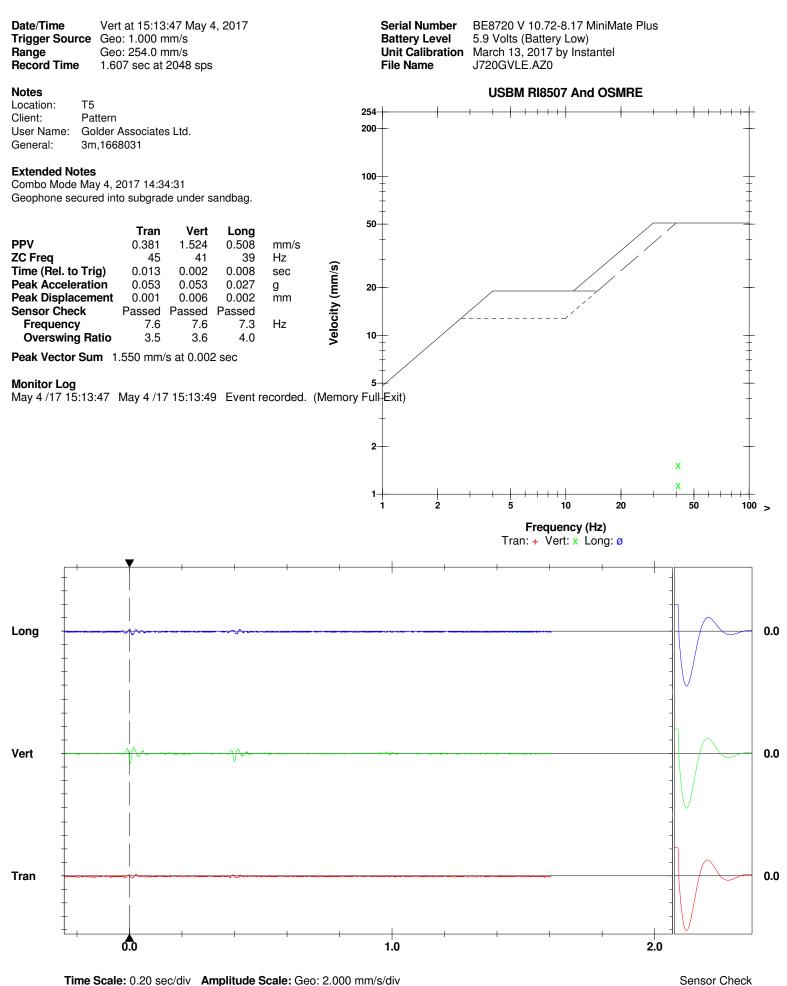
Location: T5 Client: Pattern User Name: Golder Associates Ltd. General: 3m,1668031 Serial NumberBE8720 V 10.72-8.17 MiniMate PlusBattery Level5.9 Volts (Battery Low)Unit CalibrationMarch 13, 2017 by InstantelFile NameJ720GVLE.6A0

Extended Notes

Combo Mode May 4, 2017 14:34:31 Geophone secured into subgrade under sandbag.







Trigger =
----Printed: May 12, 2017 (V 10.72 - 10.72.1)



FFT Report

 Date/Time
 Vert at 15:13:47 May 4, 2017

 Trigger Source
 Geo: 1.000 mm/s

 Range
 Geo: 254.0 mm/s

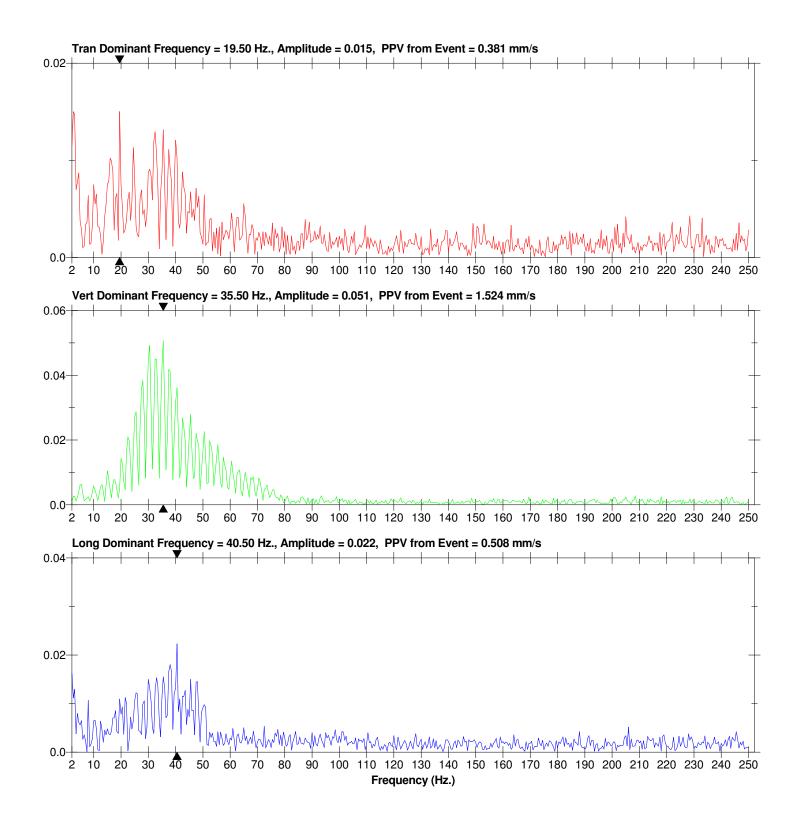
 Record Time
 1.607 sec at 2048 sps

Notes

Location: T5 Client: Pattern User Name: Golder Associates Ltd. General: 3m,1668031 Serial NumberBE8720 V 10.72-8.17 MiniMate PlusBattery Level5.9 Volts (Battery Low)Unit CalibrationMarch 13, 2017 by InstantelFile NameJ720GVLE.AZ0

Extended Notes

Combo Mode May 4, 2017 14:34:31 Geophone secured into subgrade under sandbag.





APPENDIX I Example Data Analyses





EXAMPLE VIBRATION ANALYSES

As described in the report text, ground vibrations induced by impact pile driving are transient waveforms induced by the hammer energy delivered to ground through the pile. In this case, the same source and similar energy was used for each pile strike impulse event. Ground vibrations were monitored using accelerometers. Within the resulting time history of vibration data, acceleration values from pile driving dominate accelerometer response and were many orders of magnitude larger (e.g., 500 times) than background vibrations and system signal noise. The monitored ground vibrations at the domestic water wells were also transient waveforms induced by different sources such as passing traffic, walking or jumping pedestrians (in the case of Golder personnel deliberately inducing vibrations), passing farm equipment, activities within the nearby homes or outbuildings, and wind on the well casing, sensors and cables among other background conditions. The measured acceleration values from day-to-day activities and the induced vibrations at the well locations were extremely small and much nearer the magnitude of background vibrations and signal noise (i.e., 2 to 10 times background). Multiple examples of data analysis are provided in this appendix to illustrate evaluation of:

- acceleration data time histories;
- transformation of acceleration time histories to frequency spectra through the fast Fourier transform (FFT) process;
- transformation/integration of the acceleration-frequency spectra to velocity spectra; and
- selection of the maximum particle velocities for any given time period of analysis.

An overall description of each one of these evaluation steps types is provided below for additional context, followed by an annotated list for the examples providing notes for reviewing the provided graphics. The figures provided in this appendix are numbered consistent with the examples as numbered. When examples or results summaries have been already provided in the report text and additional figures are not included, these figure numbers are omitted.

Discrete vibration events (e.g., pile driving, jumping up and down) were clearly discernable in time histories of measured acceleration data. The vibration events associated with pile driving in this case exhibited a waveform amplitude that increased suddenly then diminished over short periods of time (less than 1 second in most cases) to near background values. As described in the report text, each individual pile strike event was evaluated separately where the block of the time history subject to FFT and velocity analysis was picked to be coincident with the start of the impulse as illustrated by Figure 4 in the report body for several instruments installed within the bedrock at the T42 site. An example acceleration time history overview for Well #1 on the 28th of March, 2017 is provided in Figure I-0 in this appendix (following text). For the specific examples provided in this appendix, acceleration time history data was also clipped and shown at scale ranges adjusted to better illustrate the waveforms for each of the examples.





The clearest demonstration of the relative magnitudes of vibration amplitudes associated with different conditions and distances from the test pile driving are the maximum accelerations measured during any given event and at any given location. Acceleration time histories provided in this appendix illustrate these relative acceleration magnitudes as measured in the field at the test pile and well sites. The acceleration time history and magnitude examples do not include ambiguities, artefacts of data processing or any interpretation. In these cases, whatever background vibrations or electronic signal noise that might be within the data is also included within the measured acceleration values as presented.

To illustrate the relative magnitudes of vibrations measured during test pile monitoring, Table I-1, below summarizes the approximate maximum acceleration values for specific cases that are included either in the body of the report (e.g., Figure 4) or in this appendix. The accelerometers used in the field returned voltage readings when exposed to vibrations. These voltage readings then correlated to specific rated scales of the gravitational acceleration constant for earth, g (9.81 m/s²) for each accelerometer (see Appendix F). Example measured acceleration values are provided in Table I-1, presented in millimetres per second squared (mm/s²) for ease of comparison. Table I-0 also includes a factor F which represents measured acceleration values for different conditions divided by the values measured during the quiet site period at Well #1 on March 28, 2017. This table and the associated examples demonstrate that the pile driving vibrations in the bedrock within close proximity to the test pile were more than 500 times the quiet site values. At the well locations other activities such as driving a van in the driveway, resting a vibrating cell phone on the well casing lid, and walking or jumping near the well caused acceleration magnitudes significantly greater than those induced by pile driving.

It should be noted that multiple factors can affect background vibration conditions and background vibration amplitudes should be expected to be variable at any time on any day. For example, the influence of road traffic passing the site will be influenced by factors including the direction of travel, numbers of vehicles passing the site at any given time interval, vehicle speed, vehicle weight, tire pressures, number of tires spacing of axels in the direction of travel and discontinuities in the road at the time of the vehicle travel. Environmental conditions and background vibration levels of uncertain origin also vary. Many of the measured results are extremely small values of peak particle velocity (one to two or more orders of magnitude below thresholds for human perception) and some of the measurement signals at very small amplitudes can be dominated by background vibrations, electronic signal noise, and environmental influences.

Example Number	Example Condition	Approximate Acceleration Magnitude (mm/s ²)		F
		Negative	Positive	;
1	Borehole BH101 (Rock) - During Pile Driving on March 29, 2017 (see Figure 4)	-5236	4232	567
2	Borehole BH102 (Rock) - During Pile Driving on March 29, 2017 (see Figure 4)	-1470	1280	167
3	Borehole BH103 (Rock) - During Pile Driving on March 29, 2017 (see Figure 4)	-301	299	36
4	Well #1, Quiet Period, March 28, 2017	-8	9	1
5	Well #1, Jumping at 0.9 m from Well, March 28, 2017	-29	65	6

Table 1: Summary of Acceleration Magnitudes for Calculation Examples





Example Number	Example Condition	Approximate Acceleration Magnitude (mm/s ²)		F			
		Negative	Positive				
6	Well #1, Jumping at 1.8 m from Well, March 28, 2017	-42	40	5			
7	Well #1, Jumping at 2.7 m from Well, March 28, 2017	-29	25	3			
8	Well #1, Approaching Well, March 28, 2017	-4	5	1			
9	Well #1, Approaching Well, March 28, 2017	-19	16	2			
10	Well #1, Cell Phone Vibrating on Well Lid, March 28, 2017	-15	16	2			
11	Well #1, Walking on Porch, March 28, 2017	-9	9	1			
12	Well #1, Van on Driveway, March 28, 2017	-5	6	1			
13	Well #3, Identified Pile Driving Impulses, May 3, 2017	-2	2	0			
14	Well #3, Jumping 1.0 m Well, May 3, 2017	-20	20	2			
15A	T42, Borehole BH-103, March 23, 2017, Quiet Site	-0.2	0.5				
15B	T42, Borehole BH-103, March 23, 2017, Vacuum Truck	-0.7	0.9				

As described in the report text, the Fast Fourier Transformation (FFT) process was used to transform data from the acceleration-time domain to acceleration-frequency domain to permit subsequent integration to velocity spectra. Each of the examples dbelow includes a series of graphs illustrating various aspects of the signal analysis, processing and final results, except as noted with cross-reference to information already presented in the body of the report or the associated figures. As noted previously, the appendix figures are numbered consistent with the example numbers and, where the examples do not include additional figures, the representative numbers are omitted.

1) Borehole BH101 (Rock) - During Pile Driving

See Figure 4 and discussion in body of report

2) Borehole BH102 (Rock) - During Pile Driving

See Figure 4 and discussion in body of report

3) Borehole BH103 (Rock) - During Pile Driving

See Figure 4 and discussion in body of report

4) Well #1, Quiet Period, March 28, 2017

Figure I-4A illustrates a specific time segment (detail) from the overall Well #1 time history illustrated in Figure I-0 with the vertical and horizontal scales adjusted to better illustrate the signal in this region of the time history. Figure I-4B illustrates the acceleration FFT for a series of one-second intervals within this specific time period. Figure I-4C depicts the subsequent velocity spectra and the maxima selected from these same spectra.





5) Well #1, Jumping at 0.9 m from Well, March 28, 2017

Figure I-5A illustrates a specific time segment (detail) from the overall Well #1 time history illustrated in Figure I-0 with the vertical and horizontal scales adjusted to better illustrate the signal in this region of the time history. Figure I-5B illustrates the acceleration FFT for the series of jump instances and intervals between these instances as reported in Table 15 of the report. Figure I-7 depicts the subsequent velocity spectra.

6) Well #1, Jumping at 1.8 m from Well, March 28, 2017

Figure I-6A illustrates a specific time segment (detail) from the overall Well #1 time history illustrated in Figure I-0 with the vertical and horizontal scales adjusted to better illustrate the signal in this region of the time history. Figures I-6B and I-6C illustrate the FFT and velocity analyses of the data generated by jumping at a distance of 1.8 m from the well.

7) Well #1, Jumping at 2.7 m from Well, March 28, 2017

Figure I-7A illustrates a specific time segment (detail) from the overall Well #1 time history illustrated in Figure I-0 with the vertical and horizontal scales adjusted to better illustrate the signal in this region of the time history. Figures I-7B and I-7C illustrate the FFT and velocity analyses of the data generated by jumping at a distance of 2.7 m from the well.

8) Well #1, Approaching Well, March 28, 2017

Figure I-8A illustrates a specific time segment (detail) from the overall Well #1 time history illustrated in Figure I-0 with the vertical and horizontal scales adjusted to better illustrate the signal in this region of the time history. Figures I-8B and I-8C illustrate the series of FFT and velocity analyses of sequential one-second intervals during the footfalls as a Golder employee walked toward the well during the monitoring period.

9) Well #1, Approaching Well, March 28, 2017

See analysis results as presented in Table 15 in body of the report for a composite set of footfalls and time history that produced larger acceleration readings through the FFT process.

10) Well #1, Cell Phone Vibrating on Well Lid, March 28, 2017

Cell phone vibrations were induced by a source with a known vibration pulsation frequency and consistent amplitude. This very minor source could be directly identified and is one of the more clearly distinguishable deliberately-induced conditions. Figure I-10A illustrates a specific time segment (detail) from the overall Well #1 time history illustrated in Figure I-0 with the vertical and horizontal scales adjusted to better illustrate the signal in this region of the time history. Figures I-10B and I-10C illustrate the FFT and velocity analyses for the period of time during which the cell phone was ringing.

11) Well #1, Walking on Porch, March 28, 2017

Figure I-11A illustrates a specific time segment (detail) from the overall Well #1 time history illustrated in Figure I-0 with the vertical and horizontal scales adjusted to better illustrate the signal in this region of the time history. Figures I-11B and I-11C illustrate the FFT and velocity analyses the footfalls as an individual walked up the steps, rang the residence doorbell and descended the steps during the monitoring period.





12) Well #1, Van on Driveway, March 28, 2017

Figure I-12A illustrates a specific time segment (detail) from the overall Well #1 time history illustrated in Figure I-0 with the vertical and horizontal scales adjusted to better illustrate the signal in this region of the time history. Figures I-12B and I-12C illustrate the series of FFT and velocity analyses of sequential one-second intervals during the time a Golder employee drove a light utility van from the northern end of the driveway to the road and back to the well during the monitoring period.

13) Well #3, Identified Pile Driving Impulses, May 3, 2017

Figure I-13A illustrates a time history of accelerometer voltage data from all three accelerometers mounted on Well #3 located 911 metres from the T5 test pile site. In this and similar images, the top graph represents data from the vertically-oriented instrument, the middle graph represents the accelerometer oriented in the longitudinal direction toward the test pile, and the bottom presents data from the instrument oriented perpendicular to the longitudinal accelerometer (i.e., transverse direction). In this particular case, the accelerometer is calibrated to 1 *g* per 1000 millivolts, or 1 g per volt. This time history includes a period during which pile driving impulses could be discerned against a very low-noise background signal. This time history also includes a period during which a Golder employee jumped at a distance of 1.0 m from the well. Within this time history, the pile driving related impulses were detected near first 1/3 distance along the history and the vibrations induced by an employee jumping on the ground are near the end of this time history as illustrated in Figures I-13B and I-13C.

Figure I-13D illustrates a specific time segment (detail) from the Well #3 time history associated with pile driving as illustrated in Figures I-13A and I-13B with the vertical and horizontal scales adjusted to better illustrate the signal in this region of the time history. Figures I-13E and I-13F illustrate examples of FFT and velocity analyses associated with the pile driving impulses.

14) Well #3, Jumping 1.0 m Well, May 3, 2017

Figure I-14A illustrates a specific time segment (detail) from the Well #3 time history illustrated in Figure I-13A and I-13C with the vertical and horizontal scales adjusted to better illustrate the signal in this region of the time history. Figures I-14B and I-14C illustrate the series of FFT and velocity analyses associated with the individual impulses associated with each of the three jumping actions and intervening time periods.

15) T42 Site, March 23, 2017

Figures I-15A illustrates example acceleration data obtained during a quiet period on the T42 test site on March 23, 2017 in the absence of any site activities, without traffic on the nearby roadway and during optimum weather conditions. As summarized in the report text the particle velocity during this period, as evaluated from the most sensitive uniaxial accelerometer data, was about 0.0029 mm/s. Figures I-15B illustrates example acceleration data obtained during a time period on the T42 test site on March 23, 2017 when the vacuum truck was backing up slowly along the wooden mats and the particle velocity for this activity was about 0.0033 mm/s using data from the uniaxial accelerometer.





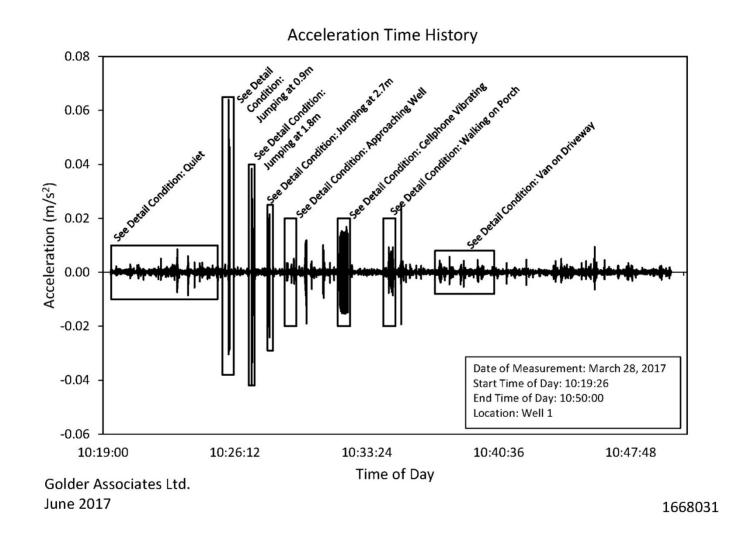
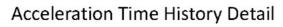


Figure I-01: March 28 Time History.







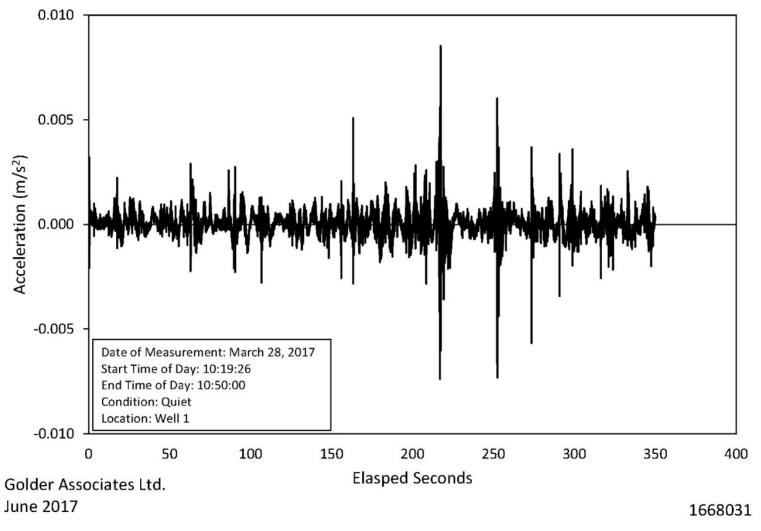
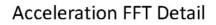
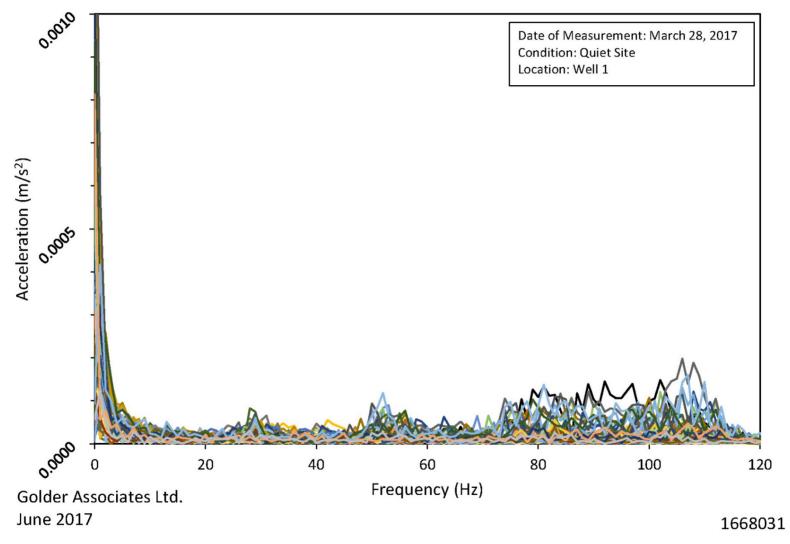


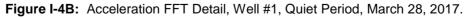
Figure I-4A: Acceleration-Time History Detail, Well #1, Quiet Period, March 28, 2017.







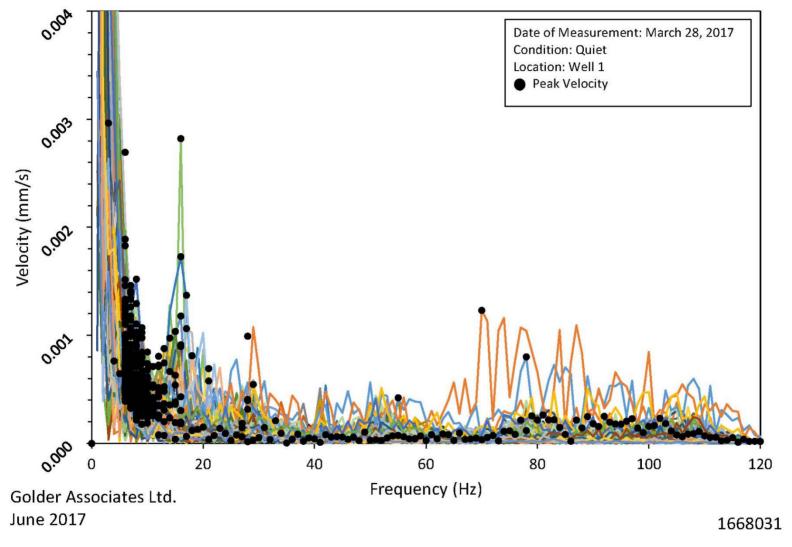


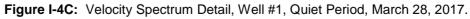




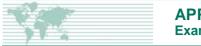


Velocity Spectrum Detail









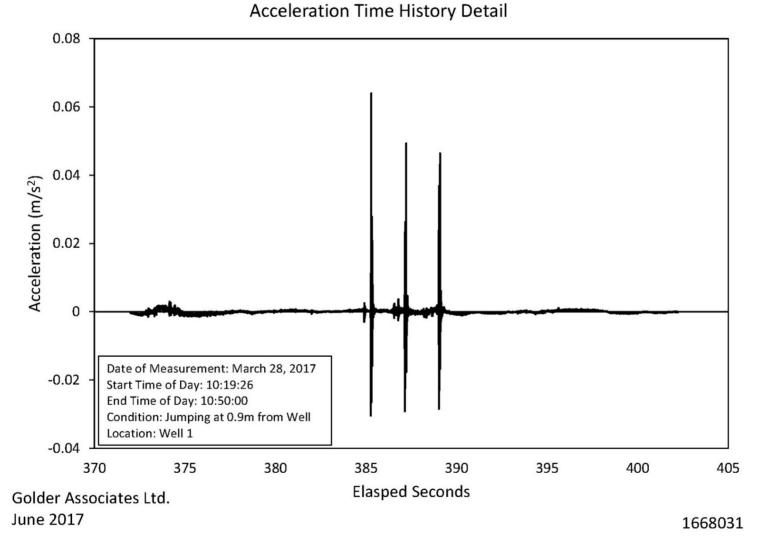


Figure I-5A: Acceleration-Time History Detail, Well #1, Jumping at 0.9 m from Well, March 28, 2017.





Acceleration FFT Detail

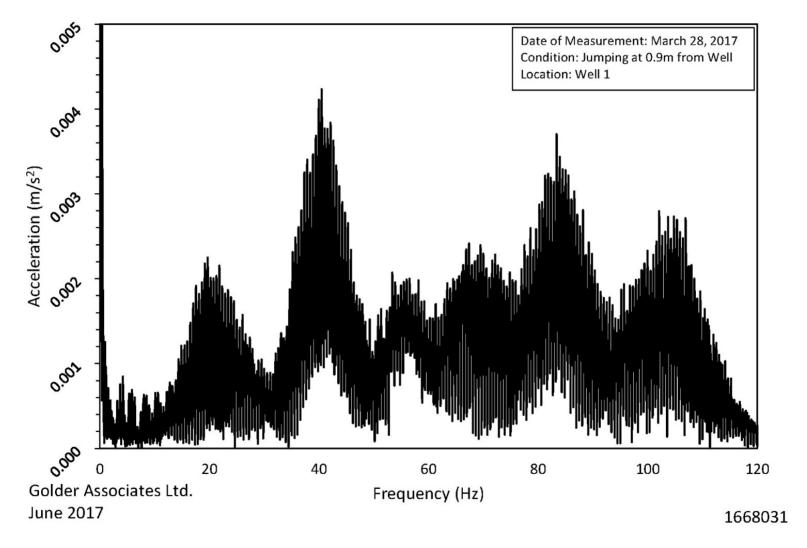


Figure I-5B: Acceleration FFT Detail, Well #1, Jumping at 0.9 m from Well, March 28, 2017.





Velocity Spectrum Detail

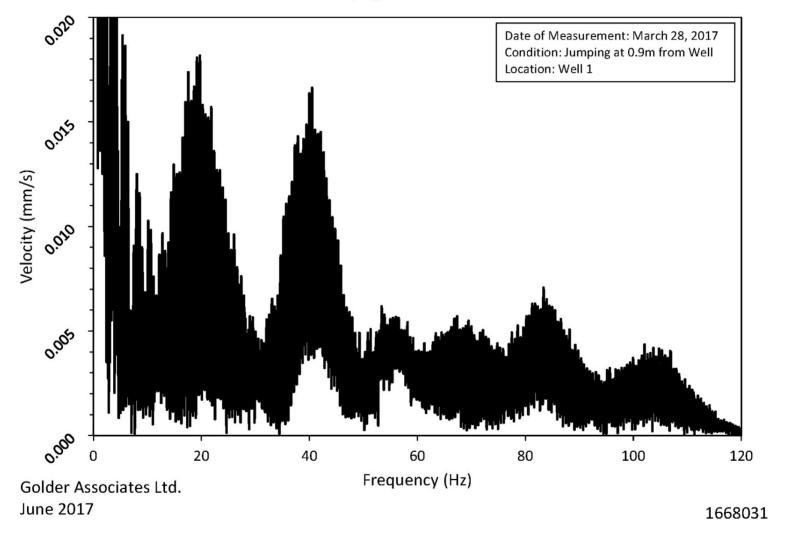
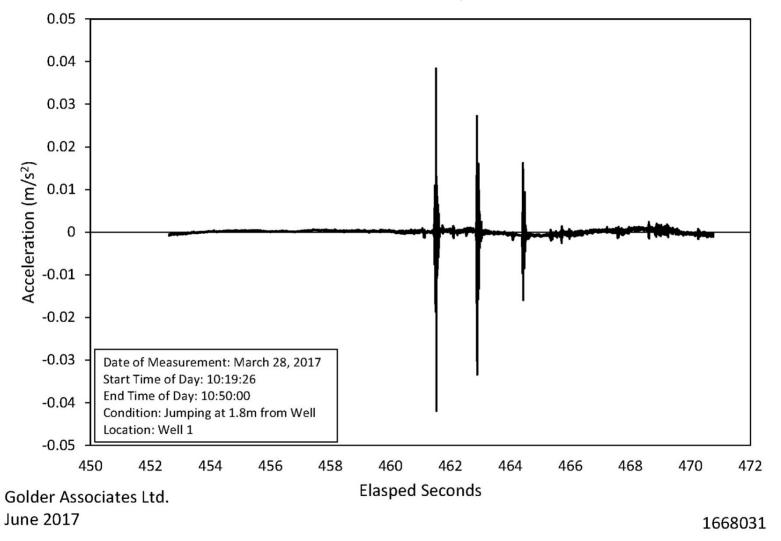


Figure I-5C: Velocity Spectrum Detail, Well #1, Jumping at 0.9 m from Well, March 28, 2017.





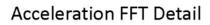


Acceleration Time History Detail

Figure I-6A: Acceleration-Time History Detail, Well #1, Jumping at 1.8 m from Well, March 28, 2017







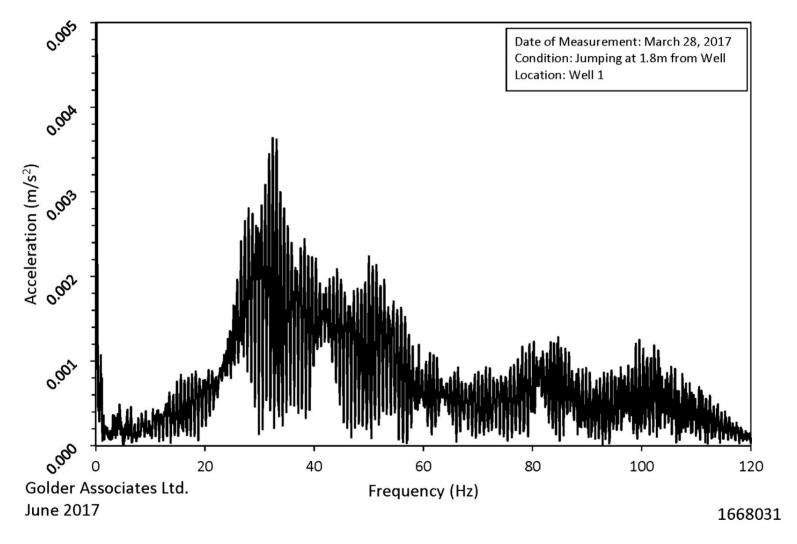


Figure I-6B: Acceleration FFT Detail, Well #1, Jumping at 1.8 m from Well, March 28, 2017.







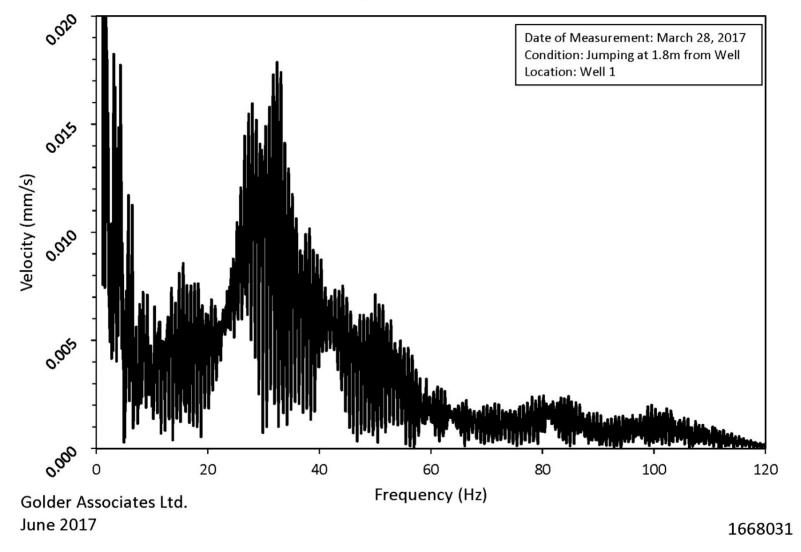


Figure I-6C: Velocity Spectrum Detail, Well #1, Jumping at 1.8 m from Well, March 28, 2017.





Acceleration Time History Detail

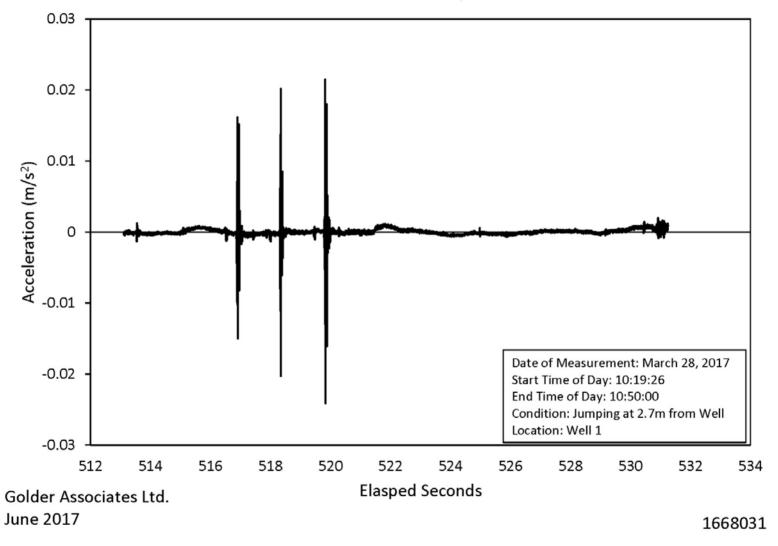
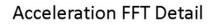


Figure I-7A: Acceleration-Time History Detail, Well #1, Jumping at 2.7 m from Well, March 28, 2017.







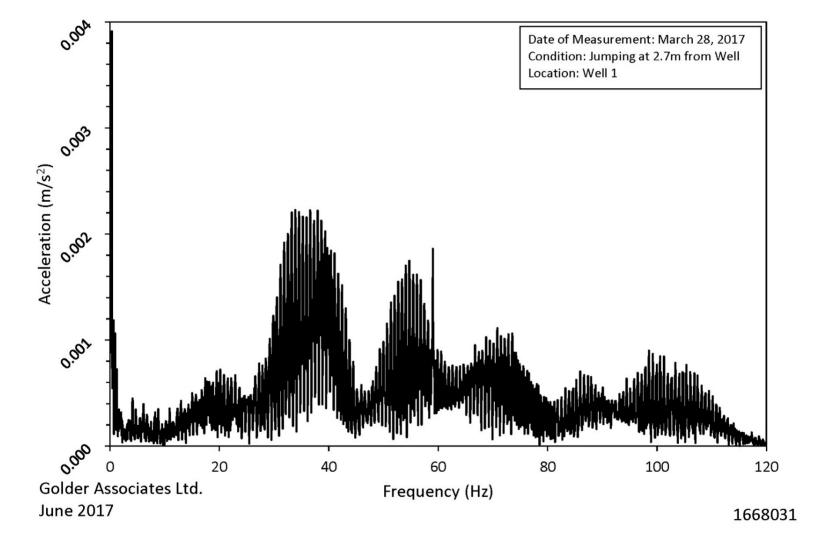


Figure I-7B: Acceleration FFT Detail, Well #1, Jumping at 2.7 m from Well, March 28, 2017





Velocity Spectrum Detail

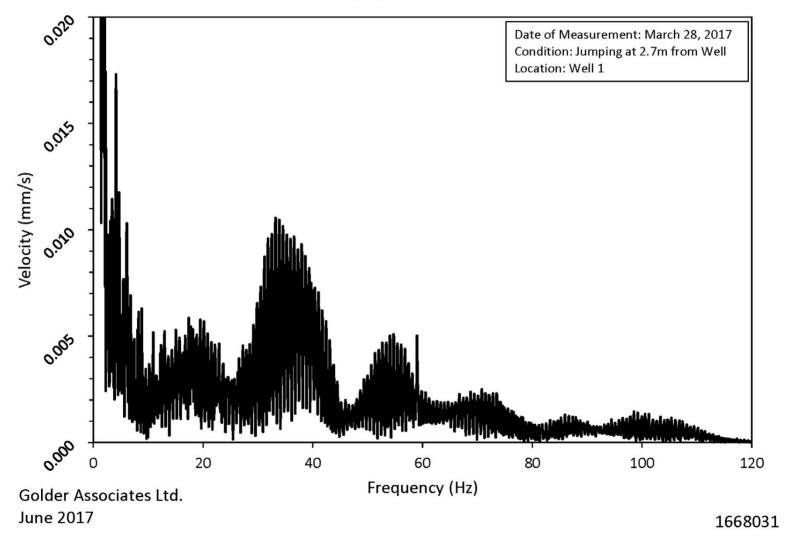


Figure I-7C: Velocity Spectrum Detail, Well #1, Jumping at 2.7 m from Well, March 28, 2017.





Acceleration Time History Detail

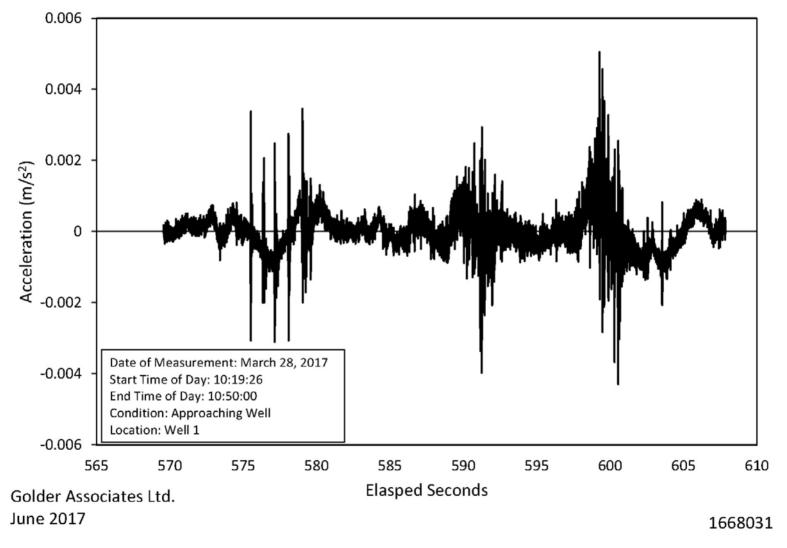
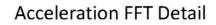
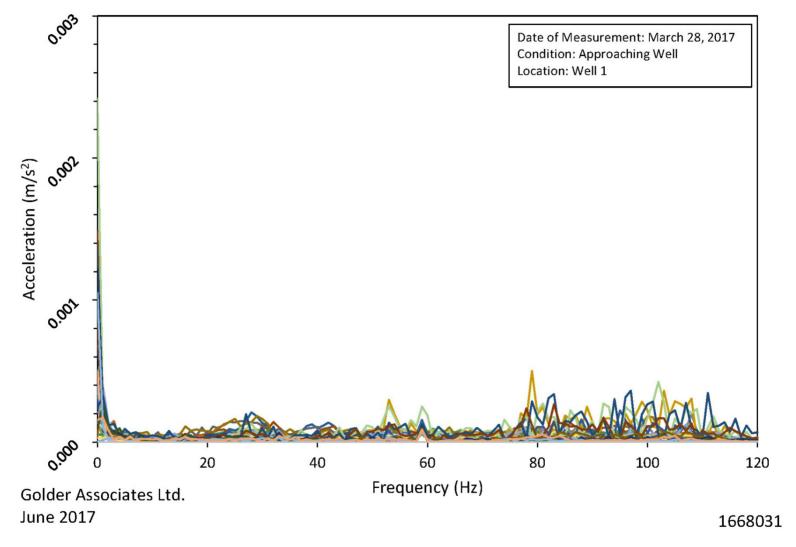


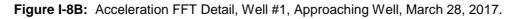
Figure I-8A: Acceleration-Time History Detail, Well #1, Approaching Well, March 28, 2017.





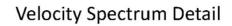


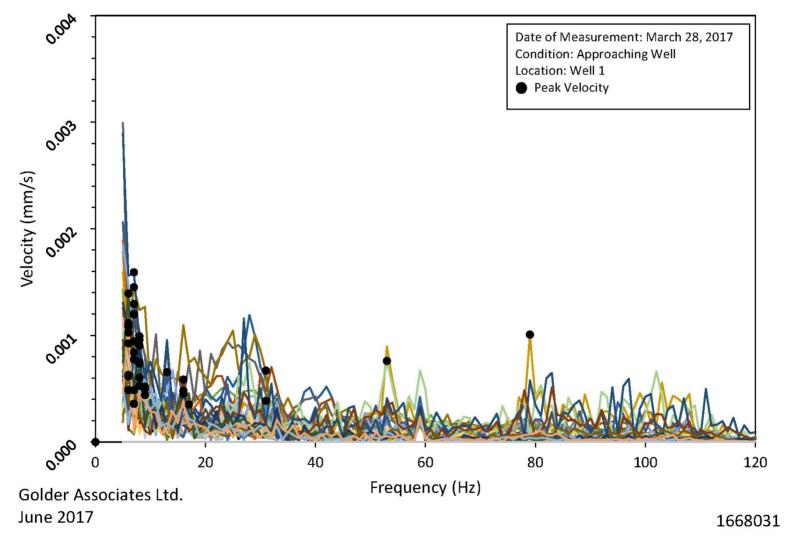


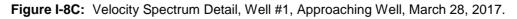
















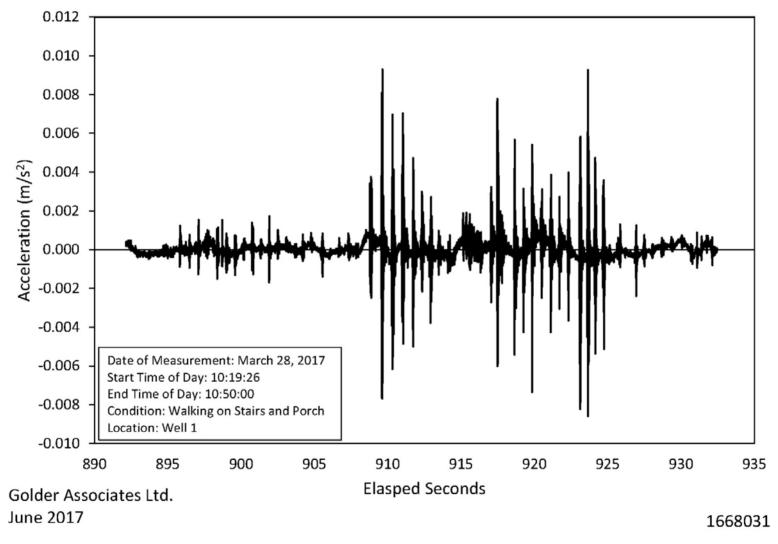
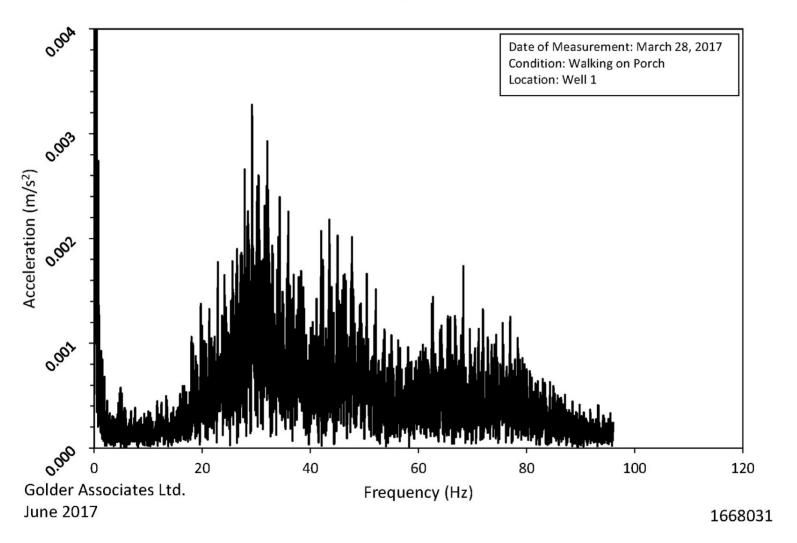


Figure I-11A: Acceleration-Time History Detail, Well #1, Walking on Porch, March 28, 2017.



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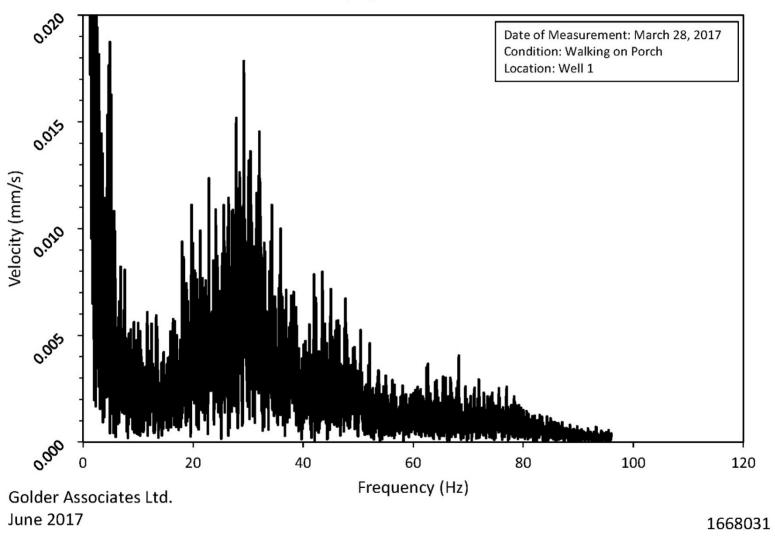


Acceleration FFT Detail

Figure I-11B: Acceleration FFT Detail, Well #1, Walking on Porch, March 28, 2017.







Velocity Spectrum Detail





Acceleration Time History Detail

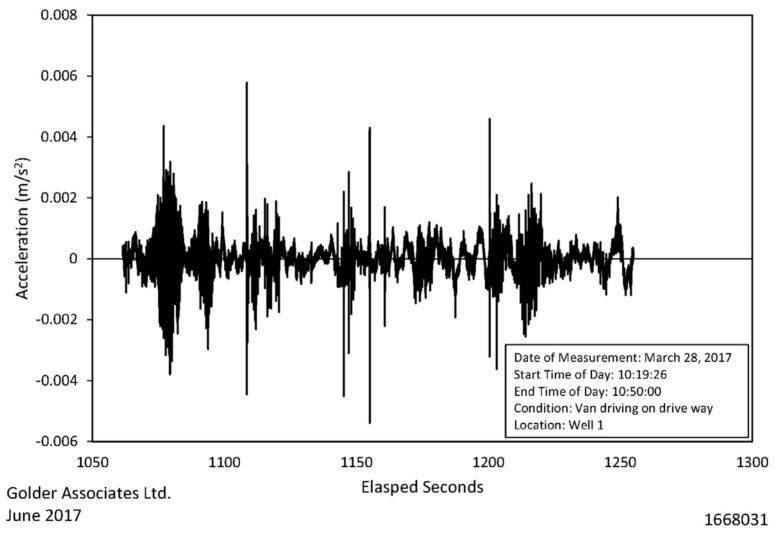
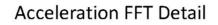
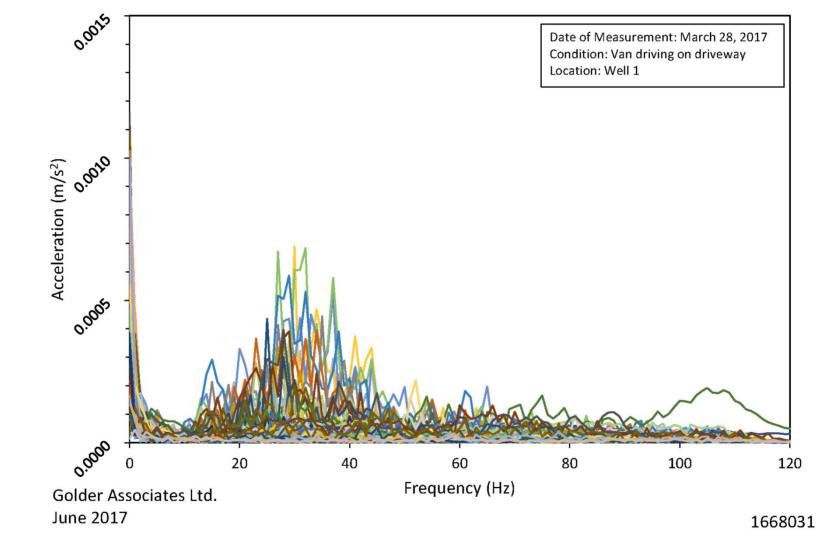


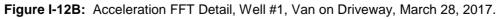
Figure I-12A: Acceleration-Time History Detail, Well #1, Van on Driveway, March 28, 2017.





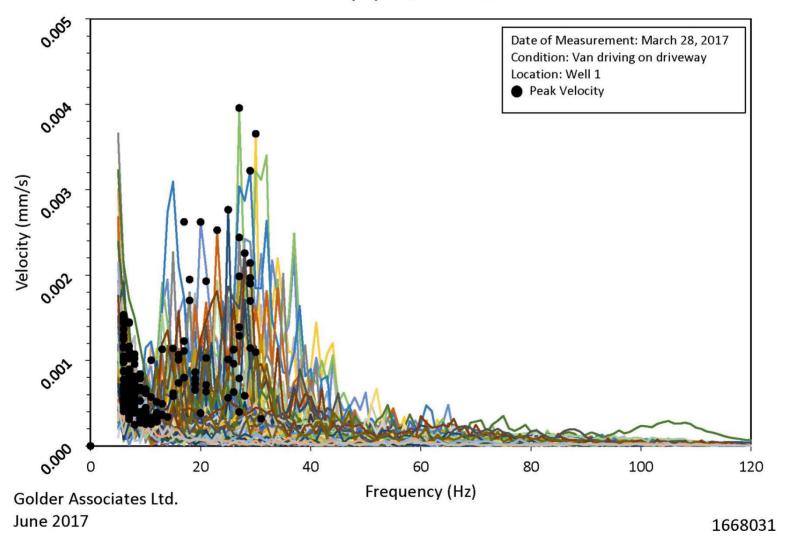








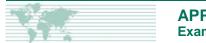




Velocity Spectrum Detail

Figure I-12C: Velocity Spectrum Detail, Well #1, Van on Driveway, March 28, 2017.





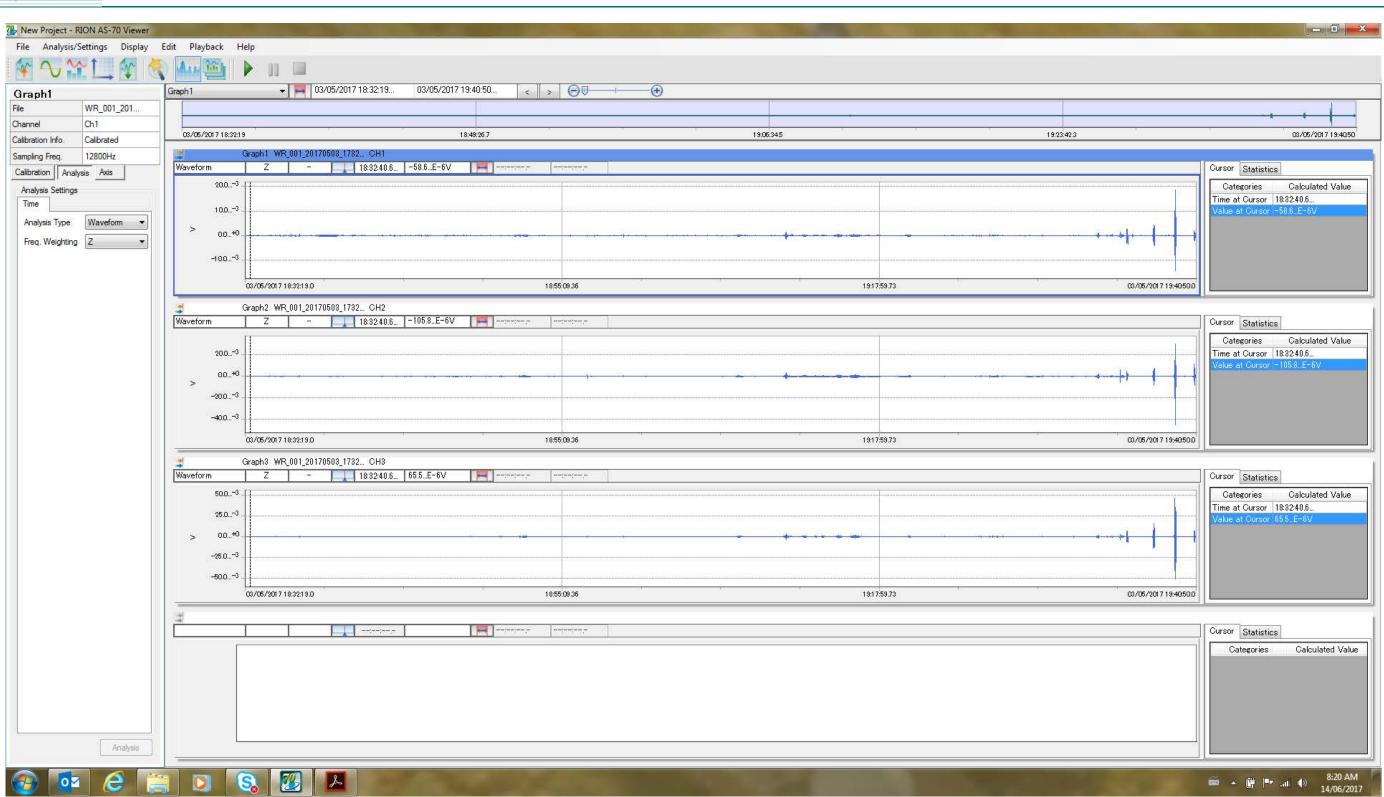


Figure I-13A: Acceleration Time History, Well #3, May 3, 2017 illustrating measured accelerometer voltage readings from mounted to well casing in vertical, longitudinal and transverse directions shown top to bottom of image, respectively. The minimum and maximum vertical axes tick mark labels and grid lines, as illustrated in each the graphs (top to bottom in image) are as follows, respectively: -10.0x10⁻³ V and 20.0x10⁻³ V; -40.0x10⁻³ V; and 20.0x10⁻³ V; and 50.0x10⁻³ V. For these accelerometers, 1 V equates to 1 g.





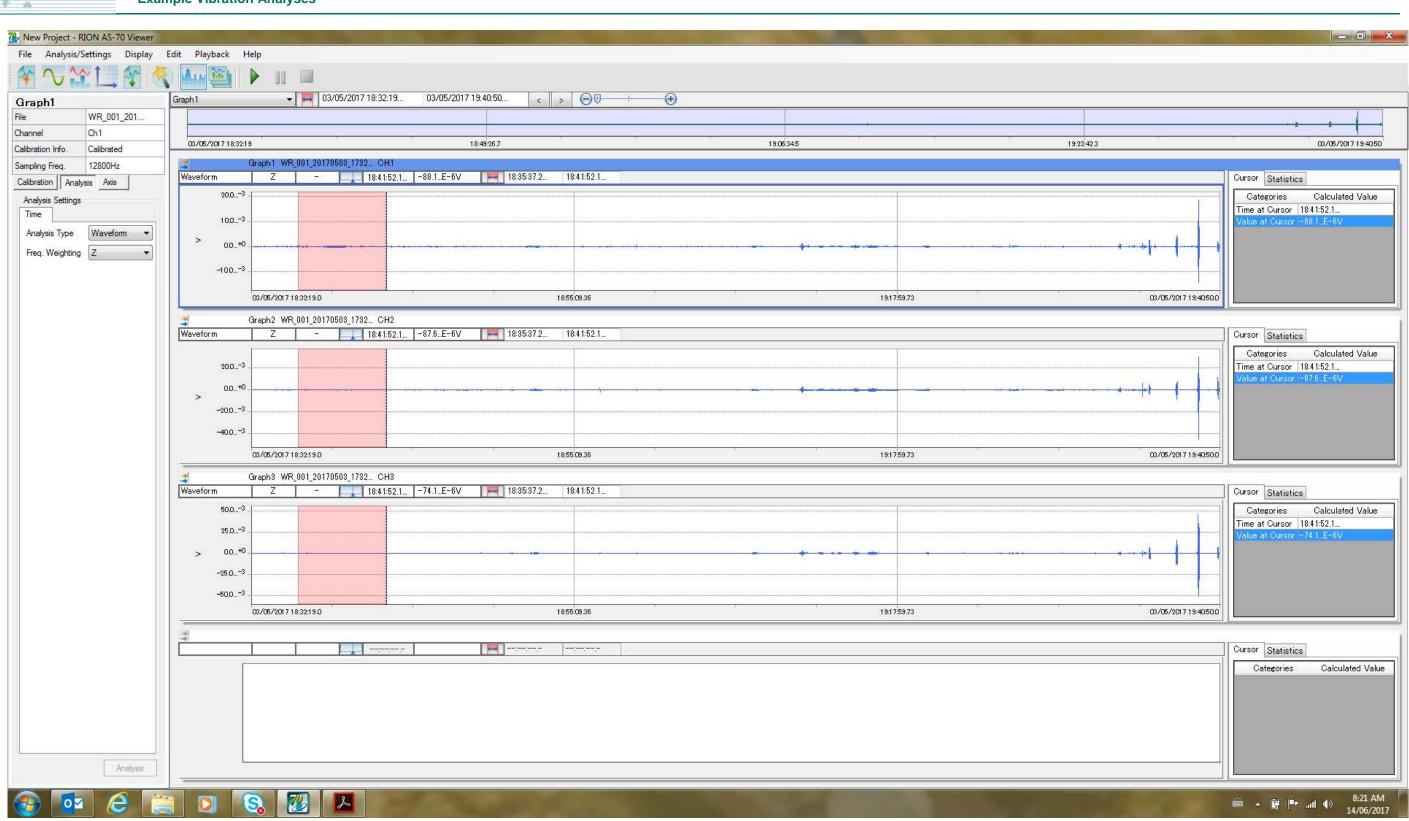


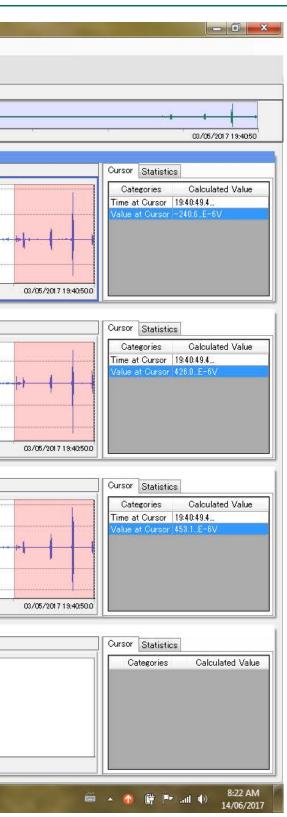
Figure I-13B: Acceleration Time History, Well #3, May 3, 2017 illustrating section of time history shown in Figure I-13A during which pile driving vibrations were identified.



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Figure I-13C: Acceleration Time History, Well #3, May 3, 2017 illustrating section of time history shown in Figure I-13A during time period when Golder employee jumped at 1 m from well.

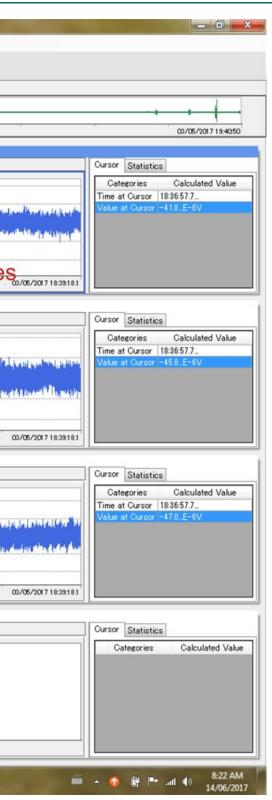






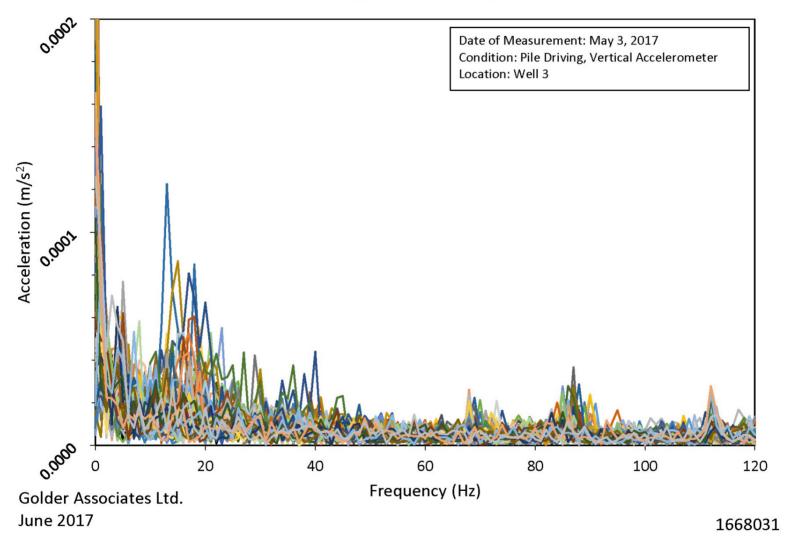
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Figure I-13D: Acceleration Time History Detail, Well #3, May 3, 2017 illustrating section of time history during which pile driving vibrations were identified as shown in Figures I-13A and I-13B. Order of accelerometer graphs on page as in Figure I-13A. The minimum and maximum vertical axes tick mark labels and grid lines, as illustrated in all graphs are -200.0x10⁻⁶ V and 200.0x10⁻⁶ V (100 times smaller than those shown in Figures I-13A through I-13C).









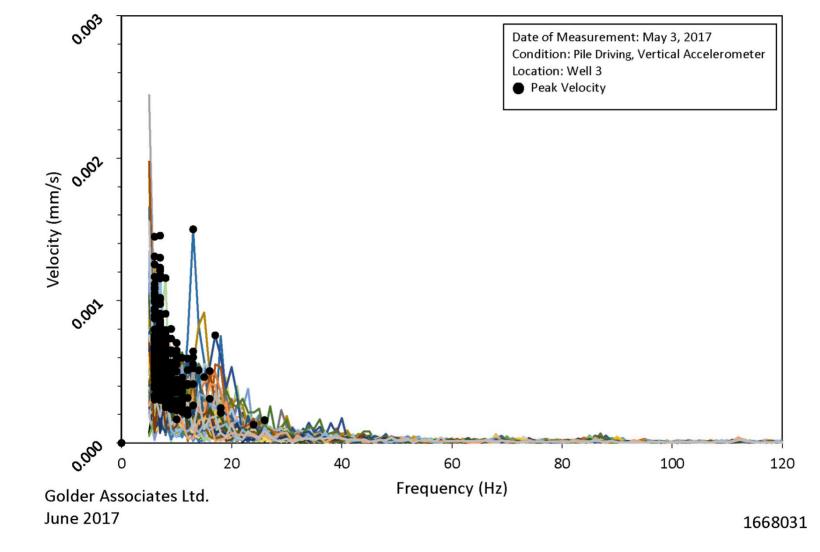
Acceleration FFT Detail

Figure I-13E: Acceleration FFT Detail, Well #3, May 3, 2017.







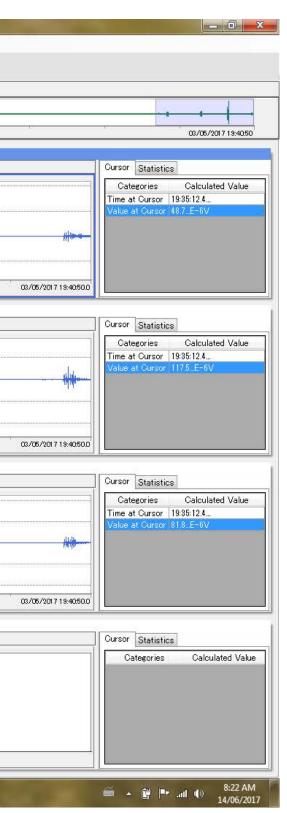






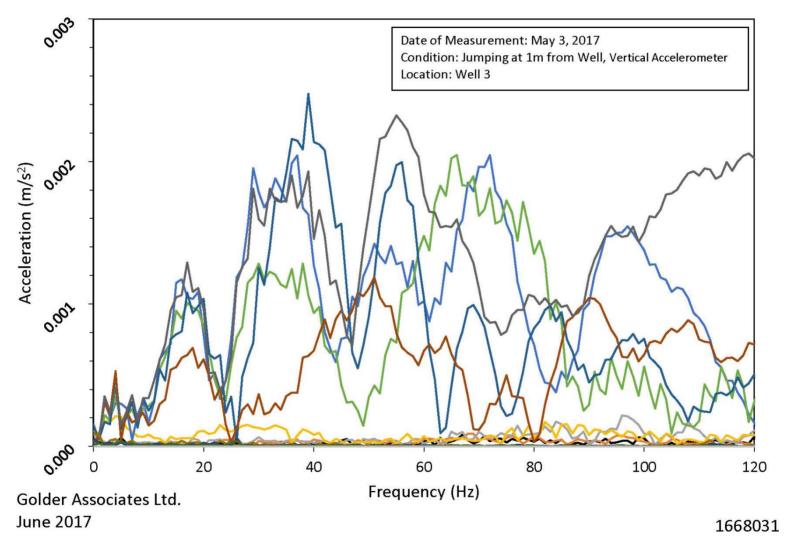
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Figure I-14A: Acceleration Time History Detail, Well #3, May 3, 2017 illustrating section of time history shown in Figures I-13A and I-13B during period when Golder employee jumped at 1 m from well.







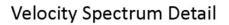


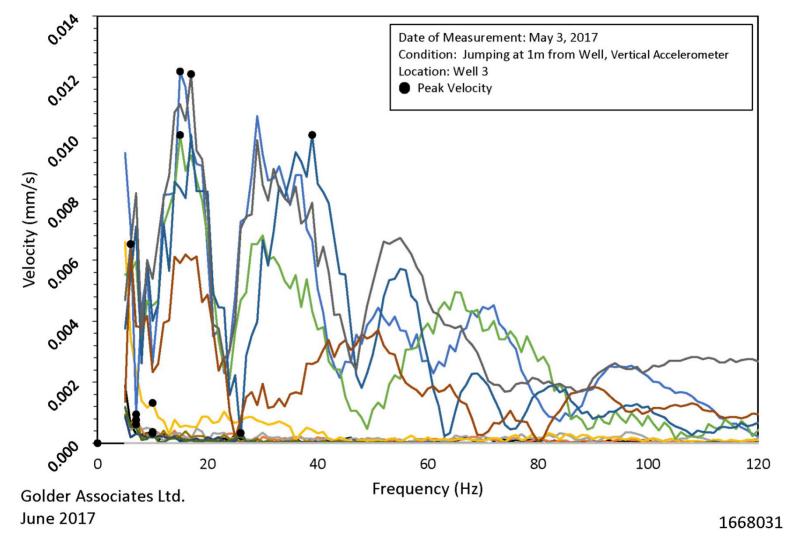
Acceleration FFT Detail

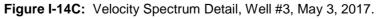
Figure I-14B: Acceleration FFT Detail, Well #3, May 3, 2017.

Golder Associates











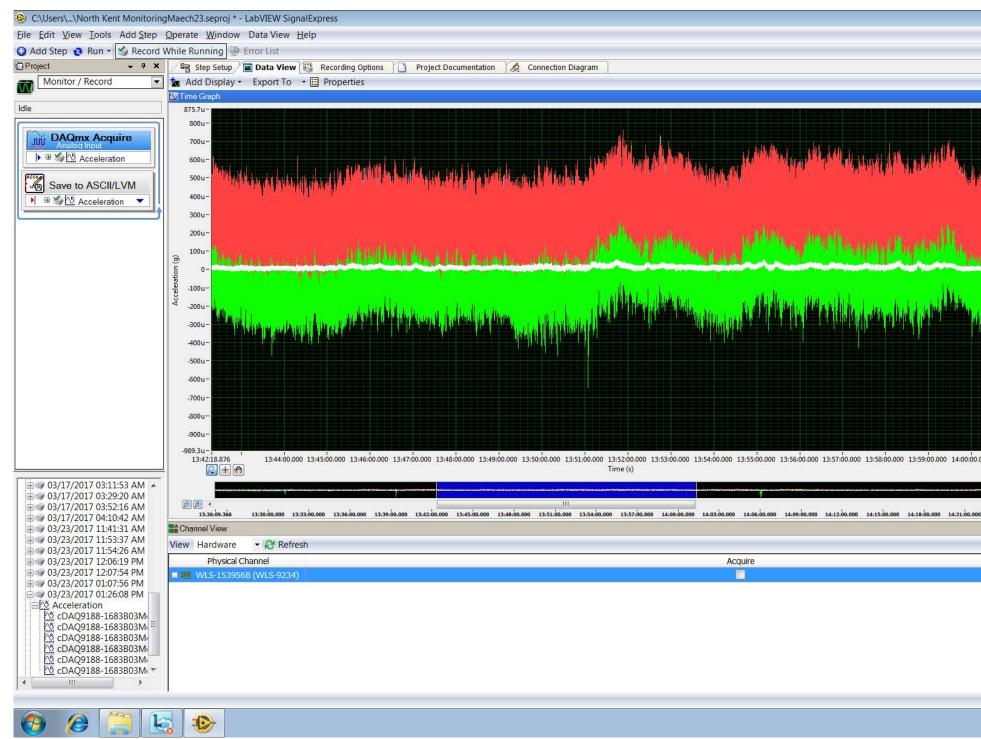


Figure I-15A: Acceleration Time History Detail, T42 Site, March 23, 2017 illustrating data from accelerometers installed within rock at borehole BH-103 during a quiet period in the absence of any site activities and in the absence of nearby traffic. Uniaxial accelerometer data shown in white, with triaxial accelerometer data shown in red (vertical), green (longitudinal) and blue (transverse) for the different orientations. Note that the different ranges of output are primarily related to the differing performance specifications of the two types of instruments. Accelerometer output, using the data logger used at this site, is shown in fractions of the gravitational acceleration constant *g*. The minimum and maximum values shown on the vertical axis are -989.3x10⁻⁶ *g* and 875.7x10⁻⁶ *g (micro-g* as identified on graph axis with unit u).

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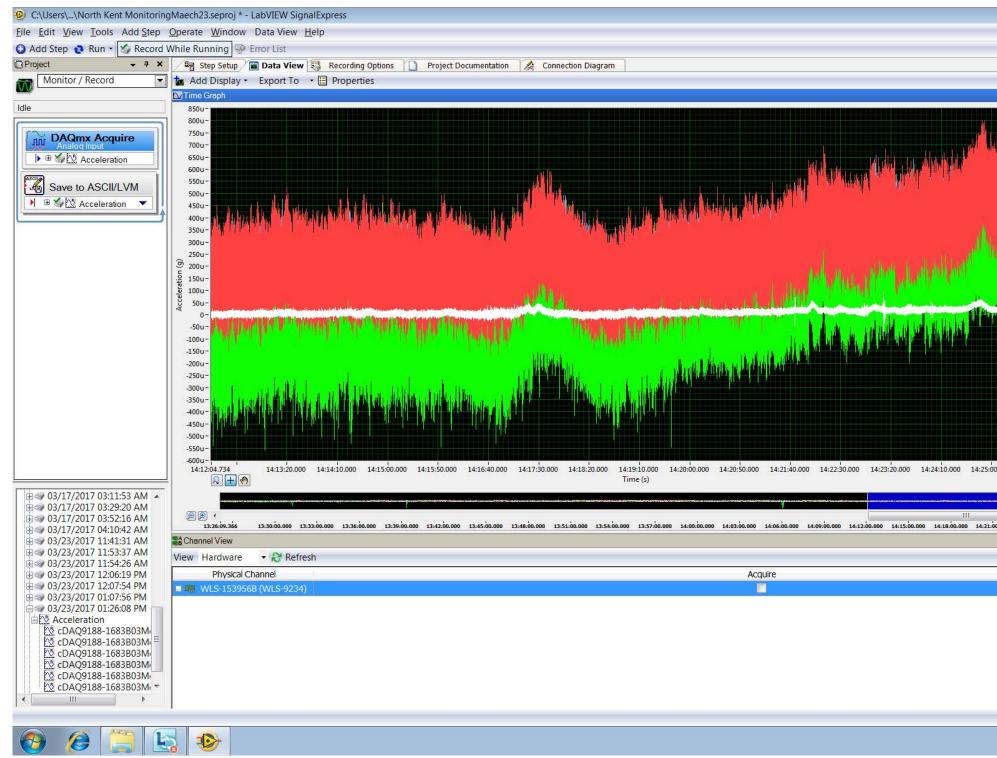


Figure I-15B: Acceleration Time History Detail, T42 Site, March 23, 2017 illustrating acceleration data obtained during a time period on the T42 test site on March 23, 2017 when the vacuum truck backed slowly along the wooden mats, passing borehole BH-103 at a distance of about 5 m, and started operating to remove soil cuttings and water from a bin located about 35 m from borehole BH-103. The minimum and maximum values shown on the vertical axis are -600x10⁻⁶ g and 850x10⁻⁶ g (*micro-g* as identified on graph axis with unit u).

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Asia

+ 27 11 254 4800

+ 86 21 6258 5522

+ 61 3 8862 3500 + 44 1628 851851

North America + 1 800 275 3281

South America + 56 2 2616 2000

Golder Associates Ltd. 309 Exeter Road, Unit #1 London, Ontario, N6L 1C1 Canada T: +1 (519) 652 0099

